



Prepared for:

CAP Norwood Developments Inc.

Cambium Reference: 14288-003

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#### 1.0 Introduction

CAP Norwood Developments Inc. (Client) retained Cambium Inc. (Cambium) to complete a hydrogeological assessment in support of a proposed a residential development at 52 Mill Street in the town of Norwood, Ontario (Site).

It is understood that the proposed development will include mixed density residential land use with 640 residential units, a stormwater management pond, and associated infrastructure including, but not limited to, roads, sidewalks, and greenspace.

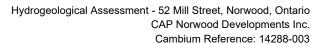
The hydrogeological assessment was required to characterize local hydrogeological conditions for reference at the detailed design stage of development. The hydrogeological assessment included a review of existing documentation, hydraulic testing of monitoring wells, groundwater sampling, a water well survey of surrounding residences, and preliminary dewatering calculations.

Cambium was also retained to complete a geotechnical investigation (Cambium, 2022a), an Environmental Impact Study (EIS) (Cambium, 2023), and a Phase I Environmental Site Assessment (ESA) (Cambium, 2022b). These reports were provided to the Client under separate covers and are referenced herein as appropriate.

## 1.1 Site Description

The Site is irregularly shaped, consisting of approximately 35.4 ha (87 acres) within Lot 18, Concession 9 of Asphodel Township (Figure 1). 52 Mill Street is designated as a Rural Zone (RU). The Site is bound by Mill Street and properties zoned as Residential 1 (R1) Zone and Residential 2 (R2) Zone to the southwest through southeast, Asphodel 10<sup>th</sup> Line and a property zoned as Rural Residential (RR) Zone to the northeast, and a railway line to the northwest.

The Site is currently developed for agricultural use and contains a single family dwelling, a barn, storage sheds, and a silo. The single family dwelling is provided water from a private supply well, and wastewater servicing (presumably) by an on-site treatment system.





It is understood that the proposed development will include mixed density residential land use with 640 units, a stormwater management pond, and associated infrastructure including, but not limited to, roads, sidewalks, and greenspace. Water and wastewater services are expected to be provided by the Township of Asphodel-Norwood (Township). All existing structures and dwellings (including associated water supply well and on-site wastewater treatment system) will be removed prior to development of the Site.

A proposed development concept plan is attached in Appendix A and the Site Plan in included in Figure 1.



## 2.0 Methodology

This section outlines the methodology followed to complete the hydrogeological assessment.

## 2.1 Background Information Review

A review of available relevant background information was undertaken for this study, which included the following resources:

- Chapman, L.J. and Putnam, D.F., 2007. Physiography of Southern Ontario; Ontario
   Geological Survey, Miscellaneous Release Data 228. Scale: 1:50,000.
- Ontario Geological Survey, 1991. Bedrock Geology of Ontario; Ontario Geological Survey.
   Scale: 1:250,000.
- Ontario Geological Survey, 2010. Surficial Geology of Southern Ontario; Ontario Geological Survey, Miscellaneous Release – Data 128-REV. Scale: 1:50,000.
- Source Protection Area Mapping provided by the Ministry of Environment, Conservation and Parks (MECP)
- Water Well Information System provided by the MECP

## 2.2 Borehole Investigation

A borehole investigation and test pit investigation were conducted on April 20, 21 and 23, 24, 2022, to assess subsurface conditions at the Site. A total of seventeen (17) boreholes, designated BH101-22 through BH117-22 and twenty-four (24) test pits, designated TP118-22 through TP141-22, were advanced at the Site for geotechnical and hydrogeological purposes and are shown on Figure 1.

Drilling and sampling for the boreholes was completed using a track-mounted drill rig, under the supervision of a Cambium technician. The boreholes were advanced to their terminated depths by means of continuous flight solid stem augers with 50 mm O.D. split spoon samplers. Standard Penetration Test (SPT) N values were recorded for the sampled intervals as the number of blows required to drive a split spoon (SS) sampler 305 mm into the soil using a 63.5



kg drop hammer falling 750 mm, as per ASTM D1586 procedures. The SPT N values are used in this report to assess consistency of cohesive soils and relative density of non-cohesive materials. Soil samples were collected at 0.75 m intervals from 0 mbeg to 5.03 mbeg. Test pits were excavated to a predetermined depth of 3 mbeg using a Cambium sourced backhoe, under the supervision of a Cambium technician. Dynamic Probe Penetration Test (DPT) values were recorded for the sampled intervals as the number of blows required to drive a 19 mm diameter steel rod 150 mm into the soil with an 8 kg hammer falling 750 mm. The DPT values are used in this report to assess consistency of cohesive soils and relative density of non-cohesive materials. Borehole and test pit logs are provided in Appendix B.

The encountered soil units were logged in the field using visual and tactile methods, and samples were placed in labelled plastic bags for transport, future reference, possible laboratory testing, and storage. Open test pits were checked for groundwater and general stability prior to backfilling. All test pits were backfilled to as close to pre-existing conditions as possible.

Borehole and test pit locations were surveyed in the field using a Sokkia RTK unit. Elevations were measured in relation to the top nut of the fire hydrant located at the intersection of King Street and Mill Street. Geodetic elevation of the fire hydrant was provided by Jewel Engineering to be 206.05 meters above sea level (masl). The ground surface at the location of each borehole and test pit has been measured relative to this elevation, with an accuracy of 0.01m.

The borehole and test pit coordinates and elevations are provided in Appendix B and locations are shown on Figure 1. Site soil and groundwater conditions are described in the following sections of this report.

# 2.3 Physical Soil Testing

Physical laboratory testing, including eight (8) particle size distribution analyses (LS-702, 705), was completed on selected soil samples to confirm textural classification. Moisture content testing (LS-701) was completed on all retrieved soil samples. Results are presented in Appendix C and are discussed in Section 3.1.



## 2.4 Hydrogeological Field Tasks

On April 18, 2022, well surveys were mailed to adjacent properties requesting information on their private supply well (if one exists) on their property. Cambium did not receive any responses to the April 18 letter mailing campaign, so a follow up mailing campaign was completed on May 19, 2022. Details pertaining to the residences contacted as part of the letter mailing campaigns and responses are outlined in Section 4.3. Copies of the letter and water well questionnaire sent to property owners are included in Appendix D.

On May 4, 2022, two groundwater samples were collected from well BH105-22 and BH115-22. The groundwater samples were analysed for general organic and inorganic parameters and compared against the Provincial Water Quality Objective (PWQO) (MOE, 1994). Prior to sample collection at least three well bore volumes of water were purged from the sampled wells. The Certificates of Analysis are attached in Appendix E.

On May 5, 2022, Cambium staff visited the Site to complete Single Well Hydraulic Tests (SWHTs) on the monitoring wells. The SWHTs were completed by inducing an instantaneous change in groundwater head in the well and monitoring water level recovery. Water level recovery was monitored using an automated water level logging device and validated with manual measurements. The hydraulic conductivity of water bearing units screened in each well were estimated using AquiferTest Pro<sup>TM</sup> software, the results of which are attached in Appendix F and are discussed further in Section 4.1.

On May 5, 2022 Cambium staff also located and measured the water level and depth of the existing supply well that serviced the dwelling on-site. The existing residential supply well was located north of the existing dwelling and is outlined on Figure 1. All equipment used in the private supply well investigation was decontaminated prior to usage.



## 3.0 Geological and Hydrogeological Setting

The topography of the Site is generally flat lying, with a general approximate elevation of 210 masl, ranging from 213 masl in the most eastern corner to 205 masl in the most western corner of the Site. Surface water is assumed to flow to the southwest, off-site through a culvert beneath the railway, and discharge into the Ouse River.

The Site is located in the Dummer Moraines region, nestled between Georgian Bay Fringe to the north, and the Peterborough Drumlin fields to the south (Chapman & Putnam, 1984). The Dummer Moraines are an area of rough stony land bordering the Canadian Shield.

According to available mapping, surficial soils at the Site are composed of coarse-grained glaciolacustrine deposits described as sand and gravel with minor silt and clay components. These sediments are interpreted as being deposited in a foreshore or basin depositional environment. The western corner of the Site has sediments described as glaciofluvial deposits composed of primarily gravel and deposited in a fluvial river environment (OGS, 2010).

Site investigations indicate that surficial soils at the Site contain more finer grained sediments than may be expected, as per the available mapping. See section 3.1.

The bedrock of the area consists of Middle Ordovician rocks of the Simcoe Group (OGS, 2007). The Site is composed of rocks composing Bobcaygeon Formation, which is described as limestone with shale partings and interbeds of varying thickness and frequency. The formation is generally 60 to 75 m thick.

## 3.1 Results of Subsurface Investigation

Subsurface conditions at the Site generally consist of layer of loose to compact, dark brown silt topsoil underlain by moist silty sand, which extends to a depth of 0.3 mbeg to 2.1 mbeg in locations BH101-22, BH102-22, BH105-22 to BH113-22, BH115-22 to BH117-22, TP118-22 to TP122-22, TP126-21 to TP130-22, and TP132-22 to TP141-22, and to 3.6 mbeg in TP121-22. In test pits and boreholes BH102-22, BH103-22, BH107-22, BH110-22, BH112-22, and BH117-22, loose, moist to wet, silt dominant soils were found underlying the silty sand soil, extending to depths of 1.4 mbeg to 2.9 mbeg. Moist to saturated, gravel and sand dominant



soils were found to extend from the base of the aforementioned soils to termination depth in all locations except BH107-22, TP121-22, TP137-22, and TP138-22. The gravel and sand was found to have a dense to very dense relative density. Bedrock was encountered at depths ranging from 1.5 mbgs to 4.88 mbgs in TP119-22, TP127-22, and TP134-22 to TP138-22, and all boreholes, except BH101-22 and BH103-22. For a more detailed description of the subsurface investigation, see the companion geotechnical investigation report (Cambium, 2022a).

## 3.1.1 Grain Size Analyses

Laboratory particle size distribution analyses were completed on eight (8) samples of the native soil taken from the boreholes and depths shown in Table 1. The grain size distribution results are provided in Appendix C.

Table 1 Grain Size Analysis Results

Borehole / Test Pit	Depth (mbgs)	Soil	% Gravel	% Sand	% Silt	% Clay
BH102-22 SS4	2.3 - 2.7	Silt some Clay trace Sand	0	3	86	11
BH104-22 SS3	1.5 – 2.0	Sand and Silt some Clay some Gravel	10	42	35	13
BH105-22 SS3	1.5 – 2.0	Sandy Silt	0	28	72	
BH107-22 SS3	1.5 – 2.0	Sandy Silt	0	28	72	
BH109-22 SS3	1.5 – 2.0	Gravelly Sand and Silt	21	40	39	
BH111-22 SS2	0.8 – 1.2	Gravelly Silty Sand trace Clay	27	44	24	5
BH115-22 SS3	1.5 – 2.0	Gravelly Silty Sand	34	46	20	
BH117-22 SS3	1.5 – 2.0	Gravelly Sand some Silt	20	65	15	

#### 3.2 Water Well Records

The Ministry of Environment Conservation and Parks (MECP) Water Well Information System (WWIS) was accessed to review water well records in the area of the Site. There were 32 water well records located within 500 m of the Site (Appendix G; Figure 2). The following water well record well types were identified:

Nineteen (19) well records were for water supply wells



- Four (4) well abandonment records were for water supply wells
- Eight (8) well abandonment records were for monitoring wells or test holes
- One (1) well record was for an unfinished well

As per the MECP records, topsoil was an average thickness of 1.0 m (when observed), topsoil was generally underlain by sand with varying gravel, silt, and clay components. Several well records recorded clay dominant and/or gravel dominant units as well. A bedrock contact was reported in 21 well records and ranged in depth between 1 mbgs and 38 mbgs (average of 13.5 mbgs). Bedrock was described as interbedded limestone and shale. The maximum depth advanced was 51.8 mbgs within bedrock. Water bearing sediments were identified within overburden between 7.6 to 26.5 mbgs, average of 12.4 mbgs. Water bearing fractures were identified in bedrock between 5.8 to 51.8 mbgs, average of 18.2 mbgs.

Static water level of overburden supply wells was an average of 6.2 mbgs; static water level of bedrock supply wells was an average of 8.2 mbgs. The recommended flow rate ranged from 0.6 litres per minute (lpm) to 364 lpm, averaging 44 lpm. Further information summarized from the water well records are listed below in Table 2.



Table 2 Water Well Record Information

		Total Depth (mbgs)	Depth Water Encountered (m)	Static Water Level (mbgs)	Recommended Pumping Rate (lpm)
Overburden	Min	7.9	7.6	2.0	23
Supply Well	Max	12.8	12.8	10.0	23
Count: 5	Avg.	11.0	10.5	6.2	23
Bedrock	Min	6.4	5.8	3.0	0.6
Supply Well	Max	51.8	51.8	29.0	36
Count: 15	Avg.	22.6	18.2	8.2	19
Monitoring	Min	4.6	7.6	3.0	364
Wells / Test Holes	Max	27.1	26.5	23.0	364
Count: 8	Avg.	10.4	12.4	13.0	364

Four water well records were plotted within the Site boundaries (5100148, 5100149, 5115821, and 7110601). These records were for wells that were installed to depths that ranged from 12.2 to 42.7 mbgs. In general, the overburden was described as sandy clay with boulders; however, one well record also described purely gravel units. The bedrock contact was found between 5.5 to 26.8 mbgs and bedrock was described as limestone or shale.

Well record 5115821 reported the deepest bedrock contact at 26.8 mbgs. This record is likely not installed within the property boundaries (or near the Site) as per the sketch provided in the record. The sketch indicates this record was installed north of Highway 7.

Well records 5100148 and 5100149 likely detail well installations on properties located adjacent the Site to the south on Mill Street (according to the provided sketches), and not within the property boundaries. The results of the well survey (and other available information) indicate that most of the residences along Mill Street are provided water by the Township. See section 4.3. As such, the wells described by records 5100148 and 5100149 may have been decommissioned, or are no longer in use.

Well record 7110601 is presumably for the on-site supply well. Cambium staff located the on-site supply well during the May 5, 2022 Site visit. The depth of the well was measured to be 43.60 mbgs and a water level 6.31 mbgs. The Universal Transverse Mercator coordinates of



the well are 263363 mE and 4919138 mN. The location of the existing well is outlined on Figure 1. No other supply wells were observed on-site by Cambium staff. Any wells not intended for use on the Site should be decommissioned according to Ontario Regulation 903.

## 3.3 Vulnerable and Regulated Areas

As per the MECP Source Water Protection Information Atlas (SPIA), the Site is located within the following areas:

- Highly Vulnerable Aquifer (HVA)
- Significant Groundwater Recharge Area (SGRA) with a vulnerability score of 6
- Wellhead Protection Areas B and C (WHPA-B/C)

The HVA covers the entirety of the Site, the SGRA covers the majority of the central and eastern portions of the Site, and the WHPAs B and C fall just within the western boundary of the Site. The SPIA and mapping is attached in Appendix A.

As Cambium understands, the Otonabee Region Conservation Authority (ORCA) has issued a Restricted Land Use Notice (Otonabee Region Conservation Authority, 2023) for the proposed development (as per Part IV the Clean Water Act, 2006, Section 59 (2) (a)). The Restricted Land Use Notice indicates that there is no prohibition or risk management plan required for the proposed development.

It is noted, the Restricted Land Use Notice also states the following:

This Notice does not reflect an assessment of significant drinking water threats where the Risk Management Official is not responsible for implementing the Trent Source Protection Plan policy, including sewage and stormwater related activities, low or moderate drinking water threats or Transport Pathways.

Discussions regarding the proposed stormwater management plan should be held between the Client and regulatory authorities, as required.



## 3.4 Hydrogeological Conditions

Findings from the geotechnical investigation indicate that overburden soils consist primarily of silty sand, underlain by silt with some sand and clay, underlain by gravel and sand dominated soils to borehole and test pit termination ranging between 1.50 and 5.03 mbgs.

An unconfined aquifer exists within the shallow overburden and is the aquifer which the monitoring wells are installed within. According to the MECP WWIS, there is assumed to be at least two bedrock aquifers present in the area of the Site. The first bedrock aquifer is generally within 25 mbgs and was the most commonly drawn upon aquifer for the bedrock water supply wells; one well record indicated a bedrock aquifer at 41.2 that is assumed that this is a separate, deeper bedrock aquifer. The connectivity of the shallow overburden aquifer and the deeper aquifer systems is not known. However, it is likely that there is some degree of hydraulic connection between the shallow overburden aquifer and the underlying bedrock aquifer.

On April 27 and May 5, 2022 water levels measured from the on-site monitoring wells ranged from 0.88 mbgs and 2.72 mbgs (this range does not include the April 20, 21, 23, and 24, results since the wells were installed on these days and as such the water levels were not representative of static conditions). Well BH104-22 was reported as dry during both monitoring events. Groundwater elevations ranged from 199.74 masl to 208.93 masl during the two measurement events. The water levels referenced herein were near their annual average high conditions, since the measurement events occurred in late April and early May.

In general, groundwater flow of the shallow overburden aquifer is southwest, where it is assumed to discharge to surface into the Ouse River, located adjacent to the Site.

Groundwater flow contours are outlined on Figure 1. Groundwater levels and elevations are outlined below in Table 3.



Table 3 Groundwater Level and Elevation Information

Well		BH104-22	BH105-22	BH112-22	BH115-22
Top of F	Pipe Elevation (masl) <sup>(1)</sup>	210.06	203.70	210.42	212.09
Ground (masl) <sup>(1</sup>	Surface Elevation	208.93	202.46	209.28	210.83
Stick-up	(m)	1.13	1.24	1.14	1.25
April	Water Level (mbgs) (2)	Dry	2.68	0.88	1.91
27, 2022	Groundwater Elev.(masl) <sup>(1)</sup>	-	199.78	208.40	208.93
May	Water Level (mbgs) (2)	Dry	2.72	1.24	2.02
May 4, 2022	Groundwater Elev.(masl) <sup>(1)</sup>	-	199.74	208.04	208.82

<sup>1.</sup> Metres above sea level

As Cambium understands, the proposed development will be provided water and wastewater servicing by the Township. Construction dewatering, and potentially operational dewatering, for the development may be required, depending on building and infrastructure depths.

Preliminary dewatering estimates are provided in Section 4.3.

<sup>2.</sup> metres below ground surface



#### 4.0 Assessment Results

This section presents the results of the SWHTs and groundwater quality analyses.

## 4.1 Single Well Hydraulic Tests

SWHTs were completed on wells BH105-22, BH112-22, and BH115-22 on May 5, 2022. The data generated from the bail tests was processed by AquiferTest Pro™ software, the results of which are summarized in Table 4.

The hydraulic conductivity results of the SWHTs ranged between 4.88 x 10<sup>-8</sup> m/s and 5.95 x 10<sup>-6</sup> m/s. The hydraulic conductivity of the tested soils for BH105-22, BH112-22, and BH115-22 were in agreement with those published in literature for silty sand and sandy silts (Fetter, 2001) (J.P.Powers, 2007). The borehole log of BH112-22 indicated that this well was screened across various types of soil, including silt. The variability of the soils could account for the lower hydraulic conductivity reported from this well. Hydraulic testing results are attached in Appendix F.

Table 4 SWHT Results (all results in m/s)

Test #	BH105-22	BH112-22	BH115-22
Test 1	1.61 x 10 <sup>-6</sup>	7.35 x 10 <sup>-8</sup>	5.95 x 10 <sup>-6</sup>
Test 2	2.79 x 10 <sup>-6</sup>	7.32 x 10 <sup>-8</sup>	2.64 x 10 <sup>-6</sup>
Test 3	3.00 x 10 <sup>-6</sup>	4.88 x 10 <sup>-8</sup>	3.25 x 10 <sup>-6</sup>
Test 4	3.50 x 10 <sup>-6</sup>	-	2.17 x 10 <sup>-6</sup>

## 4.2 Groundwater Quality

Groundwater samples were collected from monitoring wells BH105-22 and BH115-22 on May 4, 2022 and analyzed for general organic and inorganic parameters. The groundwater samples were collected with inertial lift footvalve and were unfiltered. The water quality results were then compared against the PWQO (MOE, 1994).

The total fraction of several metals were reported at concentrations greater than PWQO criteria. Further, the dissolved fraction of aluminum and iron were reported at concentrations greater than PWQO criteria at both wells. The concentration of dissolved phosphorus was



reported at a concentration greater than PWQO criteria only at well BH105-22. Parameter exceedances are outlined below in Table 5. Results are attached in Appendix E.

Table 5 Groundwater Quality

Table 5 Groundwate	er Quanty	
Parameter	Concentration – BH105-22 (mg/L)	PWQO Criteria (mg/L)
total aluminum	9.52	0.075
dissolved aluminum	0.241	0.075
total cobalt	0.00543	0.0009
total copper	0.0109	0.005 (1)
total iron	6.89	0.3
dissolved iron	0.235	0.3
total lead	0.00883	0.005 (2)
total nickel	0.0093	0.0025
total phosphorus	0.326	0.03 (3)
dissolved phosphorus	0.018	0.03 (3)
total vanadium	0.0145	0.006
total zinc	0.026	0.02
total zirconium	0.007	0.004
Parameter	Concentration – BH115-22 (mg/L)	PWQO Criteria (mg/L)
total aluminum	72.4	0.075
dissolved aluminum	0.187	0.075
total cobalt	0.00164	0.0009
total iron	2.82	0.3
dissolved iron	0.177	0.3
total nickel	0.0030	0.0025
total phosphorus	0.132	0.03 (3)

- $1. \quad \textit{Based off average regional Southern Ontario concentrations, hardness is assumed to be \verb|>20 mg/L| \\$
- 2. Based off average regional Southern Ontario concentrations, alkalinity is assumed to be >80 mg/L
- 3. PWQO criteria to avoid excessive plant growth in rivers and streams

The total fraction of most metals reported at elevated concentrations is likely a result of suspended sediments being captured in the samples. The suspended sediments were introduced into the samples by the nature of the sampling methodology (i.e., non-filtered samples and mechanical disruption of sediments from the inertial lift footvalve). Generally,



groundwater samples are not filtered in order to simulate water quality that would be dewatered from a construction excavation. The total fraction of metals can be reduced by filtering discharge water to removed suspended sediments (if required).

Dewatering during construction activities may be required (dewatering estimates are outlined in Section 5.0). The fate of dewatering discharge water was not known at the time this document was prepared. However, the Client indicates that dewatering discharge water will likely be managed wholly within the property boundaries and allowed to re-infiltrate into the subsurface. Is this scenario dewatering discharge water will not runoff the Site and treatment of discharge water before re-infiltration is not considered necessary.

If discharge water is to be disposed to the subsurface on-site then the sensitivity of those groundwater users located hydraulically down-gradient the Site should be reviewed at a later date (but prior to dewatering activities). As per Figure 1 groundwater flow across the Site is to the southwest. The water supply of those properties located southwest have not been confirmed. However, as outlined in Section 4.3, the Township of Asphodel-Norwood indicated that municipal water is provided to all residents on Mill St. between Queen Street and 108 Mill St. It is noted that the concentrations of dissolved manganese (0.107 mg/L and 0.5963 mg/L), dissolved aluminum (0.241 mg/L and 0.187 mg/L) and nitrate (23.4 mgL and 18.9 mg/L) were reported at concentrations greater than the Ontario Drinking Water Quality Standards (ODWQS) (Ministry of Environment Conservation and Parks, 2020). Manganese and aluminum are not health related parameters and only have an aesthetic/operational limit under the ODWQS (being 0.05 mg/L and 0.1 mg/L, respectively). Only nitrate is a health related parameter, and has a maximum allowable concentration of 10 mg/L, as per the ODWQS. The parameter exceedances of the ODWQS listed above are representative of background conditions within the shallow overburden aquifer. If the shallow overburden aquifer is dewatered during construction activities (on a temporary basis), and discharge water is disposed on-site through re-infiltration efforts, then no significant degradation of groundwater quality off-site is expected to occur (because the water being re-infiltrated is from the same overburden aquifer it was drawn from). Regardless, the sensitivity of those groundwater users located hydraulically down-gradient the Site should be reviewed prior to construction. Potential



groundwater users located down-gradient the Site are those properties located at 2450 Asphodel 10<sup>th</sup> Line, 24 Mill Street, 26 Mill Street and 28 Mill Street.

## 4.3 Well Survey Results

A letter mailing campaign was initiated on April 18, 2022 in order to complete a water well survey of properties adjacent the Site. A total of 19 residences were contacted during the survey. Cambium did not receive any responses to the April 18 letter mailing campaign, so a follow up mailing campaign was completed on May 19, 2022. It is noted that two additional properties were included in the May 19, 2022 letter mailing campaign. The list of addresses contacted during the letter mailing campaigns is included in Appendix D. copies of the letter and water well questionnaire sent to property owners are included in Appendix D.

One landowner responded to the May 19, 2022 letter mailing campaign. Details provided by the respondent is listed below:

 90 Mill Street – This property is serviced by town water and mentions that most, if not all, residents on Mill Street are serviced by Township water. This property does have a private septic system.

On August 17, 2022 the Client confirmed with the Township of Asphodel-Norwood that municipal water is provided to all residents on Mill St. between Queen Street and 108 Mill St.



## 5.0 Preliminary Dewatering Estimates

The proposed development will include 640 residential units. It is assumed that these units will have associated basements. Details regarding basements were not available at the time this document was prepared. However, as Cambium understands the Client intends on maintaining basements above the water table as much as possible. As such, dewatering estimates for basement excavations are not included herein.

The proposed development will include on-site servicing for water and sewer. Details pertaining to the construction of on-site services were not finalized at the time this document was prepared. Construction dewatering estimates for on-site servicing were based upon assumed depths and footprints of these excavations (during construction). Construction dewatering estimates for basements and on-site servicing should be reviewed/finalized at a later date, once detailed development plans are available for review.

On-site servicing was assumed to be built within trench excavations advanced at lengths not greater than 50 m, at any one time. It was assumed that the trench will be advanced to a depth of 3 mbgs for servicing, and the water level would be lowered to 3.5 mbgs to allow for safe working conditions.

The water level during construction dewatering was assumed to be 0.88 mbgs based off the shallowest groundwater reading measured on April 27, 2022 at BH112-22.

It was assumed that the trench would be advanced within overburden. The hydraulic conductivity of overburden soils was assumed to range from  $4.88 \times 10^{-8}$  m/s and  $5.95 \times 10^{-6}$  m/s (as per the SWHT results). The hydraulic conductivity rates were used to provide a range of construction dewatering estimates.

To calculate inflow into the excavation, the methods outlined in the Construction Dewatering and Groundwater Control (J.P.Powers, 2007) were utilized.

The preliminary construction dewatering rate estimates for the servicing trench excavations range from approximately 4 m³/day to 47 m³/day. Due to the preliminary nature of the



construction dewatering estimates a factor of safety was not applied to the results. Dewatering estimate calculations are included in Appendix H.

Based off the preliminary dewatering estimates for the trench excavations, and the assumptions outlined herein, the estimated daily dewatering rates for a trench excavation may be less than 50 m³/day (50,000 L/day). If actual construction dewatering rates are less than 50 m³/day then water taking permitting may not be required. If actual daily dewatering rates exceed 50,000 L/day (but remain less than 400,000 L/day) then dewatering activities are required to be registered on the Environmental Activity and Sector Registry (EASR). If actual daily dewatering rates exceed 400,000 L/day then a Category 3 Permit To Take Water (PTTW) will be required for water taking activities. Due to preliminary nature of these calculations, dewatering estimates should be reassessed when more details regarding the proposed development plans are available for review (i.e. excavation depth/footprint, etc.). The revised dewatering estimates should include a factor of safety. Appropriate water taking permitting should be acquired (as needed) once construction dewatering estimates have been revised.

It is noted that the daily dewatering estimates outlined herein model the daily inflow rates into construction excavations once equilibrium of groundwater inflow into the excavation has been achieved. The initial dewatering rate during groundwater inflow stabilization period (prior to equilibrium) is not included in the dewatering estimates. Further, the initial dewatering efforts of an open excavation (i.e., dewatering a construction excavation after it has been allowed to fill with groundwater over a prolonged period of time) are also not included in the dewatering calculations.

It is imperative that during construction daily dewatering rates are monitored (or the dewatering of stagnant water in the construction excavation estimated) to determine appropriate water taking permitting / registration requirements.

Further, the method of disposal of water removed from the construction excavation should be determined prior to the commencement of water withdrawal.



## 5.1 Operational Dewatering

Cambium recommends that operational dewatering estimates are confirmed at a later stage of development once detailed development plans are available for review.

#### 5.1.1 Zone of Influence

The dewatering calculations include estimates of the horizontal distance away from the walls of each excavation where the influence of water withdrawal will be negligible (i.e., the length to zero drawdown (Kyrieleis, W., Sichardt, W, 1930). The area included within the length of zero drawdown from the excavation is the zone of influence (ZOI). The maximum length to zero drawdown (as determined from the highest hydraulic conductivity) for the trench excavation was calculated at 19 m (see Figure 3; Appendix H). The orientation of on-site servicing was not known at the time this document was prepared, as such the length to zero drawdown was applied to the property boundaries as a conservative measure.

Construction dewatering influences of the proposed development intercept several adjacent private properties. The properties captured within the ZOI are listed below:

- 2370 Asphodel 10<sup>th</sup> Line
- 2366 Asphodel 10<sup>th</sup> Line
- Assessment Roll # 15010100031510000000 (located at the corner of Mill St., and 2319 Asphodel 10<sup>th</sup> Line)
- Assessment Roll # 15010100031501400000
- 78 Mill Street
- 76 Mill Street
- 36 Mill Street
- 28 Mill Street
- 26 Mill Street
- Railway Property



As per Section 4.3, the Township of Asphodel-Norwood indicated that the properties on Mill St. between Queen Street and 108 Mill Street are provided water by municipal services.

Of the properties captured within the ZOI, it is likely that only 2370 and 2366 Asphodel 10<sup>th</sup> Line are provided water from a private on-site supply well. The ZOI encompasses a railway line adjacent the Site to the northwest.

Once detailed development plans are finalized the ZOI for dewatering activities should be recalculated. If the properties at 2370 and 2366 Asphodel 10<sup>th</sup> Line (or any other property provided water with a private supply well) are captured within the revised ZOI then further attempts to contact the owners of these properties should be made, and their private supply wells investigated. A private supply well monitoring program should be completed as required, and should be based upon the results of private well investigations.

If the revised ZOI encompasses the adjacent railway line then consideration should be given to establishing land settlement monitoring program during construction dewatering activities.



#### 6.0 Conclusions and Recommendations

CAP Norwood Development Inc. (Client) retained Cambium Inc. to complete a hydrogeological assessment for the property located at 52 Mill Street in Norwood, Ontario.

Groundwater levels within the shallow overburden aquifer ranged from 0.88 to 2.72 mbgs in April and May of 2022 and flows to the southwest.

Based off the preliminary dewatering calculations for the trench excavations and the assumptions outlined herein, the estimated daily dewatering rates may be less than 50 m³/day (50,000 L/day). If daily construction dewatering rates exceed 50,000 L/day then dewatering activities are required to be registered on the Environmental Activity and Sector Registry. Cumulative daily construction dewatering rates should be monitored on a daily basis to confirm total water withdrawal rates across the Site during construction. The calculated ZOI encompasses several adjacent the Site, including a rail line.

Due to preliminary nature of the calculations outlined herein, the construction dewatering estimates and ZOI should be reviewed and revised once detailed development plan are available for review (i.e. regrading depth, orientation, footprint, etc.). The revised dewatering estimates should include a factor of safety. Appropriate water taking permitting should be acquired (as needed) once construction dewatering estimates have been revised.

If the revised ZOI encompasses residential properties provided water from a private supply well (such as 2370 and 2366 Asphodel 10<sup>th</sup> Line) and/or the adjacent rail line, then appropriate monitoring programs for these properties should be developed. It is possible that portions of the proposed development will require operational dewatering. The need and scope of ongoing monitoring of adjacent private supply wells should be determined at a later date.

It is expected that construction dewatering discharge will be managed wholly on-site (by allowing for water re-infiltration). Allowing for discharge water re-infiltration is not expected to influence off-site surface water features or down-gradient groundwater users. However, the sensitivity of those groundwater users located hydraulically down-gradient the Site should be reviewed prior to construction.



In summary, the proposed development is considered feasible at the Site. Future development plans should be reviewed and the dewatering impact assessment should be reviewed to determine the need to establish monitoring programs for adjacent properties.

Cambium Inc.

Nicole Heikoop, M.Sc., GIT

**Technologist** 

Cameron MacDougall, P.Geo.

**Project Manager** 

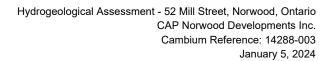
CM/nh/nr

P:\14200 to 14299\14288-003 CAP Norwood Dev - HydroG GEO ESA - 42 & 52 Mill St, Norwood\Deliverables\REPORT - HydroG\Final\2024-01-03 RPT HydroG Assessment, 52 Mill St, Norwood\docx



## 7.0 References

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- OGS. (2010). Surficial Geology of Southern Ontario, Miscellaneous Release Data 128-REV. Scale 1:50,000.
- Otonabee Region Conservation Authority. (2023). Restricted Land Use Notice 2023-0158-N2a, 52 Mill Street, Norwood, Township of Asphodel-Norwood, Roll No.: 150101000315300.





Hydrogeological Assessment - 52 Mill Street, Norwood, Ontario CAP Norwood Developments Inc.

Cambium Reference: 14288-003 January 5, 2024

## 8.0 Standard Limitations

#### **Limited Warranty**

In performing work on behalf of a client, Cambium relies on its client to provide instructions on the scope of its retainer and, on that basis, Cambium determines the precise nature of the work to be performed. Cambium undertakes all work in accordance with applicable accepted industry practices and standards. Unless required under local laws, other than as expressly stated herein, no other warranties or conditions, either expressed or implied, are made regarding the services, work or reports provided.

#### Reliance on Materials and Information

The findings and results presented in reports prepared by Cambium are based on the materials and information provided by the client to Cambium and on the facts, conditions and circumstances encountered by Cambium during the performance of the work requested by the client. In formulating its findings and results into a report, Cambium assumes that the information and materials provided by the client or obtained by Cambium from the client or otherwise are factual, accurate and represent a true depiction of the circumstances that exist. Cambium relies on its client to inform Cambium if there are changes to any such information and materials. Cambium does not review, analyze or attempt to verify the accuracy or completeness of the information or materials provided, or circumstances encountered, other than in accordance with applicable accepted industry practice. Cambium will not be responsible for matters arising from incomplete, incorrect or misleading information or from facts or circumstances that are not fully disclosed to or that are concealed from Cambium during the provision of services, work or reports.

Facts, conditions, information and circumstances may vary with time and locations and Cambium's work is based on a review of such matters as they existed at the particular time and location indicated in its reports. No assurance is made by Cambium that the facts, conditions, information, circumstances or any underlying assumptions made by Cambium in connection with the work performed will not change after the work is completed and a report is submitted. If any such changes occur or additional information is obtained, Cambium should be advised and requested to consider if the changes or additional information affect its findings or results.

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#### Site Assessments

A site assessment is created using data and information collected during the investigation of a site and based on conditions encountered at the time and particular locations at which fieldwork is conducted. The information, sample results and data collected represent the conditions only at the specific times at which and at those specific locations from which the information, samples and data were obtained and the information, sample results and data may vary at other locations and times. To the extent that Cambium's work or report considers any locations or times other than those from which information, sample results and data was specifically received, the work or report is based on a reasonable extrapolation from such information, sample results and data but the actual conditions encountered may vary from those extrapolations.

Only conditions at the site and locations chosen for study by the client are evaluated; no adjacent or other properties are evaluated unless specifically requested by the client. Any physical or other aspects of the site chosen for study by the client, or any other matter not specifically addressed in a report prepared by Cambium, are beyond the scope of the work performed by Cambium and such matters have not been investigated or addressed.

#### Reliance

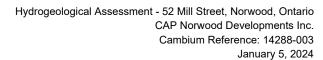
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#### **Limitation of Liability**

Potential liability to the client arising out of the report is limited to the amount of Cambium's professional liability insurance coverage. Cambium shall only be liable for direct damages to the extent caused by Cambium's negligence and/or breach of contract. Cambium shall not be liable for consequential damages.

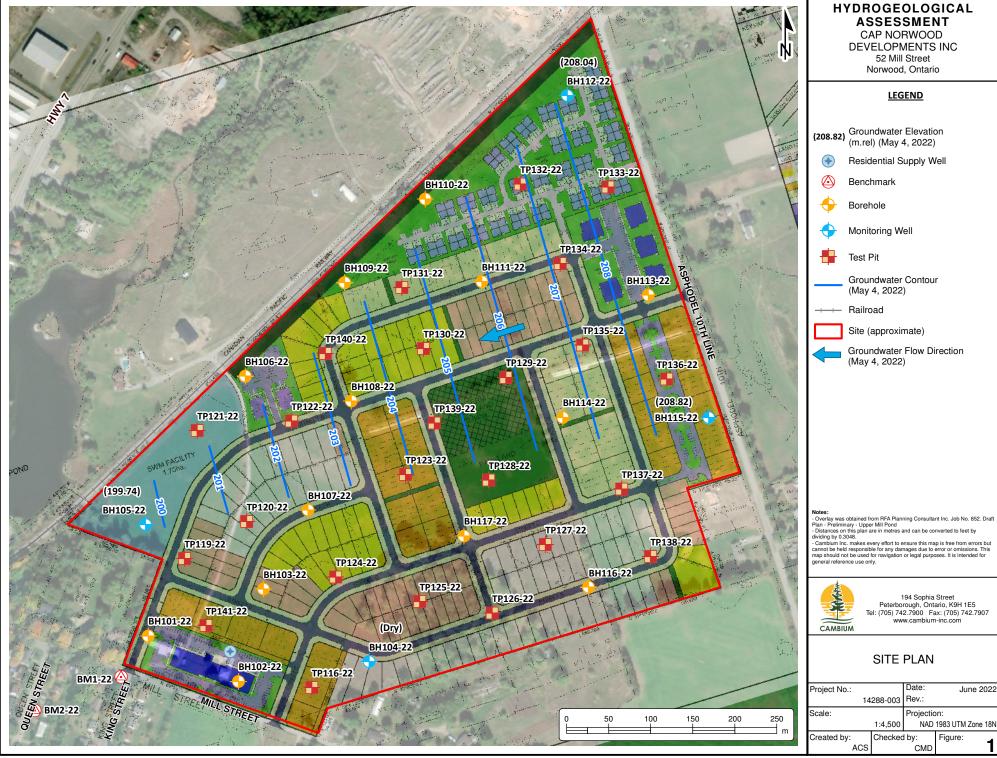
#### Personal Liability

The client expressly agrees that Cambium employees shall have no personal liability to the client with respect to a claim, whether in contract, tort and/or other cause of action in law. Furthermore, the client agrees that it will bring no proceedings nor take any action in any court of law against Cambium employees in their personal capacity.





		Appended Figures
		Appended Figures



DYGISWADSY 4200-14299/14288-003 CAP Norwood Dev - HydroG GEO ESA - 42 & 52 Mill SY, Norwood/2023-11-13 Hydro G FK3 1 - Borenale Location Pl

#### **GEOTECHNICAL INVESTIGATION**

CAP NORWOOD **DEVELOPMENTS INC** 52 Mill Street Norwood, Ontario

#### **LEGEND**

MECP Water Well Record 500m Study Area

Site (approximate)

Notes:

- Base mapping features are @ Queen's Printer of Ontario, 2019 (this does not constitute an endorsement by the Ministry of Natural Resources or the Ontario Government).

- Distances on this plan are in metres and can be converted to feet by dividing by 0.3048.

- Cambium Inc. makes every effort to ensure this map is free from errors but cannot be held responsible for any damages due to error or omissions. This map should not be used for navigation or legal purposes. It is intended for general reference use only.



194 Sophia Street Peterborough, Ontario, K9H 1E5 Tel: (705) 742.7900 Fax: (705) 742.7907

#### MECP WELL RECORDS

Project No.: June 2022 14288-003 Rev.: Scale: Projection: 1:10,000 NAD 1983 UTM Zone 18N Created by: Checked by: ACS CMD

#### **GEOTECHNICAL INVESTIGATION**

CAP NORWOOD DEVELOPMENTS INC 52 Mill Street Norwood, Ontario

#### **LEGEND**

MECP Water Well Record



19m Study Area



Site (approximate)

Notes:

- Base mapping features are @ Queen's Printer of Ontario, 2019 (this does not constitute an endorsement by the Ministry of Natural Resources or the Ontario Government).

- Distances on this plan are in metres and can be converted to feet by dividing by 0.3048.

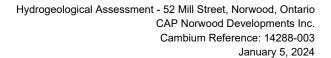
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194 Sophia Street Peterborough, Ontario, K9H 1E5 Tel: (705) 742.7900 \_ Fax: (705) 742.7907

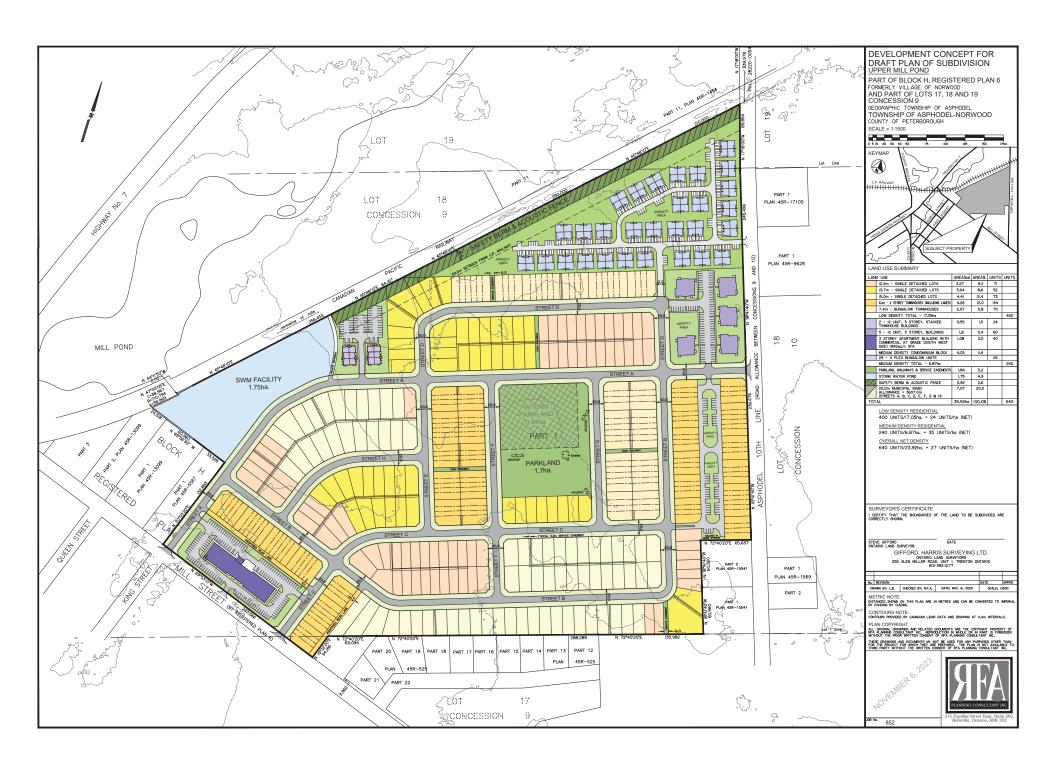
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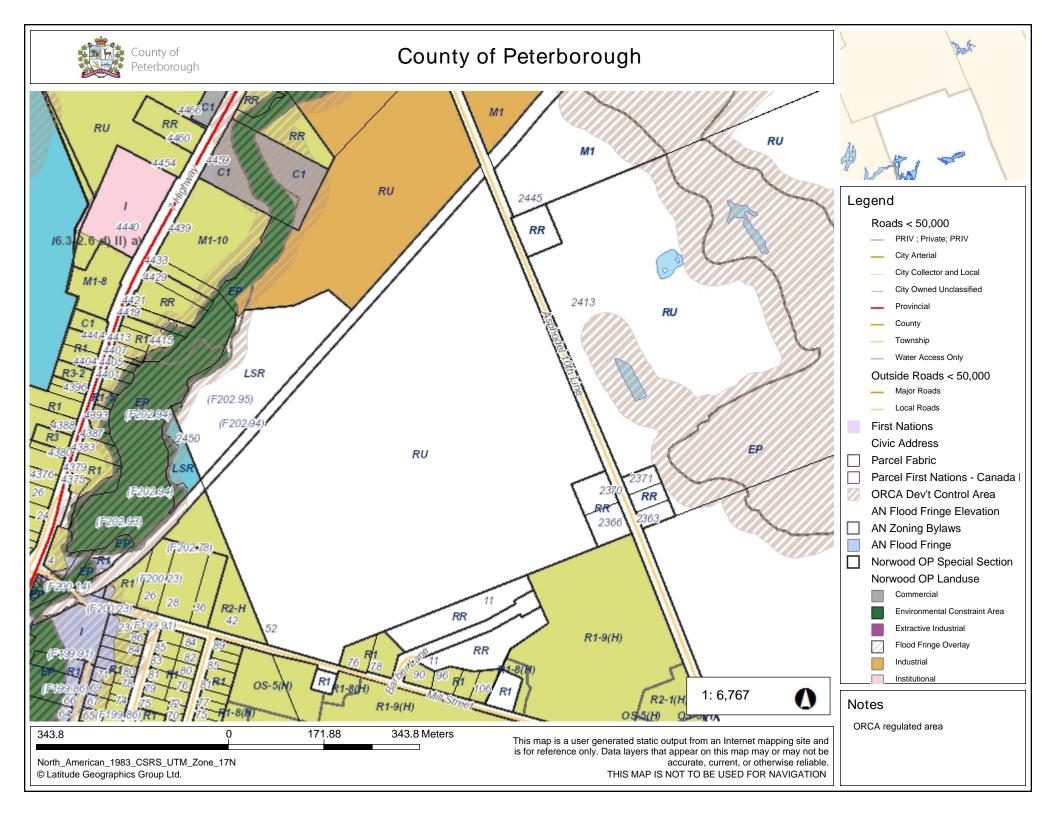
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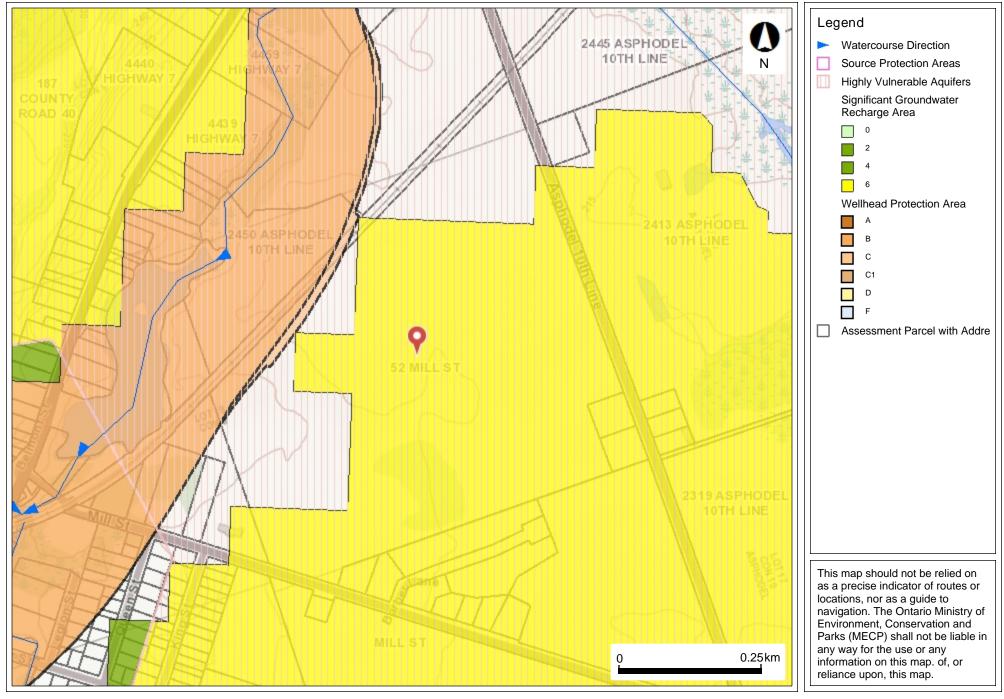


Appendix A Land Information and Proposed Development Plan





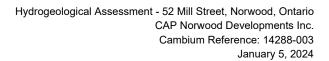
# SPIA Map





Map Created: 6/8/2022

Map Center: 44.38971 N, -77.96814 W





	Appendix B
Borehole and	<b>Test Pit Logs</b>



Log of Borehole:

BH101-22

Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Client:CAP Norwood DevProject Name:42 & 52 Mill Street, NorwoodProject No.:14288-003Contractor:Canadian EnvironmentalMethod:Solid Stem AugerDate Completed:April 20, 2022

 Location:
 42 Mill Street, Norwood
 UTM:
 18T 263264.8937 E, 4919157.519 N
 Elevation:
 204.434 masl

	;	SUBSU	RFACE PROFILE				SAN	IPLE			
Elevation	(m) Depth	Lithology	Description	Number	Туре	% Recovery	SPT (N) / DCPT	/ (N) LdS		Well Installation	Remarks
205 - 204 -			TOPSOIL: 150mm thick topsoil: Dark brown, silt, trace sand, trace organics, moist, loose	1A 1B 1C	SS SS SS	85	7				
203 -			SANDY SILT: Brown, sandy silt, trace gravel, moist, loose  SILTY SAND: Brown, silty sand, some gravel, moist, loose  SAND AND SILT: Light brown, sand and silt, some gravel, trace clay, moist to wet, compact  GRAVELLY SAND: Light brown, gravelly silty sand, some gravel, trace	2	SS	100	11				
202 -	- <del></del>		clay, moist, compact  SAND AND SILT: Light brown/grey, gravelly sand and silt, trace clay, dry to moist, compact	4	SS	100	29	•			Borehole open and dry upon completion
201 –	3   		-becomes dense	5	SS	100	46				
200 -	_ <b>-4</b>  										Cobbles
199 –	 _ <b>5</b>  		-becomes moist  Borehole terminated at 5.03 mbgs in gravelly sand and silt	6	SS	100	34				
	6										



Log of Borehole:

BH102-22

Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Project Name: Project No.: Client: **CAP Norwood Dev** 42 & 52 Mill Street, Norwood 14288-003 Contractor: Canadian Environmental Method: Solid Stem Auger Date Completed: April 20, 2022 Location: 42 Mill Street, Norwood UTM: 18T 263372.1357 E, 4919103.341 N Elevation: 205.325 masl

SU	JBSUI	RFACE PROFILE				SAM	IPLE			
Elevation (m) Depth	Lithology	Description	Number	Type	% Recovery	SPT (N) / DCPT	% Moisture	/(N) LdS 030 40	Well Installation	Remarks
206		TOPSOIL: 300mm thick topsoil: Dark brown, silt, trace sand, trace organics, moist, loose  SILTY SAND: Brown, silty sand, trace clay, moist, loose  SILT: Brown, silt, some clay, trace sand, moist to wet, loose  GRAVELLY SAND: Brown, gravelly silty sand, trace clay, moist to wet, very dense  Borehole terminated at 3.05 mbgs on presumed bedrock	1A 1B 2 3A 3B	SS SS SS SS	75 80 100	4 6 5 7 50/ 50				Groundwater first encountered at 1.52 mbgs  SS4 GSA: 0% gravel 3% sand 86% silt 11% clay Water level upon completion at 3.05 mbgs  Borehole open upon completion
<del> </del>										



Log of Borehole:

BH103-22

Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Project Name: Project No.: Client: **CAP Norwood Dev** 42 & 52 Mill Street, Norwood 14288-003 Contractor: Canadian Environmental Method: Solid Stem Auger Date Completed: April 20, 2022 Location: 42 Mill Street, Norwood UTM: 18T 263401.7241 E, 4919213.468 N Elevation: 206.096 masl

	•	SUBSU	RFACE PROFILE				SAN	IPLE			
Elevation	(m) Depth	Lithology	Description	Number	Type	% Recovery	SPT (N) / DCPT	- 55 Woisture	/(N) LdS O 40	Well Installation	Remarks
- 207 — -	- 										
- 206 — - -	- - - - - - - -		TOPSOIL: 100mm thick topsoil: Dark brown, silt, trace sand, trace organics, moist, loose SILTY SAND: Brown, silty sand, trace	1A 1B	SS SS	50	5	] ]			
- 205 —	-  -  1  -		clay, trace organics, moist, loose -no organics	2	SS	80	4		•		
- - 204 — -	2		SILT: Brown, silt, some clay, trace sand, moist to wet, loose  SILT AND SAND: Light brown, silt and sand, trace clay, moist to wet,	3	SS	100	5				
- - 203 —			compact  SILT AND SAND: Light brown, gravelly silt and sand, trace clay, moist, dense	5	SS	100	19				Borehole open and dry upon completion
- - 202 — -	- <b>4</b>		-becomes grey  SILT AND SAND: Grey, gravelly silt and sand, trace clay, dry to moist,								
- - 201 —	- - - - - - - - -		dense  Borehole terminated at 5.03 mbgs in	6	SS	80	46				
- - 200 —	- - - - 6		gravelly silt and sand								



Log of Borehole:

BH104-22 Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Project Name: Project No.: Client: CAP Norwood Dev 42 & 52 Mill Street, Norwood 14288-003 Date Completed: Contractor: Canadian Environmental Method: Solid Stem Auger April 20, 2022 UTM: Location: 42 Mill Street, Norwood 18T 263527.4327 E, 4919126.817 N Elevation: 208.934 masl

SUBSURFACE PROFILE **SAMPLE** DCPT Moisture SPT (N) / DCPT Recovery  $\hat{z}$ Lithology Number (m) Depth Well % SPT Description Remarks Installation 25 50 75 10 20 30 40 Сар 209 TOPSOIL: 600mm thick topsoil: Dark brown, silt, trace sand, trace Pipe 1 SS 50 4 organics, moist, loose Bentonite Plug GRAVELLY SAND: Light brown/grey, gravelly silty sand, trace clay, moist, 208 2 43 No water level SS 60 dense detected when measured on May 4, 2022 SAND AND SILT: Grey, sand and silt, some clay, some gravel, moist, SS3 GSA: compact 10% gravel 3 SS 100 15 42% sand 207 35% silt 13%clay SAND AND SILT: Grey, gravelly sand Sand Pack and silt, some clay, moist, dense PVC 4 SS 100 46 Borehole open and Screen dry upon completion 206 50/ 100 Сар 100 Borehole terminated at 3.2 mbgs on presumed bedrock 205 204



Log of Borehole:

BH105-22 Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Client: CAP Norwood Dev Project Name: 42 & 52 Mill Street, Norwood Project No.: 14288-003 Contractor: Canadian Environmental Method: Solid Stem Auger Date Completed: April 21, 2022 UTM: Location: 42 Mill Street, Norwood 18T 263261.4148 E, 4919290.377 N Elevation: 202.457 masl

SUBSURFACE PROFILE **SAMPLE** DCPT Moisture SPT (N) / DCPT Recovery  $\hat{z}$ Lithology Number % Well SPT Œ Description Installation Remarks 25 50 75 10 20 30 40 Cap 203 Pipe TOPSOIL: 225 mm thick topsoil: Dark 1A SS brown, silt, trace sand, trace 75 4 SS Bentonite organics, moist, loose 1B 202 Plug SANDY SILT: Brown, sandy silt, trace clay, moist, loose SANDY SILT: Light brown, sandy silt, 2 SS 100 4 trace clay, moist to wet, loose Groundwater first encountered at 1.52 mbgs 3 SS 90 3 SS3 GSA: -becomes very loose, wet 0% gravel 28% sand 72% silt & clay SS -becomes compact 200 Sand Pack 4B SS 80 37 Water level SAND AND SILT: Grey, gravelly sand PVC measured at 2.72 and silt, moist to wet, dense Screen mbgs on May 4, 2022 Water level upon 5 SS 40 29 completion at 3.05 -becomes saturated mbgs 199 Borehole caving Cap occurred up to 4.11 mbgs upon 198 completion Borehole terminated at 4.42 mbgs on presumed bedrock 197



Peterborough Barrie Oshawa Kingston T: 866-217-7900

Log of Borehole:

BH106-22

Page 1 of 1

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Project Name: Project No.: Client: CAP Norwood Dev 42 & 52 Mill Street, Norwood 14288-003 Method: Date Completed: Contractor: Canadian Environmental Solid Stem Auger April 21, 2022 UTM: 18T 263380.2677 E, 4919466.901 N Location: 42 Mill Street, Norwood Elevation: 208.343 masl

SUBSURFACE PROFILE **SAMPLE** (N) / DCPT Moisture SPT (N) / DCPT Recovery Lithology Number Well % SPT (E) Description Remarks Installation 25 50 75 10 20 30 40 209 TOPSOIL: 300mm thick topsoil: Dark SS brown, silt, trace sand, trace 208 75 4 organics, moist, loose SANDY SILT: Brown, sandy silt, trace clay, moist, loose 2A SS 16 80 2B SAND AND SILT: Light brown, gravelly sand and silt, moist, compact 207 3 SS 80 41 -becomes dense 206 4 SS 60 54 -becomes dry to moist, very dense 50/ 5 SS 50 205 425 Borehole open and dry upon completion 204 100 50 Borehole terminated at 4.57 mbgs on presumed bedrock 203



Log of Borehole:

BH107-22 Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Project Name: Project No.: Client: **CAP Norwood Dev** 42 & 52 Mill Street, Norwood 14288-003 Contractor: Canadian Environmental Method: Solid Stem Auger Date Completed: April 21, 2022 Location: 42 Mill Street, Norwood UTM: 18T 263454.8087 E, 4919307.502 N Elevation: 204.314 masl

	,	SUBSU	RFACE PROFILE				SAN	PLE			
Elevation	(m) Depth	Lithology	Description	Number	Туре	% Recovery	SPT (N) / DCPT	- 55 % Moisture	/ (N) / (S S D C N) / (O S D C D C D C D C D C D C D C D C D C D	Well Installation	Remarks
205 —	- 										
204 —	0 	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	TOPSOIL: 300mm thick topsoil: Dark brown, silt, trace sand, trace organics, moist, very loose  SANDY SILT: Brown, sandy silt, trace	1A 1B	SS SS	85	3				
203 —	_ _ 1 		clay, moist, very loose  SILT: Light brown, silt, some sand, some clay, moist to wet, loose	2A 2B	SS SS	100	8		$\left  \left\langle \cdot \right  \right $		
202 —	- - - - - - - -		SANDY SILT: Light brown, sandy silt, trace clay, moist to wet, compact	3	SS	100	18				SS3 GSA: 0% gravel 28% sand 72% silt & clay
- - - - 201 –			-sand becomes coarser	5	SS	100	21				Borehole open upon completion
- - - - 200 –	- - - - - - - -						<del>- 50/</del> -				Groundwater first encountered at 4.42
199 —	- - - - 5 - -	•••	-trace gravel, wet, very dense  Borehole terminated at 4.42 mbgs on presumed bedrock	6	SS	100	75				mbgs Water level upon completion at 4.42 mbgs
	- - - - - 6										



Log of Borehole:

BH108-22 Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Project Name: Project No.: Client: CAP Norwood Dev 42 & 52 Mill Street, Norwood 14288-003 Method: Date Completed: Contractor: Canadian Environmental Solid Stem Auger April 21, 2022 UTM: Location: 42 Mill Street, Norwood 18T 263506.3392 E, 4919437.261 N Elevation: 210.409 masl

SUBSURFACE PROFILE **SAMPLE** DCPT Moisture SPT (N) / DCPT Recovery  $\frac{1}{2}$ Lithology Number % Well SPT Œ Description Remarks Installation 25 50 75 10 20 30 40 TOPSOIL: 300mm thick topsoil: Dark 1A SS brown, silt, trace sand, trace 75 3 organics, moist, very loose SANDY SILT: Brown, sandy silt, trace clay, moist, very loose SAND AND SILT: Light brown, gravelly 2 20 SS 80 sand and silt, trace clay, moist, compact 209 Groundwater first encountered at 1.52 mbgs 3 SS 80 13 -becomes wet Water level upon completion at 2.13 mbgs 208 4 SS 60 14 5 SS 50 13 -becomes saturated 207 Borehole open upon completion 206 100 -becomes very dense 100 Borehole terminated at 4.67 mbgs on presumed bedrock 205



Client:

Contractor:

Location:

Peterborough Barrie Oshawa Kingston

T: 866-217-7900 www.cambium-inc.com

Canadian Environmental

42 Mill Street, Norwood

**CAP Norwood Dev** 

Log of Borehole:

BH109-22 Page 1 of 1

Project Name: 42 & 52 Mill Street, Norwood

 Project No.:
 14288-003

 Date Completed:
 April 21, 2022

**UTM:** 18T 263498.4301 E, 4919578.083 N

Solid Stem Auger

Method:

**Elevation:** 209.636 masl

	SUBSU	RFACE PROFILE				SAN	IPLE			
Elevation (m) Depth	Lithology	Description	Number	Type	% Recovery	SPT (N) / DCPT	- 50 % Moisture	/(N) LdS 20-	Well Installation	Remarks
210 —		TOPSOIL: 150mm thick topsoil: Dark brown, silt, trace sand, trace organics, moist, very loose  SILTY SAND: Brown, silty sand, trace clay, moist, very loose  SILT AND SAND: Brown, silt and sand, trace clay, moist to wet, very loose  SAND AND SILT: Light brown, gravelly sand and silt, moist, compact  -becomes very dense  Borehole terminated at 4.88 mbgs on presumed bedrock	1A 1B 2A 2B 3	SS SS SS SS SS	75 60 100 100	3 1 27 51 60				SS3 GSA: 21% gravel 40% sand 39% silt & clay  Borehole open and dry upon completion



Log of Borehole:

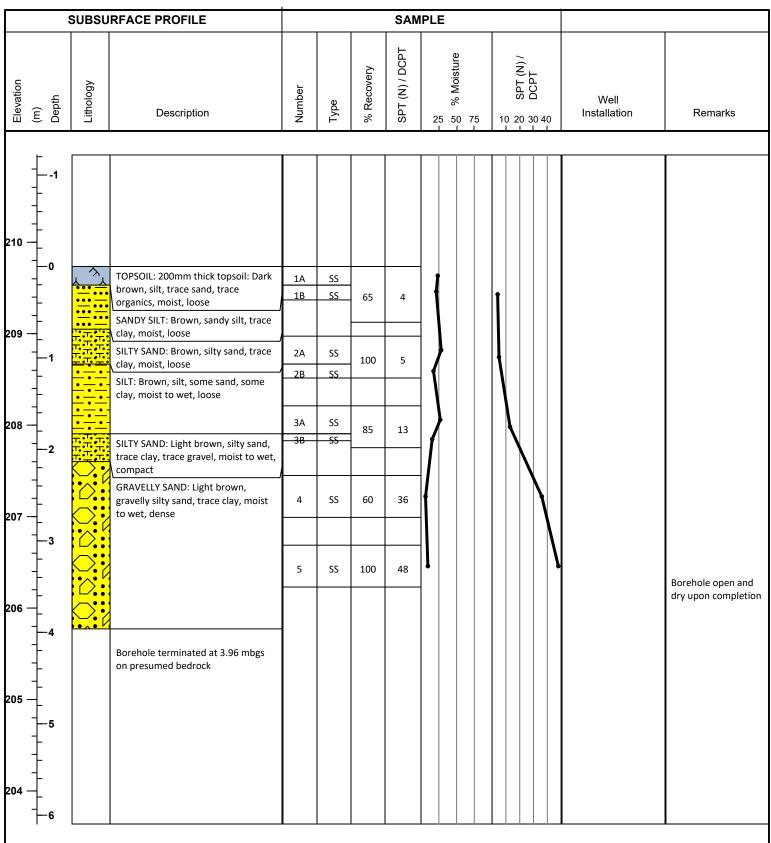
BH110-22

Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Client:CAP Norwood DevProject Name:42 & 52 Mill Street, NorwoodProject No.:14288-003Contractor:Canadian EnvironmentalMethod:Solid Stem AugerDate Completed:April 21, 2022

**Location:** 42 Mill Street, Norwood **UTM:** 18T 263594.2056 E, 4919677.787 N **Elevation:** 209.735 masl





Log of Borehole:

BH111-22 Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Project Name: Project No.: Client: **CAP Norwood Dev** 42 & 52 Mill Street, Norwood 14288-003 Contractor: Canadian Environmental Method: Solid Stem Auger Date Completed: April 21, 2022 Location: 42 Mill Street, Norwood UTM: 18T263661.3887 E, 4919579.516 N Elevation: 210.205 masl

	SU	JBSUI	RFACE PROFILE				SAN	IPLE			
Elevation (m)	Depth	Lithology	Description	Number	Туре	% Recovery	SPT (N) / DCPT	- 55 Woisture	/ (N) LdS DCbL	Well Installation	Remarks
211 —	-1										
210 —	0 -		TOPSOIL: 150mm thick topsoil: Dark brown, silt, trace sand, trace organics, moist, loose SILTY SAND: Brown, silty sand, trace clay, moist to wet, loose	1A 1B -1C	SS SS SS	85	7				
209 —	1		GRAVELLY SAND: Brown, gravelly silty sand, trace clay, moist to wet, compact -becomes moist, dense	2	SS	80	45				SS2 GSA: 27% gravel 44% sand 24% silt 5%clay
208 —	2		-becomes light brown, moist to wet, very dense	3	SS	30	70				Water level upon completion at 1.83 mbgs
	3		-becomes dense	4	SS	20	30				Groundwater first encountered at 3.05
207 —			-becomes very dense	5	SS	100	50/ 400				mbgs
206 —	4					165	50/				Borehole open upon completion
205 —	5		Borehole terminated at 4.88 on presumed bedrock	6	SS	100	275				
+	6										



Log of Borehole:

BH112-22 Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Project Name: Client: CAP Norwood Dev 42 & 52 Mill Street, Norwood Project No.: 14288-003 Date Completed: Contractor: Canadian Environmental Method: Solid Stem Auger April 21, 2022 UTM: Location: 42 Mill Street, Norwood 18T 263763.0134 E, 4919801.271 N Elevation: 209.28 masl

SUBSURFACE PROFILE **SAMPLE** DCPT Moisture SPT (N) / DCPT Recovery  $\hat{z}$ Lithology Number % Well SPT (E) Description Remarks Installation 25 50 75 10 20 30 40 Pipe TOPSOIL: 200mm thick topsoil: Dark 1A SS brown, silt, trace sand, trace 209 85 3 Bentonite SS organics, moist, very loose 1B Plug SANDY SILT: Brown, sandy silt, trace Groundwater first encountered at 0.76 clay, moist, very loose mbgs SILTY SAND: Brown, silty sand, wet, 2 Water level upon SS 100 4 completion at 0.76 208 mbgs SILT: Brown, silt, some sand, some Water level clay, wet, loose 3A 3B measured at 1.24 SS Sand Pack mbgs on May 4, 50 5 SILTY SAND: Brown, silty sand, trace PVC 2022 clay, wet, loose Screen GRAVELLY SAND: Brown, gravelly 207 silty sand, trace clay, saturated, very 50/ 4 SS 40 dense Borehole caving 375 Cap occurred up to 2.59 mbgs upon Borehole terminated at 2.67 mbgs completion on presumed bedrock 206 205 204



Log of Borehole:

BH113-22

Page 1 of 1

T: 866-217-7900 www.cambium-inc.com

Project Name: Project No.: Client: CAP Norwood Dev 42 & 52 Mill Street, Norwood 14288-003 Method: Date Completed: Contractor: Canadian Environmental Solid Stem Auger April 20, 2022 UTM: 18T 263859.3257 E, 4919563.455 N Location: 42 Mill Street, Norwood Elevation: 211.703 masl

SUBSURFACE PROFILE **SAMPLE** (N) / DCPT Moisture SPT (N) / DCPT Recovery Lithology Elevation Number (m) Depth Well % SPT Description Installation Remarks 25 50 75 10 20 30 40 212 TOPSOIL: 300mm thick topsoil: Dark SS brown, silt, trace sand, trace 80 3 organics, moist, very loose 1B SS SANDY SILT: Brown, sandy silt, trace clay, moist, very loose GRAVELLY SAND: Grey, gravelly silty 2 38 SS 50 sand, dry to moist, dense Borehole open and 210 50/ dry upon completion 3 SS 70 400 -becomes very dense Borehole terminated at 1.98 mbgs on presumed bedrock 209 208 207 206



Peterborough Barrie Oshawa Kingston T: 866-217-7900

Log of Borehole: BH114-22

Page 1 of 1

Project Name: Project No.: Client: **CAP Norwood Dev** 42 & 52 Mill Street, Norwood 14288-003 Contractor: Canadian Environmental Method: Solid Stem Auger Date Completed: April 20, 2022 Location: 42 Mill Street, Norwood UTM: 18T 263757.541 E, 4919417.733 N Elevation: 213.015 masl

	SUBSU	RFACE PROFILE				SAN	IPLE			
Elevation (m) Depth	Lithology	Description	Number	Type	% Recovery	SPT (N) / DCPT	- 55 75 Woisture	/ (N) Ld OO 30 40	Well Installation	Remarks
214										
2130		TOPSOIL: 100mm thick topsoil: Dark brown, silt, trace sand, trace organics, moist, loose GRAVELLY SAND: Brown/grey,	1A 1B	SS SS	50	16				
212 ——1  		gravelly silty sand, dry to moist, compact -becomes very dense	2	SS	60	50/ 275	- •			
211 ——2			3	SS	85	58				
			4	SS	100	50/ 100		,		
2103			5	SS	80	50/ 250				Borehole open and dry upon completion
209 — 4		Borehole terminated at 3.5 mbgs on presumed bedrock								
208 5										
 207 ——6										



Log of Borehole:

BH115-22

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T: 866-217-7900 www.cambium-inc.com

Project Name: Project No.: Client: **CAP Norwood Dev** 42 & 52 Mill Street, Norwood 14288-003 Contractor: Canadian Environmental Method: Solid Stem Auger Date Completed: April 20, 2022 Location: 42 Mill Street, Norwood UTM: 18T 263931.2517 E, 4919417.57 N Elevation: 210.833 masl

,	SUBSU	RFACE PROFILE				SAN	IPLE			
Elevation (m) Depth	Lithology	Description	Number	Туре	% Recovery	SPT (N) / DCPT	25 50 75 - 25 75	/(N) LdS 0 40	Well Installation	Remarks
212 — _—-1 _—									Сар	
211 0	^ 	TOPSOIL: 200mm thick topsoil: Dark brown, silt, trace sand, trace organics, moist, loose SANDY SILT: Brown, sandy silt, trace	1A 1B	SS SS	65	4			Pipe  Bentonite Plug	
210 —1		clay, trace organics, moist, loose GRAVELLY SAND: Brown, gravelly silty sand, moist, dense	2	SS	50	42				Groundwater first encountered at 1.52 mbgs
209 —			3	SS	80	32			Sand Pack PVC Screen	SS3 GSA: 34% gravel 46% sand 20% silt & clay Water level upon
208 —		-becomes very dense  Borehole terminated at 2.59 mbgs on presumed bedrock	4	SS	60	50/ 300			Сар	completion at 1.68 mbgs Water level measured at 2.02 mbgs on May 4, 2022 Borehole open upon
207 — 4 										completion
206 — 5 5										
205 —										



Log of Borehole:

BH116-22

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T: 866-217-7900 www.cambium-inc.com

Project Name: Project No.: Client: **CAP Norwood Dev** 42 & 52 Mill Street, Norwood 14288-003 Contractor: Canadian Environmental Method: Solid Stem Auger Date Completed: April 20, 2022 Location: 42 Mill Street, Norwood

UTM: 18T 263788.3967 E, 4919215.355 N Elevation: 210.406 masl

SUBS	SURFACE PROFILE				SAM	PLE			
Elevation (m) Depth Lithology	Description	Number	Туре	% Recovery	SPT (N) / DCPT	- 55 % Moisture	DCPT (N) / -0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	Well Installation	Remarks
211 — -1 211 — -0 210 —	brown, silt, trace sand, trace organics, moist, loose SILTY SAND: Brown, silty sand, trace gravel, trace clay, moist, loose -becomes compact  GRAVELLY SAND: Brown, gravelly sand, some silt, trace clay, moist, dense	1A 1B 2A 2B 3	SS SS SS SS	100	7 10 42 50/ 275				Borehole open and dry upon completion



Log of Borehole:

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T: 866-217-7900 www.cambium-inc.com

Project Name: Project No.: Client: **CAP Norwood Dev** 42 & 52 Mill Street, Norwood 14288-003 Contractor: Canadian Environmental Method: Solid Stem Auger Date Completed: April 20, 2022 Location: 42 Mill Street, Norwood UTM: 18T 263640.3596 E, 4919276.71 N Elevation: 210.34 masl

		HECLI	DEACE DROFT E				CAR	DI E			
		00800	RFACE PROFILE		1	I	SAN	IPLE	<u> </u>		
Elevation (m)	Depth	Lithology	Description	Number	Type	% Recovery	SPT (N) / DCPT	25 50 75 - 25 50 75	/(N) LdS DCbL	Well Installation	Remarks
211 —	1 2 3		TOPSOIL: 200mm thick topsoil: Dark brown, silt, trace sand, trace organics, moist, very loose  SILTY SAND: Brown, silty sand, trace clay, moist, loose  GRAVELLY SAND: Brown, gravelly sand, some silt, trace clay, moist, compact  SILT: Brown, silt, some clay, some sand, some gravel, moist to wet, loose  GRAVELLY SAND: Brown, gravelly silty sand, trace clay, moist to wet, compact  -becomes very dense  Borehole terminated at 4.63 mbgs on presumed bedrock	1A 1B 2 3 4 5	SS SS SS SS SS	80 100 80	3 4 17 9 29	25 50 75	10 20 30 40		SS3 GSA: 20% gravel 65% sand 15% silt & clay Groundwater first encountered at 2.29 mbgs  Water level upon completion at 4.57 mbgs Borehole open upon completion
†	-6										

### 42 & 52 Mill Street, Norwood



Test Pit ID	Depth (mbgs <sup>1</sup> )	Soil Sample	% Moisture	Material Description	Depth (m)	DPT <sup>2</sup> (Blows/150 mm)
					0.15	2
	0-0.15	GS1		150 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	4
TP118-22	0.15-0.75	GS2		Brown, SILTY SAND, trace clay, moist	0.45	6
	0.75-1.5	GS3		Light brown, GRAVELLY SILTY SAND, trace clay	0.6	8
18T 263459.2299 E	1.5-2.4	GS4		Same as above	0.75	6
4919096.046 N	2.4-3.0	GS5		Same as above	0.9	10
					1.05	17
206.812 masl					1.2	22
				Test pit terminated at 3.0 mbgs in gravelly silty sand	1.35	4
				No groundwater or caving observed upon completion	1.5	12
					1.65	50
					0.15	2
TD440 00	0-0.3	GS1		300 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	5 7
TP119-22	0.3-1.2	GS2 GS3		Light brown, SILTY SAND, trace clay, moist	0.45 0.6	6
18T 263307.8794 E	1.2-2.1 2.1-3.0	GS4		Light brown, GRAVELLY SILTY SAND, moist Same as above	0.6	6
4919249.476 N	3.3	034		Bedrock	0.73	8
4313243.470 N	3.3			Ded dek	1.05	9
203.588 masl					1.2	13
				Test pit terminated at 3.3 mbgs on bedrock	1.35	4
				No groundwater or caving observed upon completion	1.5	16
					1.65	22
					1.8	28
					1.95	27
					2.1	35
					2.25	50
					0.15	1
	0-0.15	GS1		150 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	3
TP120-22	0.15-0.9	GS2		Brown, SILTY SAND, trace clay, moist	0.45	6
	0.9-1.5	GS3		Brown, SILTY SAND, some clay, moist to wet	0.6	7
18T 263381.7756 E	1.5-2.1	GS4		Light brown, SILTY SAND, some gravel, trace clay, moist	0.75	8
4919293.79 E	2.1-3.0	GS5		Light brown, GRAVELLY SILTY SAND, trace clay, moist	0.9 1.05	9 10
203.488 masl					1.05	8
205.400 111851				Test pit terminated at 3.0 mbgs in gravelly silty sand	1.35	2
				No groundwater or caving observed upon completion	1.55	4
				The gradientation of dating data for admired on	1.65	11
					1.8	11
					1.95	10
					2.1	28
					2.25	39
					2.4	48

<sup>1.</sup> mbgs = metres below ground surface

<sup>2.</sup> Dynamic probe penetration test, consisting of driving a 19 mm diameter steel rod 150 mm into the soil with an 8 kg hammer falling 750 mm.

## 42 & 52 Mill Street, Norwood



Test Pit ID	Depth (mbgs <sup>1</sup> )	Soil Sample	% Moisture	Material Description	Depth (m)	DPT <sup>2</sup> (Blows/150 mm)
					0.15	1
	0-0.15	GS1		150 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	3
TP121-22	0.15-0.75	GS2		Brown, SILTY SAND, trace clay, moist	0.45	6
	0.75-1.5	GS3		Brown, SILTY SAND, some clay, moist to wet	0.6	8
18T 263323.0037 E	1.5-2.4	GS4		Light brown/grey, SILTY SAND, some clay, moist	0.75	9
4919401.899 N	2.4-3.0	GS5		Same as above, trace gravel	0.9	10
					1.05	13
203.75 masl					1.2	15
				Test pit terminated at 3.6 mbgs in silty sand	1.35	1
				Groundwater seepage observed at 3.3 mbgs	1.5	3
				Water level observed at 3.6 mbgs upon completion	1.65	4
				Sidewall caving observed at 3.3 mbgs	1.8	5
					1.95	9
					2.1	17
					2.25	30
					2.4	38
					0.15	2
	0-0.2	GS1		200 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	5
TP122-22	0.2-1.2	GS2		Light brown, SILTY SAND, trace clay, moist	0.45	5
407.000405.4040.5	1.2-2.25	GS3		Same as above	0.6	7
18T 263435.1018 E	2.25-3.0	GS4		Light brown, GRAVELLY SILTY SAND, moist	0.75	8
4919413.538 N					0.9	6
208.78 masl				Total vista universal de 2.0 septe in available side vond	1.05 1.2	5 5
208.78 Masi				Test pit terminated at 3.0 mbgs in gravelly silty sand No groundwater observed upon completion	1.35	-
				Sidewall caving observed at 0.9 mbgs	1.35	1 1
				Sidewan caving observed at 0.5 migs	1.65	1
					1.8	1
					1.95	1
					2.1	1
					2.25	1
					2.4	21

<sup>1.</sup> mbgs = metres below ground surface

<sup>2.</sup> Dynamic probe penetration test, consisting of driving a 19 mm diameter steel rod 150 mm into the soil with an 8 kg hammer falling 750 mm.

## 42 & 52 Mill Street, Norwood



Test Pit ID	Depth (mbgs <sup>1</sup> )	Soil Sample	% Moisture	Material Description	Depth (m)	DPT <sup>2</sup> (Blows/150 mm)
TP123-22  18T 263570.6278 E 4919349.463 N  209.674 masl	0-0.2 0.2-0.6 0.6-1.5 1.5-2.4 2.4-3.0	GS1 GS2 GS3 GS4 GS5		200 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SAND, some silt, some gravel, moist Light brown, SAND, some silt, trace gravel, moist Brown, GRAVELLY SAND, some silt, moist Same as above  Test pit terminated at 3.0 mbgs in gravelly sand No groundwater or caving observed upon completion	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35 1.5 1.65 1.8 1.95 2.1	2 5 8 8 7 5 12 22 3 9 11 13 13 17
TP124-22  18T 263487.625 E 4919227.613 N  205.446 masl	0-0.3 0.3-1.2 1.2-2.1 2.1-3.0	GS1 GS2 GS3 GS4		300 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Light brown, GRAVELLY SILTY SAND, trace clay, moist Same as above Same as above Test pit terminated at 3.0 mbgs in gravelly silty sand No groundwater or caving observed upon completion	2.4 0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35 1.5 1.65	50 3 9 39 50 4 20 50
TP125-22  18T 263587.5195 E 4919198.145 N  208.502 masl	0-0.3 0.3-1.2 1.2-2.1 2.1-3.0	GS1 GS2 GS3 GS4		300 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Light brown, GRAVELLY SILTY SAND, trace clay, moist Same as above Same as above Test pit terminated at 3.0 mbgs in gravelly silty sand No groundwater or caving observed upon completion	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35 1.5	1 2 2 4 11 50

<sup>1.</sup> mbgs = metres below ground surface

<sup>2.</sup> Dynamic probe penetration test, consisting of driving a 19 mm diameter steel rod 150 mm into the soil with an 8 kg hammer falling 750 mm.

## 42 & 52 Mill Street, Norwood



Test Pit ID	Depth (mbgs <sup>1</sup> )	Soil Sample	% Moisture	Material Description	Depth (m)	DPT <sup>2</sup> (Blows/150 mm)
TP126-22  18T 263673.8582 E 4919184.81 N  211.056 masl	0-0.15 0.15-0.9 0.9-1.5 1.5-2.4 2.4-3.0	GS1 GS2 GS3 GS4 GS5		150 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SILTY SAND, trace clay, trace organics, moist Light brown, SAND, some silt, trace clay, moist Light brown, GRAVELLY SILTY SAND, moist Same as above  Test pit terminated at 3.0 mbgs in gravelly silty sand No groundwater or caving observed upon completion	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35 1.5 1.65 1.8	1 2 2 2 3 4 3 4 3 11 29 50
TP127-22 18T 263736.9108 E 4919266.591 N 208.81 masl	0-0.15 0.15-0.75 0.75-1.65 1.65-2.55	GS1 GS2 GS3 GS4		150 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SILTY SAND, trace clay, trace organics, moist Light brown, SAND, some silt, trace clay, moist Light brown, GRAVELLY SILTY SAND, moist  Test pit terminated at 2.55 mbgs on bedrock No groundwater or caving observed upon completion	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35 1.5	2 4 6 6 6 7 5 5 8 50
TP128-22  18T 263669.5935 E 4919343.14 N  208.276 masl	0-0.2 0.2-0.9 0.9-1.5 1.5-2.4 2.4-3.0	GS1 GS2 GS3 GS4 GS5		200 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SANDY SILT, trace clay, moist Brown, SILTY SAND, trace clay, moist to wet Light brown, GRAVELLY SILTY SAND, moist to wet Same as above  Test pit terminated at 3.0 mbgs in gravelly silty sand Groundwater seepage observed at 2.4 mbgs Water level observed at 2.55 mbgs upon completion Sidewall caving observed at 0.9 mbgs	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35 1.5 1.65 1.8 1.95 2.1 2.25	2 4 5 6 9 10 12 11 2 4 5 12 17 7 15 50

<sup>1.</sup> mbgs = metres below ground surface

<sup>2.</sup> Dynamic probe penetration test, consisting of driving a 19 mm diameter steel rod 150 mm into the soil with an 8 kg hammer falling 750 mm.

## 42 & 52 Mill Street, Norwood



Test Pit ID	Depth (mbgs <sup>1</sup> )	Soil Sample	% Moisture	Material Description	Depth (m)	DPT <sup>2</sup> (Blows/150 mm)
					0.15	2
	0-0.3	GS1		300 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	7
TP129-22	0.3-0.9	GS2		Brown, SILTY SAND, trace clay, moist	0.45	8
	0.9-1.5	GS3		Brown, SILT AND SAND, trace clay, moist	0.6	12
18T 263689.7605 E	1.5-2.4	GS4		Light brown, GRAVELLY SILTY SAND, moist to wet	0.75	18
4919465.412 N	2.4-3.0	GS5		Same as above, wet	0.9	14
					1.05	13
210.846 masl					1.2	14
				Test pit terminated at 3.0 mbgs in gravelly silty sand	1.35	5
				Groundwater seepage observed at 2.4 mbgs	1.5	15
				Water level observed at 2.85 mbgs upon completion	1.65	44
				No caving observed upon completion	1.8	50
					0.15	1
	0-0.2	GS1		200 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	4
TP130-22	0.2-0.9	GS2		Brown, SILTY SAND, trace clay, moist to wet	0.45	7
	0.9-1.8	GS3		Brown, SILTY SAND, some gravel, trace clay, moist to wet	0.6	9
18T 263592.0739 E	1.8-2.4	GS4		Light brown, GRAVELLY SILTY SAND, trace clay, moist to wet	0.75	10
4919499.753 N	2.4-3.0	GS5		Same as above	0.9	12
					1.05	16
210.096 masl					1.2	22
				Test pit terminated at 3.0 mbgs in gravelly silty sand	1.35	4
				Groundwater seepage observed at 1.8 mbgs	1.5	4
				Water level observed at 3.0 mbgs upon completion  No caving observed upon completion	1.65 1.8	3
				No caving observed upon completion	1.8	13
					2.1	33
					2.25	50
					0.15	1
	0-0.2	GS1		200 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	3
TP131-22	0.2-1.2	GS2		Light brown, GRAVELLY SAND, some silt, trace clay, moist to wet	0.45	5
	1.2-2.1	GS3		Same as above	0.6	7
18T 263566.1644 E	2.1-3.0	GS4		Same as above	0.75	9
4919572.217 N					0.9	12
					1.05	50
209.662 masl				Test pit terminated at 3.0 mbgs in gravelly sand	1.2	
				No groundwater or caving observed upon completion	1.35	4
					1.5	50

<sup>1.</sup> mbgs = metres below ground surface

<sup>2.</sup> Dynamic probe penetration test, consisting of driving a 19 mm diameter steel rod 150 mm into the soil with an 8 kg hammer falling 750 mm.

### 42 & 52 Mill Street, Norwood



Test Pit ID	Depth (mbgs <sup>1</sup> )	Soil Sample	% Moisture	Material Description	Depth (m)	DPT <sup>2</sup> (Blows/150 mm)
TP132-22	0-0.15 0.15-0.6	GS1 GS2		150 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SILTY SAND, trace clay, moist	0.15 0.3 0.45	1 3 5
	0.6-1.5	GS3		Brown, SILT AND SAND, trace clay, moist	0.6	6
18T 263707.1165 E	1.5-2.4	GS4		Light brown, GRAVELLY SILTY SAND, moist to wet	0.75	9
4919695.665 N	2.4-3.0	GS5		Same as above, wet	0.9	12
200.04					1.05	12
209.94 masl				Test with the resident of the 20 miles in group like site.	1.2	11
				Test pit terminated at 3.0 mbgs in gravelly silty sand Groundwater seepage observed at 2.4 mbgs	1.35 1.5	2 5
				Water level observed at 2.85 mbgs upon completion	1.65	7
				No caving observed upon completion	1.8	10
				and define described approximation.	1.95	18
					2.1	39
					2.25	50
					0.15	2
	0-0.3	GS1		300 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	5
TP133-22	0.3-0.9	GS2		Brown, SILTY SAND, trace clay, moist	0.45	5
	0.9-1.2	GS3		Light brown, GRAVELLY SILTY SAND, wet, trace clay	0.6	10
18T 263811.4872 E	1.2-2.4	GS4		Same as above, moist	0.75	8
4919691.934 N	2.4-3.0	GS5		Same as above	0.9	14
					1.05	10
210.921 masl					1.2	20
				Test pit terminated at 3.0 mbgs in gravelly silty sand	1.35	1
				Groundwater at 1.05 mbgs	1.5	3
				No water level or caving observed upon completion	1.65 1.8	8 20
					1.8	36
					2.1	50
					2.1	30
					0.15	2
	0-0.2	GS1		200 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	4
TP134-22	0.2-1.2	GS2		Light brown, SILTY SAND, trace clay, moist	0.45	6
	1.2-1.8	GS3		Light brown, GRAVELLY SILTY SAND, moist	0.6	8
18T 263754.5517 E	1.8-2.25	GS4		Same as above	0.75	22
4919601.034 E		1			0.9	50
					1.05	
213.61 masl				Test pit terminated at 2.25 mbgs on bedrock	1.2	8
				No groundwater or caving observed upon completion	1.35	50

<sup>1.</sup> mbgs = metres below ground surface

<sup>2.</sup> Dynamic probe penetration test, consisting of driving a 19 mm diameter steel rod 150 mm into the soil with an 8 kg hammer falling 750 mm.

## 42 & 52 Mill Street, Norwood



Test Pit ID	Depth (mbgs <sup>1</sup> )	Soil Sample	% Moisture	Material Description	Depth (m)	DPT <sup>2</sup> (Blows/150 mm)
TP135-22 18T 263781.1848 E 4919504.206 N 211.982 masl	0-0.2 0.2-0.6 0.6-1.2 1.2-1.95	GS1 GS2 GS3 GS4		200 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SILTY SAND, trace clay, moist Light brown, GRAVELLY SILTY SAND, moist Same as above  Test pit terminated at 1.95 mbgs on bedrock No groundwater or caving observed upon completion	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35 1.5 1.65 1.8 1.95 2.1 2.25	2 10 8 6 5 50 7 17 50
TP136-22  18T 263881.0651 E 4919463.612 N  213.088 masl	0-0.3 0.3-0.6 0.6-1.5	GS1 GS2 GS3		300 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SILTY SAND, trace clay, moist Brown, GRAVELLY SILTY SAND, moist  Test pit terminated at 1.5 mbgs on bedrock No groundwater or caving observed upon completion  Large cobbles throughout	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35	1 5 6 19 50
TP137-22 18T 263827.6047 E 4919332.42 N 210.368 masl	0-0.3 0.3-1.2 1.2-1.95	GS1 GS2 GS3		300 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SILTY SAND, trace clay, moist Same as above  Test pit terminated at 1.95 mbgs on bedrock No groundwater or caving observed upon completion	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35 1.5 1.65	2 3 4 4 5 5 6 6 0 2 50

<sup>1.</sup> mbgs = metres below ground surface

<sup>2.</sup> Dynamic probe penetration test, consisting of driving a 19 mm diameter steel rod 150 mm into the soil with an 8 kg hammer falling 750 mm.

### 42 & 52 Mill Street, Norwood



Test Pit ID	Depth (mbgs <sup>1</sup> )	Soil Sample	% Moisture	Material Description	Depth (m)	DPT <sup>2</sup> (Blows/150 mm)
TP138-22  18T 263861.8087 E 4919252.342 N  209.877 masl	0-0.15 0.15-1.05 1.05-1.5	GS1 GS2 GS3		150 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SILTY SAND, trace clay, moist Light brown, SILTY SAND, some clay, moist to wet  Test pit terminated at 1.5 mbgs on bedrock No groundwater or water level observed upon completion Sidewall caving observed at 0.6 mbgs	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35	1 3 7 8 15 50 15 50
TP139-22  18T 263604.7646 E 4919410.865 N  209.924 masl	0-0.3 0.3-1.2 1.2-2.1 2.1-3.0	GS1 GS2 GS3 GS4		300 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SILTY SAND, trace clay, moist Light brown, GRAVELLY SILTY SAND, trace clay, moist Same as above  Test pit terminated at 3.0 mbgs in gravelly silty sand No groundwater or caving observed upon completion	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35 1.5 1.65 1.8 1.95	2 5 6 6 7 8 10 11 1 2 3 9 29 50
TP140-22 18T 263475.3943 E 4919493.625 N 209.734 masl	0-0.3 0.3-0.75 0.75-1.5 1.5-2.4 2.4-3.0	GS1 GS2 GS3 GS4 GS5		300 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist Brown, SILTY SAND, trace clay, moist Light brown, GRAVELLY SAND, trace clay, moist to wet, some silt Same as above, wet Same as above  Test pit terminated at 3.0 mbgs in gravelly sand Groundwater seepage observed at 1.95 mbgs Water level observed at 2.85 mbgs upon completion Sidewall caving observed at 0.9 mbgs	0.15 0.3 0.45 0.6 0.75 0.9 1.05 1.2 1.35 1.5 1.65	2 5 9 9 8 10 16 29 5 10 27 50

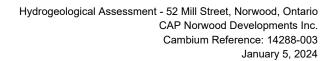
<sup>1.</sup> mbgs = metres below ground surface

<sup>2.</sup> Dynamic probe penetration test, consisting of driving a 19 mm diameter steel rod 150 mm into the soil with an 8 kg hammer falling 750 mm.

## 42 & 52 Mill Street, Norwood



Test Pit ID	Depth (mbgs <sup>1</sup> )	Soil Sample	% Moisture	Material Description	Depth (m)	DPT <sup>2</sup> (Blows/150
	(565 )					mm)
					0.15	2
	0-0.15	GS1		150 mm TOPSOIL: Dark brown, silt, trace sand, frequent rootlets, moist	0.3	8
TP141-22	0.15-0.75	GS2		Light brown, SILTY SAND, trace clay, moist	0.45	11
	0.75-1.8	GS3		Same as above, some gravel	0.6	12
18T 263333.4648 E	1.8-2.1	GS4		Light brown, GRAVELLY SILTY SAND, moist	0.75	22
4919169.725 N	2.1-3.0	GS5		Same as above	0.9	18
					1.05	22
204.859 masl					1.2	28
				Test pit terminated at 3.0 mbgs in gravelly silty sand	1.35	1
				No groundwater or caving observed upon completion	1.5	2
					1.65	6
					1.8	4
					1.95	9
					2.1	50





	Appendix						
Grain	Size	Analys	sis				





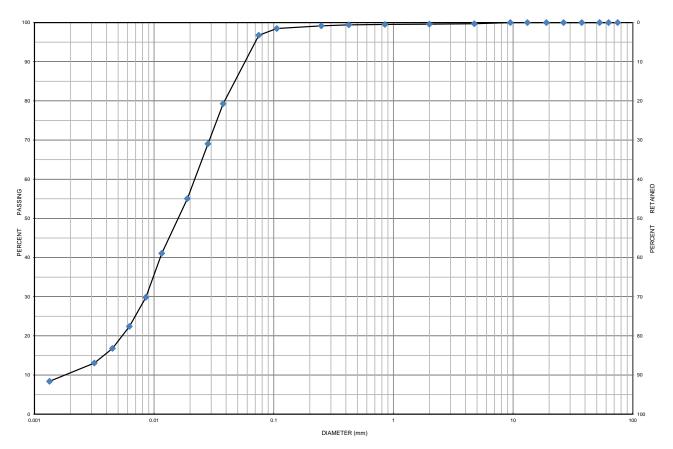
Project Number: 14288-003 Client: CAP Norwood Developments Inc.

Project Name: Hydrogeological, Geotechnical, ESA - 42 & 52 Mill St, Norwood

Sample Date: April 20 & 21, 2022 Sampled By: Josh Riseling - Cambium Inc.

**Location:** BH 102-22 SS 4 **Depth:** 2.3 m to 2.7 m **Lab Sample No:** S-22-0740

UNIFIED SOIL CLASSIFICATION SYSTEM							
CLAV 9 CHT (-0.075 mm)	SAND (<4.	75 mm to 0.075 mm)	GRAVEL (>4.75 mm)				
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	FINE	COARSE		



	MIT SOIL CLASSIFICATION SYSTEM									
CLAY	SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS		
	SILI	SAND			GRAVEL			BOULDERS		

Borehole No.	Sample No.		Depth		Gravel	ravel Sai		Silt			Clay	Moisture
BH 102-22	SS 4	2	2.3 m to 2.7 m		0		3		86		11	27.8
Description		Classification		D <sub>60</sub>		D <sub>30</sub>		D <sub>10</sub>		Cu	C <sub>c</sub>	
Silt some Clay trace Sand		ML		0.0220		0.008	7	0.0017	,	12.94	2.02	

Additional information available upon request





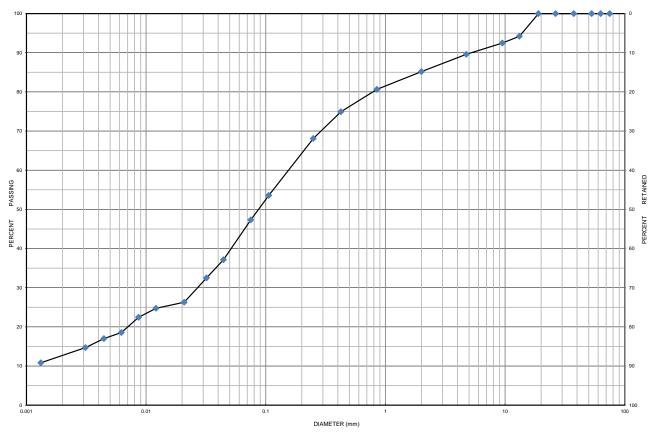
Project Number: 14288-003 Client: CAP Norwood Developments Inc.

Project Name: Hydrogeological, Geotechnical, ESA - 42 & 52 Mill St, Norwood

Sample Date: April 20 & 21, 2022 Sampled By: Josh Riseling - Cambium Inc.

**Location**: BH 104-22 SS 3 **Depth**: 1.5 m to 2 m **Lab Sample No**: S-22-0741

UNIFI	UNIFIED SOIL CLASSIFICATION SYSTEM									
CLAV 9 CHT ( -0.075 mm)	SAND (<4.	75 mm to 0.075 mm)	GRAVEL (>4.75 mm)							
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	FINE	COARSE					



		MIT SOIL CL	ASSIFICATIO	N SYSTEM				
CLAY	SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS
CLAT	SILI		SAND			GRAVEL	•	BOULDERS

Borehole No.	Sample No.		Depth	Gravel	;	Sand		Silt	Clay	Moisture
BH 104-22	SS 3		1.5 m to 2 m	10		42		35	13	8.2
	Description		Classification	D <sub>60</sub>		D <sub>30</sub>		D <sub>10</sub>	Cu	C <sub>c</sub>
Sand and S	Silt some Clay some Gr	avel	SM	0.160		0.026	3	-	-	-

Additional information available upon request





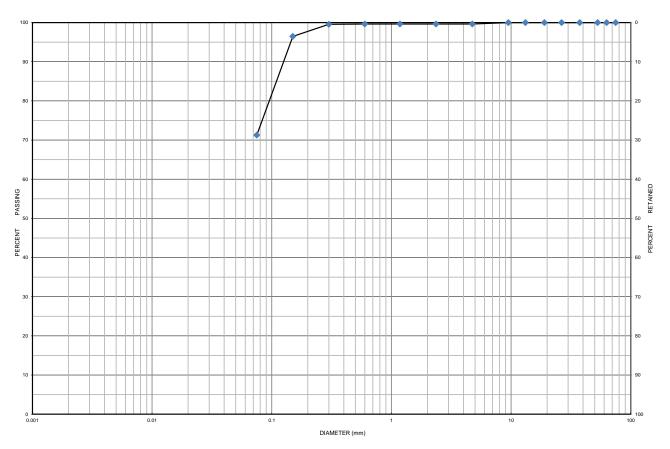
Project Number: 14288-003 Client: CAP Norwood Developments Inc.

Project Name: Hydrogeological, Geotechnical, ESA - 42 & 52 Mill St, Norwood

Sample Date: April 20 & 21, 2022 Sampled By: Josh Riseling - Cambium Inc.

**Location**: BH 105-22 SS 3 **Depth**: 1.5 m to 2 m **Lab Sample No**: S-22-0742

UNIFI	ED SOIL CLASSIF	ICATION SYSTE	M		
CLAV 9 CH T ( -0.075	SAND (<4.	75 mm to 0.075 mm)	GRAVEL (>4.75 mm)		
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	FINE	COARSE



		MIT SOIL CL	ASSIFICATIO	N SYSTEM				
CLAY	CLAY SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS
CLAT	SILI		SAND			GRAVEL		BOULDERS

Borehole No.	Sample No.	Depth		Gravel	Sa	and	Silt	Clay	Moisture
BH 105-22	SS 3	1.5 m to 2 m		0	2	28	7	2	23.7
	Description	Classificatio	n	D <sub>60</sub>		D <sub>30</sub>	D <sub>10</sub>	Cu	C <sub>c</sub>
	Sandy Silt	ML		-		-	-	-	-

	Additional	information	available	upon	request
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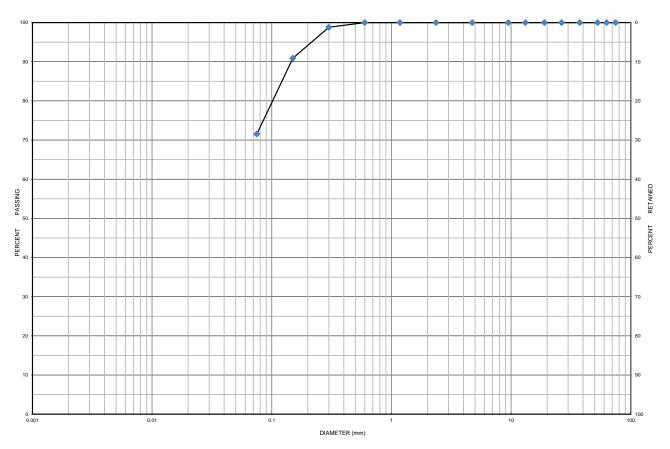
Project Number: 14288-003 Client: CAP Norwood Developments Inc.

Project Name: Hydrogeological, Geotechnical, ESA - 42 & 52 Mill St, Norwood

Sample Date: April 20 & 21, 2022 Sampled By: Josh Riseling - Cambium Inc.

Location: BH 107-22 SS 3 Depth: Lab Sample No: S-22-0743

UNIFI	ED SOIL CLASSIF	ICATION SYSTE	М		
CLAV 9 CH T ( -0.075	SAND (<4.	75 mm to 0.075 mm)	GRAVEL (>4.75 mm)		
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	FINE	COARSE



		MIT SOIL CL	ASSIFICATIO	N SYSTEM				
CLAY	CLAY SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS
CLAT	SILI		SAND			GRAVEL		BOULDERS

Borehole No.	Sample No.	Depth	Gravel	;	Sand	Silt		Clay	Moisture
BH 107-22	SS 3		0		28	7:	2		16.1
	Description	Classification	D <sub>60</sub>		D <sub>30</sub>	D <sub>10</sub>		Cu	C <sub>c</sub>
	Sandy Silt	ML	-		-	-		-	-

Additional information available upon request





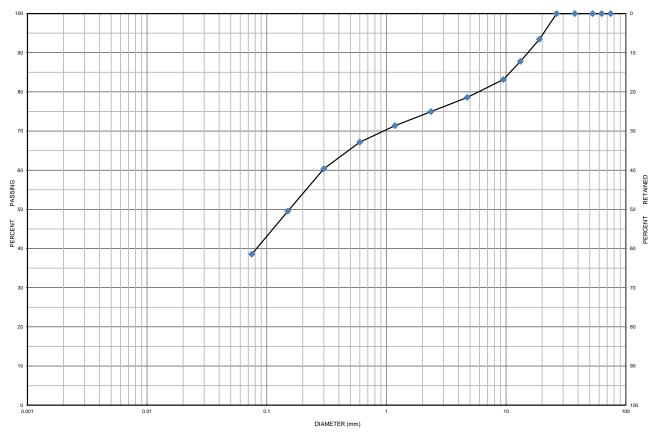
Project Number: 14288-003 Client: CAP Norwood Developments Inc.

Project Name: Hydrogeological, Geotechnical, ESA - 42 & 52 Mill St, Norwood

Sample Date: April 20 & 21, 2022 Sampled By: Josh Riseling - Cambium Inc.

**Location:** BH 109-22 SS 3 **Depth:** 1.5 m to 2 m **Lab Sample No:** S-22-0744

UNIFI	ED SOIL CLASSIF	ICATION SYSTE	М		
CLAV 9 CHT ( .0 075 mm)	SAND (<4.	75 mm to 0.075 mm)	GRAVEL (>4.75 mm)		
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	FINE	COARSE



		MIT SOIL CL	ASSIFICATIO	N SYSTEM				
CLAY	SILT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS
CLAT	SILI		SAND			GRAVEL		BOULDERS

Borehole No.	Sample No.	Depth	Gravel	Sand			Silt		Clay	Moisture
BH 109-22	SS 3	1.5 m to 2 m	21		40		39	39		8.2
	Description	Classification	D <sub>60</sub>		D <sub>30</sub>		D <sub>10</sub>		Cu	C <sub>c</sub>
Gra	velly Sand and Silt	SM	0.300		-		-		-	-

Additional information available upon request





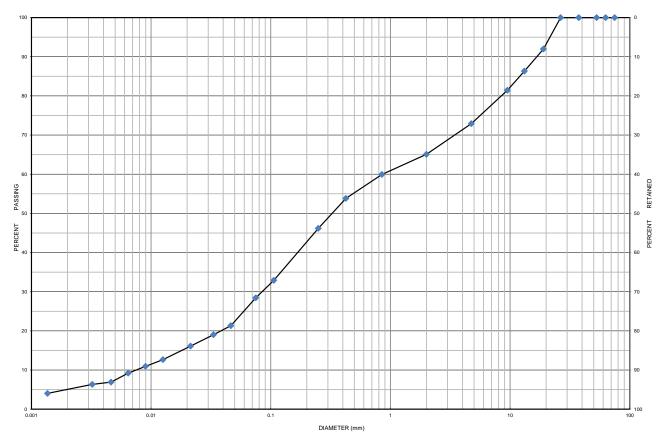
Project Number: 14288-003 Client: CAP Norwood Developments Inc.

**Project Name:** Hydrogeological, Geotechnical, ESA - 42 & 52 Mill St, Norwood

Sample Date: April 20 & 21, 2022 Sampled By: Josh Riseling - Cambium Inc.

**Location:** BH 111-22 SS 2 **Depth:** 0.8 m to 1.2 m **Lab Sample No:** S-22-0747

UNIFIED SOIL CLASSIFICATION SYSTEM											
OLAY 8 CH T ( 0.075 ****)	SAND (<4.	75 mm to 0.075 mm)	GRAVEL (>4.75 mm)								
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	FINE	COARSE						



	MIT SOIL CLASSIFICATION SYSTEM											
CLAV	SHT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS				
CLAT	CLAY SILT		SAND			GRAVEL		BOULDERS				

Borehole No.	Sample No.	Depth		Gravel	Sand		Silt		Clay		Moisture
BH 111-22	SS 2	0.8 m to 1.2 m		27		44		24		5	6.3
	Description	Classification		D <sub>60</sub>		D <sub>30</sub>		D <sub>10</sub>		Cu	C <sub>c</sub>
Gravell	y Silty Sand trace Clay	SM		0.8800		0.086	0	0.0075	5	117.33	1.12

Additional information available upon request





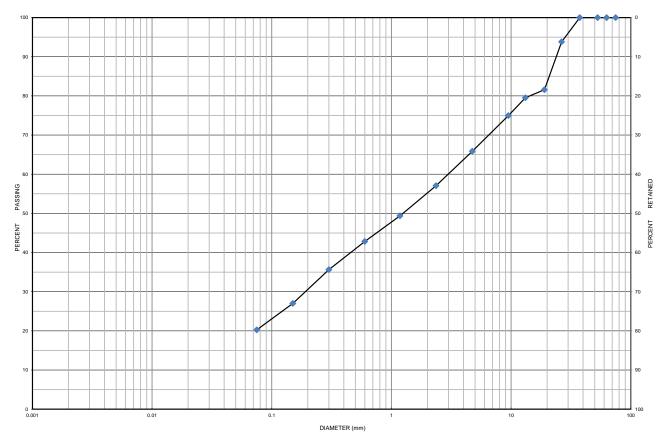
Project Number: 14288-003 Client: CAP Norwood Developments Inc.

**Project Name:** Hydrogeological, Geotechnical, ESA - 42 & 52 Mill St, Norwood

Sample Date: April 20 & 21, 2022 Sampled By: Josh Riseling - Cambium Inc.

**Location:** BH 115-22 SS 3 **Depth:** 1.5 m to 2 m **Lab Sample No:** S-22-0745

UNIFIED SOIL CLASSIFICATION SYSTEM											
OLAY 8 CH T ( 0.075 ****)	SAND (<4.	75 mm to 0.075 mm)	GRAVEL (>4.75 mm)								
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	FINE	COARSE						



	MIT SOIL CLASSIFICATION SYSTEM											
CLAV	SHT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS				
CLAT	CLAY SILT		SAND			GRAVEL		BOULDERS				

Borehole No.	Sample No.	Depth	Gravel	Bravel S		Silt			Clay	Moisture
BH 115-22	SS 3	1.5 m to 2 m	34		46		20			6.4
	Description	Classification	D <sub>60</sub>		D <sub>30</sub>		D <sub>10</sub>		Cu	C <sub>c</sub>
G	ravelly Silty Sand	SM	3.000		0.190	)	-		-	-

Additional information available upon request





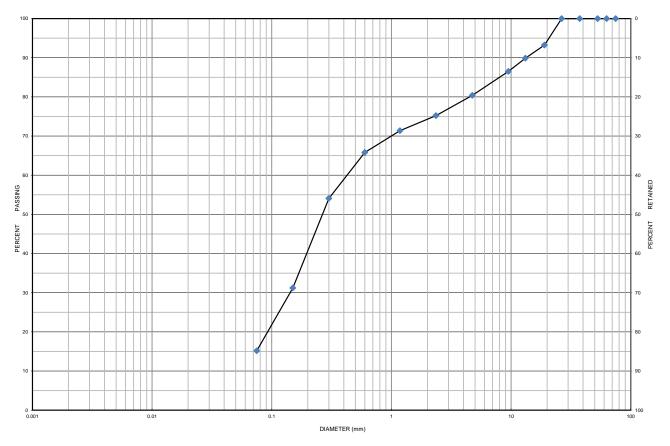
Project Number: 14288-003 Client: CAP Norwood Developments Inc.

Project Name: Hydrogeological, Geotechnical, ESA - 42 & 52 Mill St, Norwood

Sample Date: April 20 & 21, 2022 Sampled By: Josh Riseling - Cambium Inc.

**Location:** BH 117-22 SS 3 **Depth:** 1.5 m to 2 m **Lab Sample No:** S-22-0746

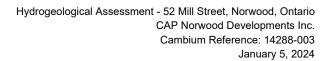
UNIFIED SOIL CLASSIFICATION SYSTEM											
OLAY 8 CH T ( 0.075 ****)	SAND (<4.	75 mm to 0.075 mm)	GRAVEL (>4.75 mm)								
CLAY & SILT (<0.075 mm)	FINE	MEDIUM	COARSE	FINE	COARSE						



	MIT SOIL CLASSIFICATION SYSTEM											
CLAY SILT	SHT	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	BOULDERS				
	SILI		SAND			GRAVEL		BOULDERS				

Borehole No.	Sample No.	Depth	Gravel	Sand			Silt		Clay	
BH 117-22	SS 3	1.5 m to 2 m	20		65	15		5		4.7
	Description	Classification	D <sub>60</sub>		D <sub>30</sub>		D <sub>10</sub>		Cu	C <sub>c</sub>
Grav	velly Sand some Silt	SM	0.420		0.150	)	-		-	-

Additional information available upon request





Appendix D
Water Well Survey Letter and Questionnaire



Environmental

Geotechnical

**Building Sciences** 

Construction Quality Verification

#### Telephone

(866) 217.7900 (705) 742.7900

#### Facsimile

(705) 742.7907

#### Website

cambium-inc.com

#### **Mailing Address**

P.O. Box 325, Peterborough, Ontario Canada, K9J 6Z3

#### Locations

Peterborough Kingston Barrie Oshawa Calgary

**Laboratory** Peterborough





Cambium Reference: 14288-003

Dear property owner,

Cambium Inc. is completing a hydrogeological assessment of the property at 52 Mill Street, Norwood in support of potential residential development of that site. As part of the assessment, we are taking inventory of private groundwater users located adjacent to the work area. The purpose of the inventory is to identify nearby water supply wells that may be sensitive to the development.

If a supply well is located on your property, we are requesting that you please review and complete the attached questionnaire. Complete as much information as possible and scan the document (or take a photograph) and email to <a href="michelle.rea@cambium-inc.com">michelle.rea@cambium-inc.com</a>. Please note, Cambium Inc. may contact you at a later date to request permission to monitor the water level in your well in the future.

You are not obligated to complete this form and participation on your part is voluntary. If you choose to provide a response to this letter, please do so before May 30, 2022.

If you have any questions regarding this assessment, please contact Michelle Rea at 1-705-957-3558.

Thank you. Best regards,

Cambium Inc.

Cameron MacDougall, P.Ge6.

**Project Manager** 

CJM

Attached: Water Well Survey Questionnaire

14288-003 Page 1



# CAMBIUM INC. BASELINE ASSESSMENT OF RESIDENTIAL WATER SUPPLY

Resident/Owner:	Info. Provided By:	
Address:		
		_
Phone: Home	Work	
Email Address:		
Part I: Well Construction Detai	<u>ils</u>	
Location of Well:		
Well Record Number (i.e., tag on we	ell)	
Record Available?:(attack	ch copy) Construction Date:	
Well Depth (m):	Diameter (cm):	
Casing Length (m):	Diameter (cm):	
Screen Installed?	Depth to Water From Ground Surface (m)	
Details (slot size, diameter, length, d	depth)	
Depth to Bedrock:	Bedrock Type:	
Part II: Pump Installation Detai	<u>ils</u>	
Pump Type (submersible, centifugal,	, jet, etc.):	
Manufacturer/Model No.:	Power:	
Design Pumping Rate (units):	Design Head (m):	
Setting Depth (m):	Discharge Line (materials, diameter):	
Pitless Adaptor (type, depth) :		
Storage Details (pressure or holding	tanks, filters or other treatment, operating pressures, etc.):	
		_



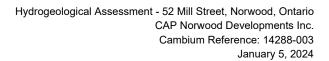
## CAMBIUM INC. BASELINE ASSESSMENT OF RESIDENTIAL WATER SUPPLY

### Part III: Groundwater Usage

What is groundwater used for (specify for each well)?				
Water quantity (problems, amounts)				
Water Discharge (septic system, settling ponds, other surface water, age, location, etc.)				
Water Quality Tested ?: (attach results if available)				
Water quality (odour, taste, colour, hardness)				
Diagram:				

### Residential survey contact addresses

					Contacted	Contacted
#	Address	City	Province	Postal Code	in April	in May
24	Mill St	Norwood	Ontario	KOL 2VO	х	Х
26	Mill St	Norwood	Ontario	KOL 2VO	х	х
28	Mill St	Norwood	Ontario	KOL 2VO	х	х
36	Mill St	Norwood	Ontario	KOL 2VO	х	х
67	Mill St	Norwood	Ontario	KOL 2VO	х	х
76	Mill St	Norwood	Ontario	KOL 2VO	х	х
78	Mill St	Norwood	Ontario	KOL 2VO	х	х
90	Mill St	Norwood	Ontario	KOL 2VO	х	Х
96	Mill St	Norwood	Ontario	KOL 2VO	х	х
102	Mill St	Norwood	Ontario	KOL 2VO	х	х
106	Mill St	Norwood	Ontario	KOL 2VO	х	х
112	Mill St	Norwood	Ontario	KOL 2VO	x	Х
2363	Asphodel 10th Line	Norwood	Ontario	KOL 2VO	х	х
2366	Asphodel 10th Line	Norwood	Ontario	KOL 2VO	х	х
2367	Asphodel 10th Line	Norwood	Ontario	KOL 2VO	x	Х
2370	Asphodel 10th Line	Norwood	Ontario	KOL 2VO	х	х
2371	Asphodel 10th Line	Norwood	Ontario	KOL 2VO	x	Х
2413	Asphodel 10th Line	Norwood	Ontario	KOL 2VO	х	х
2447	Asphodel 10th Line	Norwood	Ontario	KOL 2VO	Х	Х
2450	Asphodel 10th Line	Norwood	Ontario	KOL 2VO		х
2319	Asphodel 10th Line	Norwood	Ontario	KOL 2VO		х





	Ap	pendix E
Certificate	of Laboratory	Analysis



#### SGS Canada Inc.

P.O. Box 4300 - 185 Concession St. Lakefield - Ontario - KOL 2HO

Phone: 705-652-2000 FAX: 705-652-6365

#### Cambium Inc.

Attn: Cameron MacDougall

194 Sofia Street Peterborough, ON K9H 1E3, Canada

Phone: 705-742-7900 Fax:705-742-7907

**Project:** 14258-003

11-May-2022

**Date Rec.:** 04 May 2022 LR Report: CA17977-MAY22 Reference: 14288-003, Cameron

MacDougall

Copy: Final #1

## CERTIFICATE OF ANALYSIS Final Report

Analysis	1:	2:	3:	4:	6:	7:
	Analysis Start Date	Analysis Start Time	Analysis Completed Date	Analysis Completed Time	MW105-22	MW115-22
Sample Date & Time					04-May-22 13:45	04-May-22 14:30
Temp Upon Receipt [°C]	***	***	***	***	***	***
TSS [mg/L]	05-May-22	16:19	06-May-22	13:48	2020	3260
CI [mg/L]	10-May-22	16:39	11-May-22	13:59	9	18
NH3+NH4 [as N mg/L]	04-May-22	22:31	05-May-22	10:36	< 0.1	< 0.1
NO2 [as N mg/L]	10-May-22	11:44	11-May-22	10:08	< 0.03	0.06
NO3 [as N mg/L]	10-May-22	11:44	11-May-22	10:08	18.9	23.4
NO2+NO3 [as N mg/L]	10-May-22	11:44	11-May-22	10:08	18.9	23.5
Al (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.241	0.187
Al (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	9.52	3.56
Sb (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	< 0.0009	< 0.0009
Sb (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	< 0.0009	< 0.0009
As (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	< 0.0002	< 0.0002
As (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.0019	0.0009
Ba (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.0251	0.0291
Ba (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.122	0.0724
Be (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.000015	0.000021
Be (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.000449	0.000161
B (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.018	0.035
B (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.030	0.017
Bi (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	< 0.00001	< 0.00001
Bi (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00004	0.00002
Cd (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	< 0.000003	< 0.000003
Cd (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.000046	0.000030
Ca (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	125	128
Ca (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	242	172
Cr (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.0111	0.00400



#### SGS Canada Inc.

P.O. Box 4300 - 185 Concession St. Lakefield - Ontario - KOL 2HO

Phone: 705-652-2000 FAX: 705-652-6365

**Project:** 14258-003

LR Report : CA17977-MAY22

Analysis	1: Analysis Start Date	2: Analysis Start Time	3: Analysis Completed Date	4: Analysis Completed Time	6: MW105-22	7: MW115-22
Cr (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00067	0.00050
Co (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.000489	0.000285
Co (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00543	0.00164
Cu (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.0014	0.0011
Cu (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.0109	0.0042
Fe (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.235	0.177
Fe (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	6.89	2.82
Pb (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00042	0.00032
Pb (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00883	0.00244
Li (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.0006	0.0008
Li (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.0083	0.0034
Mg (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	4.78	3.53
Mg (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	8.40	4.57
Mn (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.107	0.05963
Mn (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.897	0.137
Mo (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00013	0.00018
Mo (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00055	0.00027
Ni (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.0007	0.0006
Ni (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.0093	0.0030
P (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.018	0.020
P (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.326	0.132
K (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	1.29	1.02
K (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	4.80	2.12
Se (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	< 0.00004	0.00009
Se (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00009	0.00007
Si (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	4.33	4.22
Si (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	23.2	12.9
Ag (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	< 0.00005	< 0.00005
Ag (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00012	< 0.00005
Na (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	5.72	3.11
Na (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	5.88	3.12
Sr (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.305	0.282
Sr (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.473	0.360
TI (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.000021	0.000016
TI (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.000232	0.000089
Sn (diss) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00026	0.00048
Sn (tot) [mg/L]	09-May-22	18:57	11-May-22	16:01	0.00180	0.00146
Ti (diss) [mg/L]	09-May-22	18:57	11-May-22	16:02	0.0107	0.00870
Ti (tot) [mg/L]	09-May-22	18:57	11-May-22	16:02	0.510	0.171
U (diss) [mg/L]	09-May-22	18:57	11-May-22	16:02	0.000439	0.000234
U (tot) [mg/L]	09-May-22	18:57	11-May-22	16:02	0.000705	0.000293
V (diss) [mg/L]	09-May-22	18:57	11-May-22	16:02	0.00073	0.00052



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**Project:** 14258-003

LR Report : CA17977-MAY22

Analysis	1: Analysis Start Date	2: Analysis Start Time	3: Analysis Completed Date	4: Analysis Completed Time	6: MW105-22	7: MW115-22
V (tot) [mg/L]	09-May-22	18:57	11-May-22	16:02	0.0145	0.00492
Zn (diss) [mg/L]	09-May-22	18:57	11-May-22	16:02	< 0.002	< 0.002
Zn (tot) [mg/L]	09-May-22	18:57	11-May-22	16:02	0.026	0.014
Zr (diss) [mg/L]	09-May-22	18:57	11-May-22	16:02	< 0.002	< 0.002
Zr (tot) [mg/L]	09-May-22	18:57	11-May-22	16:02	0.007	0.004

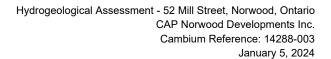
Note: Unionized ammonia calculated using lab results for pH and temperature.

Temperature of Sample upon Receipt: 11 degrees C Cooling Agent Present: Yes Custody Seal Present: Yes

Chain of Custody Number: N/A

Jill Cumpbell

Jill Campbell, B.Sc., GISAS Project Specialist, Environment, Health & Safety





Appendix F
Aquifer Test Pro Results



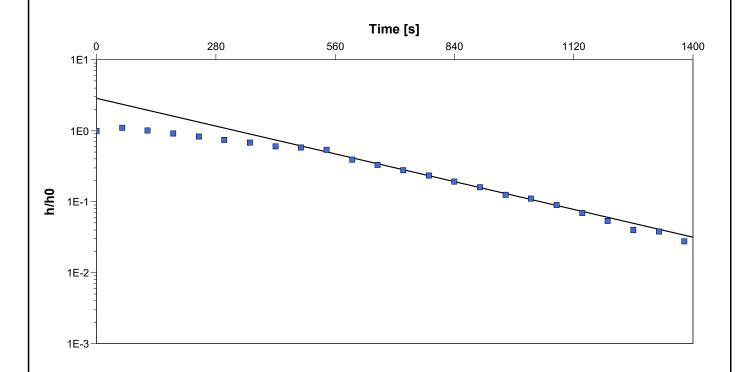
Slug Test Analysis Report

Project: Hydrogeological Assessment

Number: 14288-003

Client: CAP Norwood Development

Location: 52 Mill Street, Norwood	Slug Test: MW105-22 - Slug Test 1	Test Well: MW105-22
Test Conducted by: J. Munro		Test Date: 5/4/2022
Analysis Performed by: N. Heikoop	MW105-22 - Slug Test 1	Analysis Date: 6/16/2022



Calculation	ucina	Hyordoy
Calculation	usina	Hvorsiev

Observation Well	Hydraulic Conductivity	
	[m/s]	
MW105-22	1.61 × 10 <sup>-6</sup>	



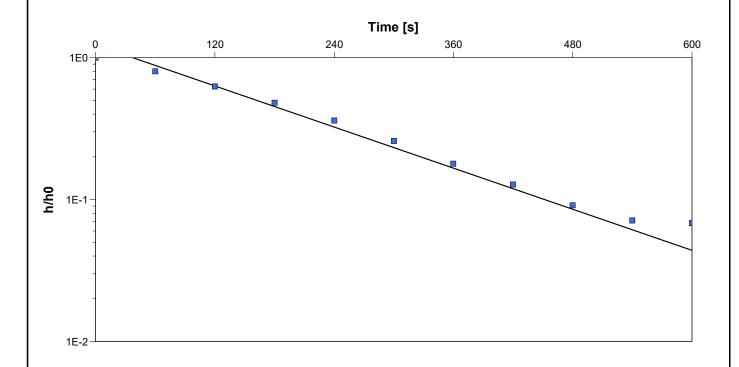
Slug Test Analysis Report

Project: Hydrogeological Assessment

Number: 14288-003

Client: CAP Norwood Development

Location: 52 Mill Street, Norwood	Slug Test: MW105-22 - Slug Test 2	Test Well: MW105-22
Test Conducted by: J. Munro		Test Date: 5/4/2022
Analysis Performed by: N. Heikoop	MW105-22 - Slug Test 2	Analysis Date: 6/16/2022



Calculation	ucina	Hyorc	ار دا
Calculation	usiiiu	1111015	LΕV

Observation Well	Hydraulic Conductivity	
	[m/s]	
MW105-22	2.79 × 10 <sup>-6</sup>	



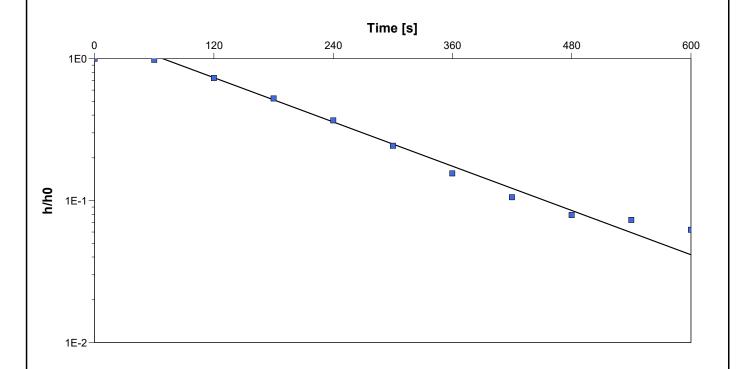
Slug Test Analysis Report

Project: Hydrogeological Assessment

Number: 14288-003

Client: CAP Norwood Development

Location: 52 Mill Street, Norwood	Slug Test: MW105-22 - Slug Test 3	Test Well: MW105-22
Test Conducted by: J. Munro		Test Date: 5/4/2022
Analysis Performed by: N. Heikoop	MW105-22 - Slug Test 3	Analysis Date: 6/16/2022



Calculation	ucina	Hyordoy
Calculation	usina	Hvorsiev

Observation Well	Hydraulic Conductivity	
	[m/s]	
MW105-22	3.00 × 10 <sup>-6</sup>	



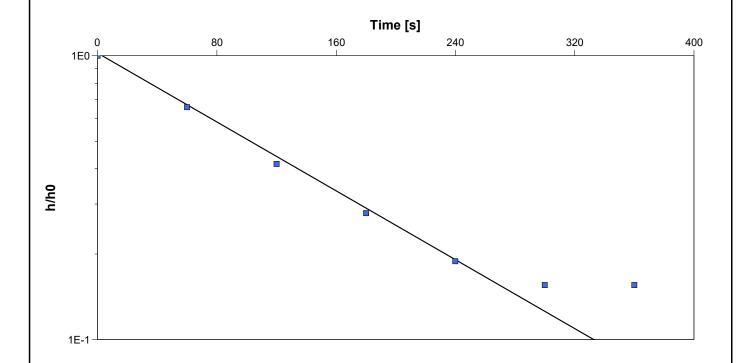
Slug Test Analysis Report

Project: Hydrogeological Assessment

Number: 14288-003

Client: CAP Norwood Development

Location: 52 Mill Street, Norwood	Slug Test: MW105-22 - Slug Test 4	Test Well: MW105-22
Test Conducted by: J. Munro		Test Date: 5/4/2022
Analysis Performed by: N. Heikoop	MW105-22 - Slug Test 4	Analysis Date: 6/16/2022



Calculation	unina	Llygraloy
Calculation	usina	Hvorsiev

Observation Well	Hydraulic Conductivity	
	[m/s]	
MW105-22	3.50 × 10 <sup>-6</sup>	



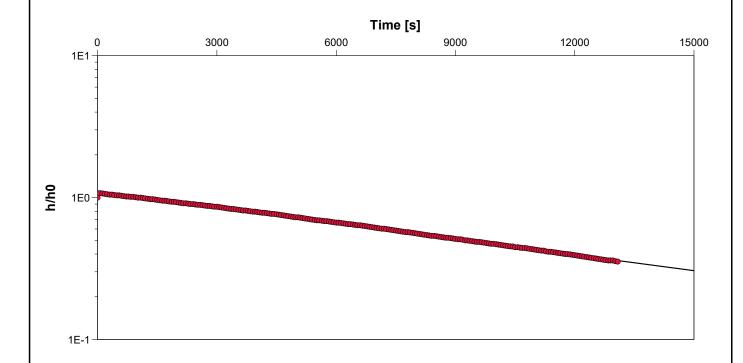
Slug Test Analysis Report

Project: Hydrogeological Assessment

Number: 14288-003

Client: CAP Norwood Development

Location: 52 Mill Street, NorwoodSlug Test: MW112-22 - Slug Test 1Test Well: MW112-22Test Conducted by: J. MunroTest Date: 5/4/2022Analysis Performed by: N. HeikoopMW112-22 - Slug Test 1Analysis Date: 6/16/2022



Calculation	ucina	Hyordoy
Calculation	usina	Hvorsiev

Observation Well	Hydraulic Conductivity	
	[m/s]	
MW112-22	7.35 × 10 <sup>-8</sup>	



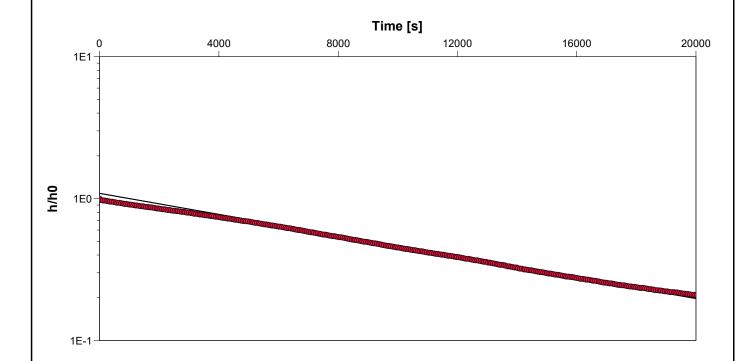
Slug Test Analysis Report

Project: Hydrogeological Assessment

Number: 14288-003

Client: CAP Norwood Development

Location: 52 Mill Street, Norwood	Slug Test: MW112-22 - Slug Test 2	Test Well: MW112-22
Test Conducted by: J. Munro		Test Date: 5/4/2022
Analysis Performed by: N. Heikoop	MW112-22 - Slug Test 2	Analysis Date: 6/16/2022



Calculation using Hvorslev		
Observation Well	Hydraulic Conductivity	
	[m/s]	
MW112-22	7.32 × 10 <sup>-8</sup>	



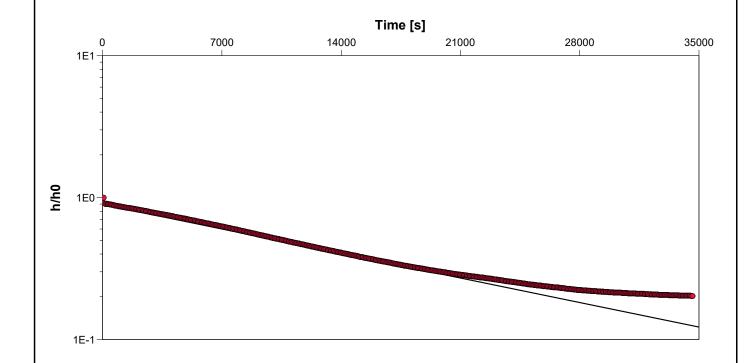
Slug Test Analysis Report

Project: Hydrogeological Assessment

Number: 14288-003

Client: CAP Norwood Development

Location: 52 Mill Street, Norwood	Slug Test: MW112-22 - Slug Test 3	Test Well: MW112-22
Test Conducted by: J. Munro		Test Date: 5/4/2022
Analysis Performed by: N. Heikoop	MW112-22 - Slug Test 3	Analysis Date: 6/16/2022



Calculation	ucina	Hyordoy
Calculation	usina	Hvorsiev

Observation Well	Hydraulic Conductivity	
	[m/s]	
MW112-22	4.88 × 10 <sup>-8</sup>	



Slug Test Analysis Report

Project: Hydrogeological Assessment

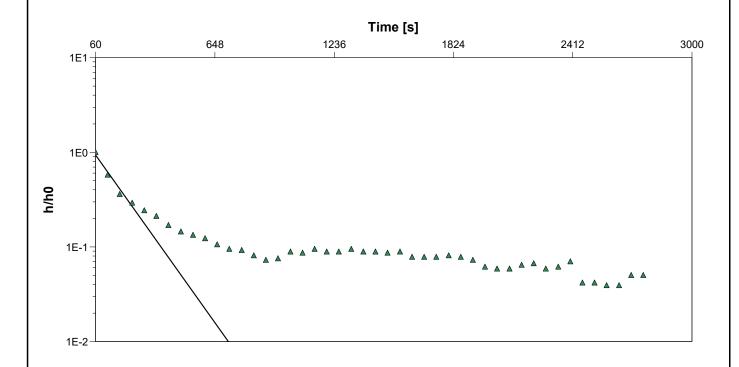
Number: 14288-003

Client: CAP Norwood Development

Location: 52 Mill Street, Norwood Slug Test: MW115-22 - Slug Test 1 Test Well: MW115-22

Test Conducted by: J. Munro Test Date: 5/4/2022

Analysis Performed by: N. Heikoop MW115-22 - Slug Test 1 Analysis Date: 6/16/2022



<b>~</b> · · · · ·		
Calculation	usina	Hvorslev

Observation Well	Hydraulic Conductivity	
	[m/s]	
MW115-22	5.95 × 10 <sup>-6</sup>	



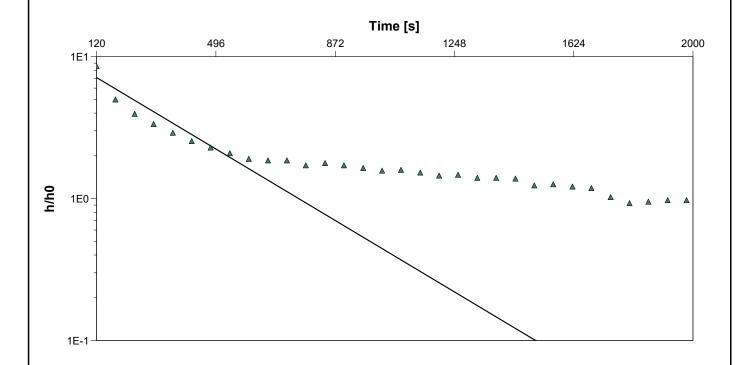
Slug Test Analysis Report

Project: Hydrogeological Assessment

Number: 14288-003

Client: CAP Norwood Development

Location: 52 Mill Street, Norwood	Slug Test: MW115-22 - Slug Test 2	Test Well: MW115-22
Test Conducted by: J. Munro		Test Date: 5/4/2022
Analysis Performed by: N. Heikoop	MW115-22 - Slug Test 2	Analysis Date: 6/16/2022



Calculation	ucina	Hyordoy
Calculation	usina	Hvorsiev

Observation Well	Hydraulic Conductivity	
	[m/s]	
MW115-22	2.64 × 10 <sup>-6</sup>	



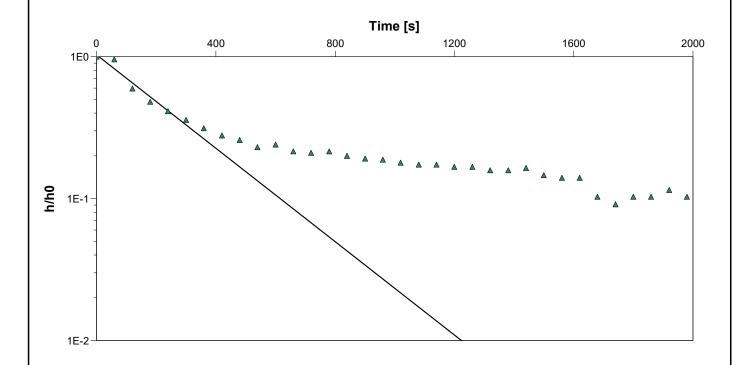
Slug Test Analysis Report

Project: Hydrogeological Assessment

Number: 14288-003

Client: CAP Norwood Development

Location: 52 Mill Street, NorwoodSlug Test: MW115-22 - Slug Test 3Test Well: MW115-22Test Conducted by: J. MunroTest Date: 5/4/2022Analysis Performed by: N. HeikoopMW115-22 - Slug Test 3Analysis Date: 6/16/2022



Calculation	ucina	Hyordoy
Calculation	usina	Hvorsiev

Observation Well	Hydraulic Conductivity	
	[m/s]	
MW115-22	3.25 × 10 <sup>-6</sup>	



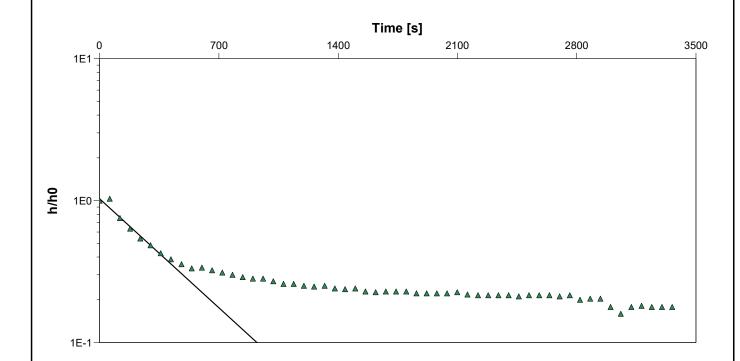
Slug Test Analysis Report

Project: Hydrogeological Assessment

Number: 14288-003

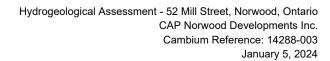
Client: CAP Norwood Development

Location: 52 Mill Street, Norwood	Slug Test: MW115-22 - Slug Test 4	Test Well: MW115-22
Test Conducted by: J. Munro		Test Date: 5/4/2022
Analysis Performed by: N. Heikoop	MW115-22 - Slug Test 4	Analysis Date: 6/16/2022



Calculation	ucina	Hyordoy
Calculation	usina	Hvorsiev

Observation Well	Hydraulic Conductivity	
	[m/s]	
MW115-22	2.17 × 10 <sup>-6</sup>	





		Α	ppendix	G
MECP	Well	Record	Summa	ıry

## Water Well Records Summary Report

Produced by Cambium Inc. using MOECC Water Well Information System (WWIS)

All units in meters unless otherwise specified



Well ID: 5100148 Construction Date: 12/3/1959	<b>Easting:</b> 263546 <b>Northing:</b> 4919090	UTM Zone 18 Positional Accuracy: margin of error:	100 m - 300 m
	Well Depth: 12.2 Water First Found: 9.75 Static Level: 6	Water Kind FRESH Final Status Water Supply Primary Water Use: Domestic	Pump Rate (LPM): 0 Recommended Pump Rate: 0 Pumping Duration (h:m): 5:0
	Layer: Driller's Description:	Top: Bottom:	
	1 CLAY	0 7.92	
	2 LIMESTONE	7.92 12.2	
Well ID: 5100151 Construction Date: 5/19/1952	<b>Easting:</b> 263282 <b>Northing:</b> 4920013	UTM Zone 18 Positional Accuracy: unknown UTM	
	Well Depth: 11 Water First Found: 10.7 Static Level: 8	Water Kind FRESH Final Status Water Supply Primary Water Use: Domestic	Pump Rate (LPM): 0 Recommended Pump Rate: Pumping Duration (h:m): 24:0
	Layer: Driller's Description:	Top: Bottom:	, ,
	1 MEDIUM SAND	0 11	
Well ID: 5100152 Construction Date: 9/19/1952	<b>Easting:</b> 263040 <b>Northing:</b> 4919685	UTM Zone 18 Positional Accuracy: unknown UTM	
	Well Depth: 12.2 Water First Found: 10.7 Static Level: 3	Water Kind FRESH Final Status Water Supply Primary Water Use: Public	Pump Rate (LPM): 23 Recommended Pump Rate: Pumping Duration (h:m): 3:0
	Layer: Driller's Description:  1 MEDIUM SAND	<b>Top: Bottom:</b> 0 10.7	
	2 GRAVEL	10.7 12.2	
Well ID: 5100153 Construction Date: 12/28/1954	Easting: 263222 Northing: 4919869	UTM Zone 18 Positional Accuracy: unknown UTM	
	Well Depth: 12.8 Water First Found: 12.8 Static Level: 10	Water Kind FRESH Final Status Water Supply Primary Water Use: Commerical	Pump Rate (LPM): 23 Recommended Pump Rate: Pumping Duration (h:m): 2:0
	Layer: Driller's Description: 1 TOPSOIL	<b>Top: Bottom:</b> 0 0.30	
	2 MEDIUM SAND	0.30 11.6	
	3 CLAY	11.6 12.8	
Well ID: 5100154 Construction Date: 11/14/1955	<b>Easting:</b> 263344 <b>Northing:</b> 4920056	UTM Zone 18 Positional Accuracy: unknown UTM	
	Well Depth: 6.4 Water First Found: 5.79 Static Level: 3	Water Kind FRESH Final Status Water Supply Primary Water Use: Domestic	Pump Rate (LPM): 14 Recommended Pump Rate: Pumping Duration (h:m): 0:45
	Layer: Driller's Description:  1 MEDIUM SAND	<b>Top: Bottom:</b> 0 3.66	
	2 LIMESTONE	3.66 6.40	

Well ID: 5100156 Construction Date: 6/2/1959	<b>Easting:</b> 263343 <b>Northing:</b> 4920026	UTM Zone 18 Positional Accuracy: margin of error: 100 m - 300 m				
	Well Depth: 16.5 Water First Found: 15.2 Static Level: 10	Water Kind Final Status Primary Water Use:	Water Supply	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	27 3:0	
	Layer: Driller's Description: 1 TOPSOIL	<b>Top: Bottom:</b> 0 0.91				
	2 GRAVEL	0.91 10.7				
	3 MEDIUM SAND	10.7 11.9				
	4 LIMESTONE	11.9 16.5				
Well ID: 5100157 Construction Date: 6/6/1960	<b>Easting:</b> 263053 <b>Northing:</b> 4919765	UTM Zone 18 Positional Accuracy:	margin of error : 10	0 m - 300 m		
	Well Depth: 51.8 Water First Found: 51.8 Static Level: 29	Water Kind Final Status Primary Water Use:	Water Supply	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	23 23 1:0	
	Layer: Driller's Description:  1 MEDIUM SAND	<b>Top: Bottom:</b> 0 13.4				
	2 LIMESTONE	13.4 51.8				
Well ID: 5100158 Construction Date: 1/3/1966	<b>Easting:</b> 263159 <b>Northing:</b> 4919796	UTM Zone 18 Positional Accuracy:	margin of error : 10	0 m - 300 m		
	Well Depth: 12.2 Water First Found: 11.6 Static Level: 8	Water Kind Final Status Primary Water Use:	Water Supply	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	23 23 1:0	
	Layer: Driller's Description:	Top: Bottom:				
	1 TOPSOIL	0 1.22				
	<ul><li>2 MEDIUM SAND</li><li>3 LIMESTONE</li></ul>	1.22 11 11 12.2				
Well ID: 5100159 Construction Date: 2/14/1967	<b>Easting:</b> 262857 <b>Northing:</b> 4919335	UTM Zone 18 Positional Accuracy:	margin of error : 10	0 m - 300 m		
	Well Depth: 7.92 Water First Found: 7.62 Static Level: 2	Water Kind Final Status Primary Water Use:	Water Supply	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	23 23 2:0	
	Layer: Driller's Description:	Top: Bottom:				
	1 TOPSOIL	0 0.30				
	2 CLAY 3 GRAVEL	0.30 7.62 7.62 7.92				
		7.42 7.32				
Well ID: 5100160 Construction Date: 8/29/1967	Easting: 263210 Northing: 4919815	UTM Zone 18 Positional Accuracy:	margin of error : 10	0 m - 300 m		
	Well Depth: 11 Water First Found: 10.7 Static Level: 8	Water Kind Final Status Primary Water Use:	Water Supply	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	23 <b>23</b> 0:30	
	Layer: Driller's Description: 1 PREVIOUSLY DUG	<b>Top: Bottom:</b> 0 4.88				
	2 MEDIUM SAND	4.88 11				

Well ID: 5100163 Construction Date: 9/6/1966	<b>Easting:</b> 263981 <b>Northing:</b> 4919757	UTM Zone 18 Positional Accuracy: margin of error: 100 m - 300 m
	Well Depth: 18.3 Water First Found: 10.7 Static Level: 6	Water KindFRESHPump Rate (LPM):5Final StatusWater SupplyRecommended Pump Rate:5Primary Water Use:DomesticPumping Duration (h:m):1:30
	Layer: Driller's Description:  1 MEDIUM SAND	: <b>Top: Bottom:</b> 0 5.18
	2 LIMESTONE	5.18 16.8
	3 GRANITE	16.8 18.3
Well ID: 5109119 Construction Date: 8/16/1978	<b>Easting:</b> 263080 <b>Northing:</b> 4920072	UTM Zone 18 Positional Accuracy: margin of error: 100 m - 300 m
	Well Depth: 12.2 Water First Found: 12.2 Static Level: 9	Water KindFRESHPump Rate (LPM):45Final StatusWater SupplyRecommended Pump Rate:45Primary Water Use:DomesticPumping Duration (h:m):3:0
	Layer: Driller's Description:	: Top: Bottom:
	1 SAND	0 11
	2 LIMESTONE	11 12.2
Well ID: 5109754 Construction Date: 6/2/1980	<b>Easting:</b> 264030 <b>Northing:</b> 4919322	UTM Zone 18 Positional Accuracy: margin of error: 100 m - 300 m
	Well Depth: 21.3 Water First Found: 19.8 Static Level: 5	Water KindFRESHPump Rate (LPM):23Final StatusWater SupplyRecommended Pump Rate:23Primary Water Use:DomesticPumping Duration (h:m)::
	Layer: Driller's Description:  1 PREVIOUSLY DUG	•
	2 LIMESTONE	3.66 21.3
Well ID: 5110843 Construction Date: 9/28/1983	<b>Easting:</b> 263230 <b>Northing:</b> 4919922	UTM Zone 18 Positional Accuracy: margin of error: 100 m - 300 m
	Well Depth: 12.2 Water First Found: 11.9 Static Level: 9	Water Kind Not stated Pump Rate (LPM): 45 Final Status Water Supply Recommended Pump Rate: 36 Primary Water Use: Commerical Pumping Duration (h:m): 10:0
	Layer: Driller's Description:	
	1 GRAVEL	0 11.3
	1 GRAVEL 2 LIMESTONE	0 11.3 11.3 12.2
	2 LIMESTONE	11.3 12.2
Well ID: 5111528 Construction Date: 11/1/1985	<b>Easting:</b> 263075 <b>Northing:</b> 4919905	UTM Zone 18 Positional Accuracy: unknown UTM
	Well Depth: 25.6 Water First Found: 22.9 Static Level: 5	Water KindNot statedPump Rate (LPM):5Final StatusWater SupplyRecommended Pump Rate:5Primary Water Use:DomesticPumping Duration (h:m):3 :
	Layer: Driller's Description	
	1 TOPSOIL	0 0.61
	2 SAND	0.61 9.14
	3 LIMESTONE	9.14 25.6

Well ID: 5115821 Construction Date: 5/5/1992	Easting: 7	263264 : 4919324	UTM Zone Positional		unknown UTM		
	Well Dep Water Fir Static Lev	st Found: 26.5	Water Kin Final Statu Primary W	us	FRESH Test Hole Commerical	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	364 364 11:0
	Layer: I	Driller's Description:	<b>Top:</b> 0	Bottom: 7.62			
	2	GRAVEL	7.62	13.7			
	3	GRAVEL	13.7	26.5			
	4	GRAVEL	26.5	26.8			
	5	SHALE	26.8	27.1			
Well ID: 5116541 Construction Date: 3/8/1994	Easting: 7	263451 : 4918748	UTM Zone	_	unknown UTM		
	Well Dep Water Fir Static Lev	st Found: 3.35	Water Kin Final Statu Primary V	us	FRESH Abandoned-Su Municipal	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	227 24:0
	Layer:	Driller's Description:	Top:	Bottom:	·		
	1	FINE SAND	0	2.44			
	2	GRAVEL	2.44	3.66			
	3	COARSE GRAVEL	3.66	9.75			
	4	GRAVEL	9.75	11			
Well ID: 5116818 Construction Date: 2/15/1995	<b>Easting:</b> 263075 <b>Northing:</b> 4919905		UTM Zone 18 Positional Accuracy: unknown UTM		unknown UTM		
	Well Dep Water Fir Static Lev	st Found: 21.3	Water Kin Final Statu Primary W		FRESH Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	27 23 1:0
	Layer:	Driller's Description:	Тор:	Bottom:			
	1	TOPSOIL	0	0.30			
	1	TOPSOIL	0	0.30			
	2	CLAY	0.30	9.14			
	2	CLAY	0.30	9.14			
	3	SHALE	9.14	9.45			
	3	SHALE	9.14	9.45			
	4	LIMESTONE	9.45	22			
	4	LIMESTONE	9.45	22			
Well ID: 7047958 Construction Date: 8/9/2007	Easting: 7	263996 : 4919669	UTM Zone Positional		margin of error : :	10 - 30 m	
	Well Dep Water Fir Static Lev	st Found: 24.7	Water Kin Final Statu Primary W		FRESH Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	23 23 1:
	Layer: 1	Driller's Description: TOPSOIL	<b>Top:</b> 0	Bottom: 0.15			
	1	TOPSOIL	0	0.15			
	1	1013012	Ü	0.13			

	2	SAND	0.15	1.22		
	3	CLAY	1.22	7.92		
	3	CLAY	1.22	7.92		
	4	SHALE	7.92	9.14		
	4	SHALE	7.92	9.14		
	5	LIMESTONE	9.14	25.9		
	5	LIMESTONE	9.14	25.9		
Well ID: 7110601 Construction Date: 8/28/2008	Easting: 26		UTM Zone Positional	_	margin of error : 1	1.0 - 30 m
	Well Depth Water First Static Leve	t Found: 41.2	Water Kind Final Status Primary Wa	S	Untested Water Supply Domestic	Pump Rate (LPM): 23 Recommended Pump Rate: 23 Pumping Duration (h:m): 1:
	Layer: Di	riller's Description:	Тор:	Bottom:		
	1	SAND	0	5.49		
	1	SAND	0	5.49		
	2	LIMESTONE	5.49	42.7		
	2	LIMESTONE	5.49	42.7		
Well ID: 7186311 Construction Date: 9/4/2012	Easting: 26		UTM Zone Positional		margin of error : 3	30 m - 100 m
	Well Depth Water First Static Leve	Found:	Water Kind Final Status Primary Wa	S	Observation W Monitoring an	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):
	Layer: Di	riller's Description:	Тор:	Bottom:		
	1	FILL	0	0.61		
	2	SAND	0.61	2.44		
	3	SAND	2.44	4.57		
Well ID: 7186312 Construction Date: 9/4/2012	Easting: 26		UTM Zone Positional		margin of error : 3	30 m - 100 m
	Well Depth Water First Static Leve	Found:	Water Kind Final Status Primary Wa	S	Observation W Monitoring an	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):
	-	riller's Description:	Top:	Bottom:		
	1	GRAVEL	0	1.5		
	-					
	1	GRAVEL	0	1.5		
Well ID: 7186313 Construction Date: 9/4/2012	1 2	GRAVEL FILL SAND	0 1.5 3.1	1.5 3.1 4.57	margin of error : 3	30 m - 100 m
	1 2 3 Easting: 26	GRAVEL FILL SAND 52855 4919091 a: 4.57 t Found:	0 1.5 3.1	1.5 3.1 4.57 18 Accuracy:	margin of error : 3 Test Hole Monitoring an	80 m - 100 m  Pump Rate (LPM):  Recommended Pump Rate:  Pumping Duration (h:m):
	2 3 Easting: 26 Northing: 4 Well Depth Water First Static Leve	GRAVEL FILL SAND 52855 4919091 a: 4.57 t Found:	0 1.5 3.1  UTM Zone Positional A Water Kinc	1.5 3.1 4.57 18 Accuracy:	Test Hole	Pump Rate (LPM): Recommended Pump Rate:
	2 3  Easting: 26 Northing: 4 Well Depth Water First Static Leve Layer: De	GRAVEL FILL SAND  52855 4919091 n: 4.57 t Found: l: riller's Description:	0 1.5 3.1  UTM Zone Positional A Water Kinc Final Status Primary Water Top:	1.5 3.1 4.57  18 Accuracy: I s ater Use: Bottom:	Test Hole	Pump Rate (LPM): Recommended Pump Rate:

Well ID: 7189653 Construction Date: 10/16/2012	<b>Easting:</b> 263990 <b>Northing:</b> 4919241			UTM Zone 18 Positional Accuracy: margin of error: 30 m - 100 m				
	Well De Water I Static L	First Found: 27.4	Water Kin Final State Primary V		Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	16 <b>16</b> 1:	
	Layer: 1 2	<b>Driller's Descripti</b> SAND LIMESTONE	0	3.05 30.5	:			
Well ID: 7189660 Construction Date: 10/16/2012	_	: 263875 ng: 4919869	UTM Zone		margin of error :	30 m - 100 m		
	Well De Water I Static L	First Found: 12.2	Water Kin Final Stati Primary V	ıs	Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	16 <b>16</b> 1:	
	Layer:	Driller's Descripti	-	Bottom:	:			
	1	SAND SAND	0	8.53 8.53				
	2	LIMESTONE		18.3				
	2	LIMESTONE		18.3				
Well ID: 7281327 Construction Date: 2/17/2017	_	: 263300 ng: 4919946	UTM Zone		margin of error :	30 m - 100 m		
	Well De Water I Static L	First Found: 7.62	Water Kin Final State Primary V	ıs	Untested Monitoring an Test Hole	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):		
	Layer:	Driller's Descripti	on: Top:	Bottom: 0.61				
	2	SAND	0.61	10.7				
Well ID: 7294205 Construction Date: 9/6/2017		: 263944 ng: 4919330	UTM Zone		margin of error :	30 m - 100 m		
	Well De Water I Static L	First Found: 26.8	Water Kin Final State Primary V		Untested Water Supply Domestic	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):	16 <b>16</b> 1:0	
	Layer:	Driller's Descripti TOPSOIL	on: Top:	Bottom: 3.66	:			
	2	LIMESTONE	3.66	30.5				
<b>Well ID:</b> 7294797 <b>Construction Date:</b> 9/15/2017	_	: 263042 ng: 4919872	UTM Zone Positional		margin of error :	30 m - 100 m		
	Well De Water I Static L	First Found:	Water Kin Final State Primary V	ıs	Test Hole Test Hole	Pump Rate (LPM): Recommended Pump Rate: Pumping Duration (h:m):		
	Layer:	<b>Driller's Descripti</b> SAND	<b>on: Top:</b> 0	Bottom: 1.22	:			
	2	BOULDERS	1.22	22.9				
			1.22 22.9	22.9 25.3				

Well ID: 7294798 Easting: 263010 UTM Zone 18 Construction Date: 9/15/2017 Northing: 4919733 Positional Accuracy: margin of error: 30 m - 100 m Well Depth: **Water Kind** Pump Rate (LPM): **Water First Found: Final Status** Observation W **Recommended Pump Rate: Static Level:** Primary Water Use: Test Hole Pumping Duration (h:m): **Driller's Description:** Layer: Top: **Bottom:** SAND 1 0 2 SHALE 3 LIMESTONE Well ID: 7294799 Easting: 262944 UTM Zone 18 Positional Accuracy: margin of error: 30 m - 100 m Construction Date: 9/15/2017 Northing: 4919693 Well Depth: **Water Kind** Pump Rate (LPM): Water First Found: **Final Status** Observation W **Recommended Pump Rate:** Static Level: Primary Water Use: Test Hole Pumping Duration (h:m): **Driller's Description:** Layer: Top: **Bottom:** SAND 1 0 2 SHALE 3 LIMESTONE Well ID: 7294800 **Easting: 262818** UTM Zone 18 Positional Accuracy: margin of error: 30 m - 100 m Construction Date: 9/15/2017 Northing: 4919738 Well Depth: **Water Kind** Pump Rate (LPM): **Water First Found: Final Status Recommended Pump Rate:** Test Hole Static Level: Primary Water Use: Test Hole Pumping Duration (h:m): Layer: **Driller's Description:** Top: **Bottom:** 1 **SAND** 0

2

3

SHALE

LIMESTONE



51 Nº

149

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Elev.	15/1x	1011	10	P	1
107	12/1	1 1			,

The Ontario Water Resources Commission Act, 1957

## WATER WELL RECORD

on. 9 Lot	18 17 Town	Date co	mpleted 19	October	1959
		ress	NORWOOD,		year)
Coomy and percent Recon	u .		Pu	mping Test	
nside diameter of casing 61. L.D	•	Static	level None		
Total length of casing			umping rate		
Type of screen			ng level		
ength of screen		. Durati	on of test pumpin	s コノバ	
Depth to top of screen		Water	clear or cloudy at	end of test. No	water.
Diameter of finished hole 61 I.D	A	. Recom	mended pumping	rateNone	G.P.1
		wit	h pumping level of	of <del></del>	*************************
Well Log			W	iter Record	
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	No. of feet water rises	Kind of wate (fresh, salty, sulphur)
Sandy Clay and Boulders	0	26			-
Limestone	26	100	No water		
					1
			·		
Letter of Dec 1/59 from a	Willing A	irm			·
says casing was pul	100.				
	(1)				
		T	-		
r what purpose(s) is the water to be used?			Locat	ion of Well	
Domestic			In diagram below		
well on upland, in valley, or on hillside?		¥ 80	road and lot line.		
Upland			VERHEAD RAILWAY BR	10625	++++++++
illing Firm OTONABEE WATER WEL	IS ITD		"	SATELY	
dress INDIAN RIVER, ONTA			63	Mary.	
			RWC II	· M	
	••••••	1	* (344)	25/	11/1/2
ence Number3.78		5 CLF 3D ROUG	7	3/	
me of DrillerPaulBerthelot		27-	17	45//	
dress Indian River, O	nt	1	HASTINGS	"	8
0ctober 27, 195			15.73 84	CRPCRATION	ill.
Leven W. Shows	lint		[3]	<u>C</u> F	7.
(Signature of Licensed Drilling Contractor	)		1   ∄	NORWOOD	Lin
/		-			
Form 5		CONCES	SION		
		\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		CONCESSION	
				IX	Casasa

35 31 CSU WATER RESOURCES 5 VISION 9 OCT 19 1984 Ontario Water Resources Commission Act CHTARIO WATER RESOURCES COMMEN .Township, Village, Town or City... Date completed 3/ 20 Con..... Casing and Screen Record **Pumping Test** Inside diameter of casing... Static level ..... Total length of casing Test-pumping rate Type of screen Pumping level. Duration of test pumping..... Length of screen Water clear or cloudy at end of test dry hole Depth to top of screen Recommended pumping rate Diameter of finished hole with pump setting of feet below ground surface Well Log Water Record Depth(s) at which water(s) Kind of water From Overburden and Bedrock Record (fresh, salty, sulphur) found For what purpose(s) is the water to be used? Location of Well In diagram below show distances of well from road and lot line. Indicate north by arrow. Is well on upland, in valley, or on hillside? Upland Drilling or Boring Firm Reycraft + Tlaya Licence Number Name of Driller or Borer. (Signature of Licensed Drilling or Boring Contractor) laye brick form house Form 7 10M-62-1152 OWRC COPY

31050

# The Ontario Water Resources Commission Act WATER WELL RECORD

Water management in Or	nfario 1. PRINT ONLY IN SI 2. CHECK ⊠ CORRE	PACES PROVIDED CT BOX WHERE APPLICABLE	11	510578	10	0,2 CON.	22 23 24
COUNTY OR DISTRICT	no	CLS 100	CITY, TOWN, VILLAGE	3	g CON., BLOCK, TRACT, S	SURVEY, ETC.	0/8
, , , , , ,		0	# 2	non	/	DAY 20	MO 9 48-53
		1	9520 4	ELEVATION OF TOP	RC. BASIN CODE		III IV
	LO	G OF OVERBURD	EN AND BEDRO	CK MATERIA	LS (SEE INSTRUCTIONS)		47
GENERAL COLOUR	MOST COMMON MATERIAL	ľ	MATERIALS		GENERAL DESCRIPTION		DEPTH - FEET FROM TO
	COMMON MATERIAL	tan	mil				64
		Hard	had	4 -:		- /	47
		Times	tour	sch	1.12	*	4 45
					0		
11					Try		
					1 10.		
	1			1	may.		
			24 27				
			<u> </u>		200		
31 0904	192 1 1 000	7/14/1/1/100	G5 V5 111	لبلاتينا	لبلليينا ليلب	لتنبا ليلي	ا لىلىلىل
32	4 15	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	ليليليل	43	111 41111	111 111	75 80
41 WATER	RECORD		OPEN HOLE	RECORD	Z SIZE(5) OF OPENING	31-33 DIAMETER	34-38 LENGTH 39-40
WATER FOUND AT - FEET	RESH 3 SULPHUR	INSIDE MATERIAL INCHES	THICKNESS FRO	то то	MATERIAL AND TYPE	DEPT	INCHES FEET TH TO TOP 41-44 80 SCREEN
2 SA	ALTY 4 MINERAL	10-11 1 STEEL 2 GALVANIZE 3 CONCRETE	11/4	00/0	[N]		FEET
	CTY 4 MINERAL	4 OPEN HOL		20-23	DEPTH SET AT - FEET	G & SEALIN	(CEMENT GROUT,
275	SH 3 SULPHUR 24	2 GALVANIZE		2343730	FROM TO 10-13 14-17	MATERIAL AND THE	LEAD PACKER, ETC.)
25-28 1   FF 2   SA		4 OPEN HOL	26	27-30	18-21 22-25		No.
30-33 1 FF 2 SA		2 ☐ GALVANIZE 3 ☐ CONCRETE 4 ☐ OPEN HOL			26-29 30-33	80	
71 PUMPING TEST METHOD			OF PUMPING		LOCATION	OF WELL	
	BAILER WATER LEVEL 25	3843	15-16 17-18 HOURS MINS	LOT	HAGRAM BELOW SHOW DISTAN	NCES OF WELL FROM R	DOAD AND
STATIC LEVEL	PUMPING WITES	LEVELS DURING	RECOVERY UTES 60 MINUTES 35-37	101	LINE. INDICASE NORTH BY	ARROW.	·_
() FEET	PEET FEE	FEET .	FEET FEET		/ h		
Z IF FLOWING.	GPM.	WATER AT	EAR 2 CLOUDY		1	1	
RECOMMENDED PUMP	TYPE RECOMMENDED		DED 8 46-49		1	4	
50-53	GPM./FT. SPECI		i i	4 0	. 12	315	
FINAL	WATER SUPPLY OBSERVATION WEI		NSUFFICIENT SUPPLY		1	( ' -	
OF WELL	3 ☐ TEST HOLE 4 ☐ RECHARGE WELL	UNFINISHED	OOR GUALIT		F	かっか	
55-56	1 <b>3 - 50 - 50 - 50</b> - 50 - 50 - 50 - 50 - 5	5 COMMERCIAL 6 MUNICIPAL	y				/
WATER USE	3 ☐ IRRIGATION 4 ☐ INDUSTRIAL	7 ☐ PUBLIC SUPPLY 8 ☐ COOLING OR AIR (			1		
57	OTHER_		NOT USED			1	
METHOD	CABLE TOOL ROTARY (CONVENT		ND	i.		75	
OF DRILLING	3 ☐ ROTARY (REVERSE 4 ☐ ROTARY (AIR). 5 ☐ AIR PERCUSSION	9 ☐ DRIVIN		DBILLEDG BENAN	ks:farm.		
NAME OF WELL COS			LICENCE NUMBER	DATA	58 CONTRACTOR 5	59-62 DATE RECEIVED	63-68 80
O MODRESS	mine	3	4811	SOURCE DATE OF INSPE	CTION INSPECT	190	72
× 13 s	Lunne	ر ا	LICENCE NUMBER	REMARKS:		Km	
NAME OF DRILLER OF				F			PK
SIGNATURE OF CON	TRACTOR	SUBMISSION DAT	70 3570 N	OFFICE		1'~ 3	WI
OWRC CO	<del>/C1 / 12 / C</del>	J.			- North	*	



# The Ontario Water Resources Act WATER WELL RECORD

Ontario Env	ironment	SPACES PROVIDED 11	5116542  5,1,7,0,3	
COUNTY OR DISTRICT	2. CHECK 🗵 CORR	TOWNSHIP, BOROUGH CITY TOWN VILLAGE	CON BLOCK TRACT, SURVEY ETC	LOT 25-27
PETER	REYOUGH	VILLAGE OF N	WILL ASPHODEL	IX I/
OWNER (SURNAME FI	RST) 28-47	ADDRESS (NO. 100)	CUTARIO.	5 No 7 789
P.O.	ZONE EASTING	NORTHING RC	ELEVATION RC BASIN CODE	1 . " . 1 . " 1
21	1 12 12	1 17 16 24 25	26 30 31 31 30 31 31 30 31 30 31 30 31 30 31 30 31 31 30 31 31 30 31 31 31 31 31 31 31 31 31 31 31 31 31	47
	LC Most	OG OF OVERBURDEN AND BEDRO	GENERAL DESCRIPTION	DEPTH FEET
GENERAL COLOUR	COMMON MATERIAL	OTHER MATERIALS		FROM 10
BiLWM	SAND		FINE	0 4
	BULDERS	GRAVEL , SAND	COARSE	4 19
	GRAVEL	SAND	COARSE, LOOSE	19:24
	GRAVEL	SAND	COARSE MACKED	24 26
	GRAVEL	SAMD (FINE.	)	26 31
	GRAYEL	SAND (FINE)M	ARL	31 35'
31	Hilililii		بالبليلياليياليليلياليا	لا لىلىللىن
32	تتنا لتلتليلل	لطبلطلينيا لطبارال	يا لتلتللنيا لتلتلبليا	بالباليا
41 W	ATER RECORD	51 CASING & OPEN HOLE	RECORD Z SIZE-SI OF OPENING 31-33 DIA	METER 34-38 LENGTH 39-40
WATER FOUND AT - FEET	KIND OF WATER	INSIDE WALL THICKNESS IS	DEPTY FEET WATERIAL AND TYPE	DEPTH TO TOP 41-44 10
10-13	FRESH 3 SULPHUR 14 SALTY 6 SALTY	10-11 1 DATEEL 12	STEEL	25 FEET
	T FRESH 3 DSULPHUR	1 / 3 CONCRETE	25 61 PLUGGING & SE	ALING RECORD
1	SALTY 6 DGAS	4 DOPEN HOLE 5 DPLASTIC 17-16 1 DSTEEL 19	20-23 DEPTH SET AT - FEET MATERIAL	AND TYPE CEMENT GROUT
20-23 1	FRESH 3 DSULPHUR 44 DMINERALS 6 DGAS	2 GALVANIZED 3 GONCRETE 4 GOPEN HOLE	FROM 10	
	FRESH 3 SULPHUR 29 SALTY 6 GAS	5 □ PLASTIC 26	27-30 19-21 22-25	
30-33	FRESH 3 DSULPHUR 34	2 GALVANIZED 3 CONCRETE	25-29 30-33 80	
	SALTY 6 DGAS	DPLASTIC !		
71 PUNPING TEST N	HETHOD 10 PUMPING RA	TE 11-14 DURATION OF PUMPING 12-16 17-18 GPM 10-16 17-18	LOCATION OF WE	LĻ
STATIC	1 WAYER LEVEL   25	1 D PUMPING	IN DIAGRAM BELOW SHOW DISTANCES OF WELL LOT LINE INDICATE NORTH BY ARROW.	L FROM ROAD AND
TEVEL 195	PUMPING 21 22-24 IS MINUTES	S V 30 MINUTES SMINUTES   60 MINUTES		1.
78°P	111	) mi mi	4/6	N/
UN IF FLOWING. GIVE RATE  RECOMMENDED	38-41 PUNP INTAK	E SET AT LIND OF TEST 42	Wex .	IAS
RECOMMENDED		DED 43-45 RECOMMENDED 46-49	No.	10
SO-53	OW DEEP SETTING	PUMPING FEET RATE GPM	i	
30.53				
FINAL	1   WATER SUPPLY 2   OBSERVATION W	5 ABANDONED, INSUFFICIENT SUPPLY ELL 6 ABANDONED POOR QUALITY	Tu 2/89 4	
OF WELL	3 TEST HOLE	7 UNFINISHED	1	
	55-56 1 DOMESTIC	5 COMMERCIAL 6 OM MUNICIPAL	<b>&gt;</b>	
WATER		PUBLIC SUPPLY COOLING OR AIR CONDITIONING		
USE	4 1 INDUSTRIAL	• □ COOLING ON AIR CONDITIONING		
METHOR	57 1 CABLE TOOL	♦ ☐ BORING	11	. N
METHOD OF	3 ROTARY (REVER	SE) I DETTING		88726
CONSTRUC	TION   A   ROTARY (AIR)	DIGGING OTHER	DRILLERS REMARKS	
NAME OF WE	LL CONTRACTOR	WELL CONTRACTOR'S	I > Isource	
1 -	WATERSUE	PLY LTD LICENCE NUMBER	DATE OF INSPECTION 28 01 NA	R D 8 1994
ADDRESS R	x 310 F	BARRIE.	O DATE OF INSPECTION	**
SIGNATURE CONTRACTOR	VELL TECHNICIAN	WELL TECHNICIAN'S		
SIGNATURE	OF TECHNIGIAN/CONTRACTOR	SUBMISSION DATE	OFFICE DISTRIBUTION OF THE PROPERTY OF THE PRO	CSS.ES
	11/11/	DAY 3 NO 1 YR92	[0]	
MINISTR	OF THE ENVIRON	IMENT COPY		FORM NO. 0506 (11/86) FORM S

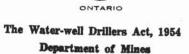
St Name THE CORP  illing Address (Street	PORATION Number/Name)	OFTHE	TOWNSHIPO	unicipality	F-mail Address	Postal Code		elephone	by We	constructed Il Owner area code)
440 HIGHW			N To	orwood	ONT	Lot . c		Concessio		בו ככ
	HWAY 7	Name)	ř	Isphodel		19	Provinc	9	Postal	Code
eter boto M Coordinates Zone	ugh	Northing	Ci g Mi	NOT Wood unicipal Plan and Suble	ot Number		Onta		Ko	
verburden and Bed		bandonme	THE RESERVE THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER.	d see instructions on the		neral Description		anum.	Dep	th (m/ft)
Aband	onmen	t of	well	record =	# 152	iorai Dosenpriori			From	10
	Bentoni	te c	hips						40	28
	Benton	ite =	slurry						78	9
	Bentoni	Te cl	reenings						95	2
	LIMESTON	2 3	recrimings	,						
				THE STATE OF THE S		Results of W	all Viels	d Testing	n	
Depth Set at (m/ft)	Туре	e of Sealant	Used	Volume Placed	After test of well yiel	d, water was:	Dra	aw Down Water Lev	R	ecovery Water Le
From To	(Mat	terial and Tyl	pe)	(m³/ft³)	Clear and sand	d free	(min) Static	(m/ft)	(min)	(m/ft)
					If pumping discontin	nued, give reason:	Level			
					Pump intake set at	(m/ft)	2		2	
							3		3	
Method of Co	nstruction		Well Us	PERSONAL PROPERTY OF THE PERSON NAMED IN COLUMN 1	Pumping rate (I/min	n/GPM)	3		3	
	·	Date:					4		4	
Rotary (Conventional		Public Domesti		rcial Not used	Duration of pumpir	ng min	5		5	
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Rotary (Conventional Rotary (Reverse) Boring Air percussion Other, specify  Col Inside Open Holo Diameter (Galvanize	Driving Digging  Digging  Digging	Domesti Livestoc Irrigatior Industria Other, s	ic Municipals   Municipals   Test Holin   Cooling   all   Specify   Depth (m/ft)	cial Not used al Dewatering e Monitoring & Air Conditioning  Status of Well Water Supply Replacement Well Test Hole Recharge Well Dewatering Well Observation and/or Monitoring Hole Alteration	hrs + Final water level end  If flowing give rate  Recommended pu  Recommended pu  (Vmin / GPM)	min d of pumping (m/lt, (l/min / GPM) mp depth (m/lt) mp rate	5 10 10 15 20 25 30 40 50		5 10 15 20 25 30 40	
Rotary (Conventional Rotary (Reverse) Boring   Boring   Air percussion   Other, specify   Collinside   Open Holi (Galvanize (cmvin)   Concrete,	Digging  Dig	Domesti Livestoc Irrigator Industria Other, s rd - Casing Wall ckness cm/ln) F	ic Municipals   Municipals   Test Holin   Cooling   all   Specify   Depth (m/ft)	cial Not used al Dewatering e Monitoring & Air Conditioning    Status of Well Water Supply Replacement Well Recharge Well Dewatering Well Observation and/or Monitoring Hole	If flowing give rate Recommended pu Recommended pu (Vmin / GPM) Well production (Vi	min d of pumping (m/lt) (Wmin / GPM) mp depth (m/lt) mp rate min / GPM)	5 10 15 20 25 30 40 50 60		5 10 15 20 25 30 40	
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Rotary (Conventional Rotary (Reverse) Boring Boring Air percussion Other, specify  Col Inside Diameter (cm/in) Concrete,  Concrete,  Col Concrete Concrete Management	Digging  Digging  Digging  Digging  Digging  Digging  Digging	Domesti Livestoc Irrigation Other, s  rd - Casing Wall ckness cm/in) F	ic	cial Not used al Dewatering e Monitoring & Air Conditioning  Status of Well Water Supply Replacement Well Test Hole Recharge Well Dewatering Well Observation and/or Monitoring Hole Alteration (Construction) Abandoned, Insufficient Supply Abandoned, Poor	hrs + Final water level end If flowing give rate Recommended pu Recommended pu (Vmin / GPM) Well production (VM) Disinfected?	min d of pumping (m/lt) (Wmin / GPM) mp depth (m/lt) mp rate min / GPM)  Map of W	5 10 15 20 25 30 40 50 60 Well Loc		5 10 15 20 25 30 40 50 60	
Rotary (Conventional Rotary (Reverse) Ro	) Jetting Driving Digging  Digging  nstruction Recore e OR Material ad, Fibreglass, Plastic, Steel)  onstruction Recore	Domesti Livestoc Irrigation Other, s  rd - Casing Wall ckness cm/in) F	ic	cial Not used al Dewatering e Monitoring & Air Conditioning & Air Conditioning    Status of Well Water Supply Replacement Well Personal Recharge Well Dewarding Hole Alteration (Construction) Abandoned, Insufficient Supply Abandoned, Poor Water Quality Abandoned, other,	hrs + Final water level end If flowing give rate Recommended pu Recommended pu (Vmin / GPM) Well production (VM) Disinfected?	min d of pumping (m/lt) (l/min / GPM) mp depth (m/lt) mp rate min / GPM)  Map of W ap below following	5 10 15 20 25 30 40 50 60 Well Loc		5 10 15 20 25 30 40 50 60	
Rotary (Conventional Rotary (Reverse) Ro	nstruction Recore OR Material Ad, Fibreglass, Plastic, Steel)  onstruction Recore OR Material Ad, Fibreglass, Plastic, Steel)	Domesti Livestoc Industria Other, s rd - Casing Wall ckness cm/(n) F	ic	cial Not used at Dewatering e Monitoring & Air Conditioning  Status of Well Water Supply Replacement Well Dewatering Well Dewatering Well Dewatering Well Dewatering Well Alteration (Construction) Abandoned, Insufficient Supply Abandoned, other, specify  Other, specify	If flowing give rate Recommended pu Recommended pu (Vmin / GPM) Well production (Vi Disinfected? Yes No	min d of pumping (m/lt) (l/min / GPM) mp depth (m/lt) mp rate min / GPM)  Map of W ap below following	5 10 15 20 25 30 40 50 60 Well Loc		5 10 15 20 25 30 40 50 60	ίν Τ
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Rotary (Conventional Rotary (Reverse) Boring Boring Air percussion Other, specify  Col Inside Open Hole (Galvanize Concrete, Manual Convin) Concrete,  Col Utside Diameter (cm/in) (Plastic, Galvanize (cm/in) (Convin) (Plastic, Galvanize (cm/in) (Convin)	Digging Diggin Digging Digging Digging Digging Digging Digging Digging Digging	Domesti Livestoc Irrigation Other, s  rd - Casing Wall ckness cm/in) F	Depth (m/ft)  From To  Depth (m/ft)  From To  Depth (m/ft)  From To	cial Not used al Dewatering e Monitoring & Air Conditioning	hrs + Final water level end If flowing give rate Recommended pu Recommended pu (Vmin / GPM) Well production (Vi Disinfected? Yes No Please provide a m	min d of pumping (m/lt) (l/min / GPM) mp depth (m/lt) mp rate min / GPM)  Map of W ap below following	5 10 15 20 25 30 40 50 60 Well Loc		5 10 15 20 25 30 40 50 60	ν <b>γ</b> Τ
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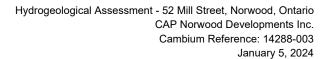






## Water-Well Record

	0		Mecol	1								
			nip, Village, Town or n Village, Town or C									
			n Village, Tewn or City)									
Date completed/Q(day)	(month)	5.8 (year)			•••••••••••••••••••••••••••••••••••••••							
Pipe and Casing			Pumping rate 200 salher tu.									
Casing diameter(s)												
Length of screen)			Duration of test J krs.									
Well Log			Water Record									
Overburden and Bedrock Record	From ft.	To ft.	Depth(s) at which water(s) found	No. of feet water rises	Kind of water (fresh, salty, or sulphur)							
reprevious to	0	42'	42' 14-42'	56'	hesh							
rey limestone	43'	92'										
or what purpose(s) is the water of swater clear or cloudy?	40 ar		In diagram below s	ation of Well show distances of Indicate north	well from							
ddress 68 / Water	St. Noio g		Norwood !	10 - mile	> <b>o</b>							
cence Number 456	regoing											
statements of fact a			45 Hwy.									
	-5(		• 11									





Appendix H
Preliminary Dewatering Calculations



#### **DEWATERING CALCULATIONS**

Modified Dupuit-Forchheimer Equation: unconfined flow into a linear excavation.

Calculations assume no flow boundary at aquifer base

Excavation Area	Initial depth	to.	Base of			Hydraulic Conductivity (K)	s	R <sub>o</sub>	L = R <sub>o</sub> /2	r <sub>s</sub> = b/2	$In(R_o/r_s)$ [If $r_s < R_o$ ]  Or $In((R_o+r_s)/r_s)$ [If $r_s > = R_o$ ]	н	h = H-s	Q <sub>ends</sub>	Q <sub>trench</sub>		Q <sub>total</sub>	
	mbgs	mbgs	mbgs	m	m	m/s	m	m	m	m	-	m	m	m³/s	m³/s	m <sup>3</sup> /s	L/s	L/d
Elongated Trench @ 50 m Increments N	in 0.88	3.5	5.0	50	2	4.88E-08	2.62	1.74	0.87	1.00	0.55	4.12	1.50	0.000004	0.000041	0.000045	0.05	3,929
M	0.88	3.5	5.0	50	2	5.95E-06	2.62	19.17	9.59	1.00	2.95	4.12	1.50	0.000093	0.000457	0.000550	0.55	47,533
A	g 0.88	3.5	5.0	50	2	2.75E-07	2.62	4.12	2.06	1.00	1.42	4.12	1.50	0.000009	0.000098	0.000107	0.11	9,264

s = target drawdown (initial - target depth to groundwater) (m)

R<sub>o</sub> = radius of influence of construction dewatering/pumping (m)

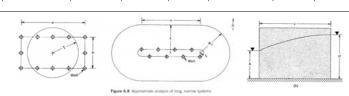
L = distance to line source (m)

r<sub>s</sub> = equivalent single well radius (m)

H = Initial hydraulic head in aquifer (m)

h = hydraulic head at radius of well (m)

Q = construction dewatering rate (m<sup>3</sup>/s)



$$Q = \frac{\pi K(H^2 - h^2)}{\ln R_0 / r_s} + 2 \left[ \frac{x K(H^2 - h^2)}{2L} \right]$$
 (6.10b) x = unit length of trench

 $R_o = 3000*s*sqrt(K)$ 

Source: Kyrieleis, W. and Sichardt, W. "Grundwasserabsenkung bei Fundierungsarbeiten" Springer, Berlin, 1930

Source: Powers, J. Patrick, et al. "Construction dewatering and groundwater control." (2007)