



January 15, 2024

Ms. Marnie Saunders Senior Land Use Planner D.M. Wills Associates Limited 150 Jameson Drive, Peterborough, ON K9J 0B9

Re: Response to Peer Review comments provided by Stantec Consulting Ltd., dated June 7, 2023, for the proposed Residential Subdivision in Township of Douro-Dummer, County of Peterborough

Dear Ms. Saunders,

As requested, we have reviewed the Peer Review Report provided by Stantec Consulting Ltd. (Appendix A1) for the Traffic Impact Study (TIS) dated June, 2018 of the proposed residential subdivision to be located on the northern edge of the Hamlet of Warsaw along east side of CR 4. This letter provides our response to the comments which follow the same numbering as the comments in peer review report dated June 7, 2023.

#### TIS ADDENDUM

#### **Our Response:**

- 1. The 2018 TIS used 2017 traffic data as the existing condition. The study has been updated with the recent 2023 traffic data. Please see **Exhibit 1.1** (attached at the back of this report) for the Existing 2023 traffic volumes used as the base for the traffic analysis.
- 2. Tranplan Associates Inc. collected the peak hour traffic volumes on County Road 4 & English Line South intersection on November 23,2023. The detailed 15 min traffic count report and an output diagram is attached in Appendix A.2 for reference.
- 3. The site trip generation is done based on ITE Trip Generation Manual-11<sup>th</sup> Edition. The Land-use code LU 210 (single family detached dwelling) was used to calculate trips based on average rate and fitted curve equation; the method that generated the highest number of trips was used for analysis. The fitted curve equation generated 17 total trips (4 inbound, 13 outbound) during AM peak hour and 22 total trips (14 inbound, 8 outbound) during PM peak hour.



Table 1: Trip Generation

		TRIP	GENERATION	CALC	ULAT	'ION	S			
Land Use	Units	Expected	Rate	AM P	EAK H	OUR	Rate	PM P	EAK H	OUR
Lanu Ose	Ullits	Units	Rate	TOTAL	IN	OUT	Nate	TOTAL	IN	OUT
LU 210 (Single family detached dwelling)					26%	74%			63%	37%
(1) Based on ITE 11th Edition						Averag	ge Rate			
Single Family detached dwelling	Units	20	0.70	14	4	10	0.94	19	12	7
			Total Trips	14	4	10		19	12	7
(2) Based on ITE 11th Edition					Fitt	ed Curv	ve Equation			
Single Family detached dwelling	Units	20	Ln(T)= 0.91Ln(X) + 0.12	17	4	13	Ln(T)= 0.94Ln(X) + 0.27	22	14	8
			Total Trips	17	4	13		22	14	8

4. Future Background Traffic Volumes: The existing 2023 traffic volumes are projected ahead to 2033 using an annual growth rate of 2% (compounded). No other background developments are expected to be in the planning approval/construction stage near the study area. Please see Exhibit 1.2 for the 2033 future background traffic volumes and Exhibit 1.3 for the new site traffic generated by the residential subdivision. The directional orientation and site traffic distribution remains the same as in the 2018 traffic study.

Future Traffic Volumes: The new site traffic volumes are added to the background traffic volumes to generate the future 2033 traffic volumes. Please see Exhibit 1.4 for the 2033 total traffic volumes.

#### 5. Available Sight Lines

Based on the TAC Geometric Design Guide for Canadian Roads, 2017, Section 9.9, the following table provides the intersection sight distance standards for 60 km/h design speed:

Table 2: Sight Line Requirements

		Intersection S	ight Distance	
Docian	Minimum	Left Turns	Right Turns	Left Turns
Design Speed*	Stopping	from Minor	from Minor	from Major
Speed	Distance (m)	Road (m)	Road (m)	Road (m)
60km/h	85	130	110	95

<sup>\*</sup>County Road 4, has a posted speed limit of 50 km/h. The study assumed 10km/h above posted speed as design speed.



As requested, Exhibit 2.0 illustrates approximate "airline" distance of relevant sight distance measured from the proposed site access on County Road 4 (measured using Google Aerial Photo Distance Measure Function).

It indicates that the available sight distance to the north of County Road 4 for vehicles turning left from proposed site access to travel south is approximately 300m which meets the requirement of 130m. Similarly, the available sight distance to the south of County Road 4 for vehicles turning right from the proposed site access to travel north is approximately 140m which meets the requirement of 110m. Both these distances also satisfy the minimum stopping distance standard of road with 60km/h design speed.

### 6. Synchro Parameters:

- For intersection capacity analysis using Synchro, the observed peak hour factors are used as synchro parameters for each approach on the County Road 4 & English Line South intersection. The observed peak hour factors can be found in Appendix A.2, Traffic Data.
- Pedestrian volumes were observed during the AM & PM peak hours and no conflicting pedestrians were noted for the County Road 4 & English Line South intersection. The pedestrians were observed to be coming from the south side of County Road 4, using the sidewalk connected to English Line South and then crossing at the pedestrian cross provided near Warsaw Public school entrance.

The synchro analysis was updated using the revised traffic volumes. The resulting synchro reports for Existing 2023 condition, 2033 background condition and 2033 total traffic condition is attached in Appendix B, Synchro Reports. The intersection capacity analysis summary table is provided below.

**Existing 2023 Traffic Conditions Background 2033 Traffic Conditions Total 2033 Traffic Conditions** INTERSECTIONS **AM Peak Hour** PM Peak Hour PM Peak Hour LoS Delay V/C 95th Q LoS Delay V/C LoS Delay V/C 95th Q LoS Delay V/C LoS Delay V/C 95th Q LoS Delay V/C 95th Q A 9.6 0.10 2.7 A 9.6 0.11 2.8 A 9.3 0.08 2.0 A 9.7 0.02 0.5 B 10.0 0.03 0.7 A 10.0 0.03 0.8 Country Rd 4 & English Line S 0.0 0.04 0.0 0.06 0.0 0.05 0.0 0.07 0.0 0.0 0.05 0.0 0.0 0.08 0.0 A 1.7 0.01 0.4 SBTL 1.6 0.01 0.3 A 0.1 0.00 0.0 A 0.1 0.00 0.0 A 1.6 0.02 0.4 A 0.2 0.00 0.0 WBLR 9.1 0.02 0.4 9.3 0.01 0.0 Country Rd 4 & Site Entrance **NBTR** 0.0 0.04 0.0 0.0 0.06 SBTL 0.1 0.00 0.0 0.5 0.00 0.1

Table 3: Intersection capacity analysis summary table



The intersection capacity analysis indicates that all individual movements of County Road 4 & English Line South is operating very well with LOS "A", minimal delay, and queue lengths at the existing, background and total traffic conditions. The proposed site access is also forecast to operate very well.

Please feel free to contact if you have any questions or concerns.

Yours truly,

Sreelakshmi Changaradil, M.Sc., E.I.T

Reviewed By,

Seo-Woon (Swan) Im, B.E.S

Tranplan Associates,

Senior Transportation Planner

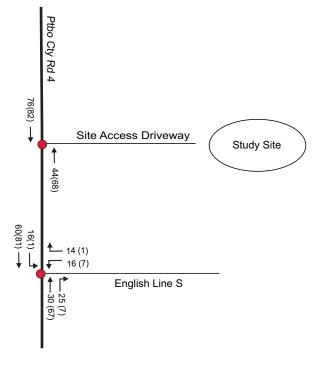


## **EXHIBITS**

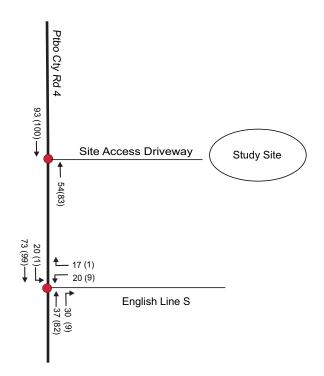
## **EXHIBIT: 1 Traffic Volumes**



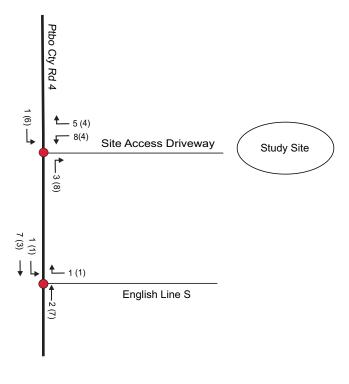
**EXHIBIT: 1.1 Existing 2023 Traffic Volumes** 



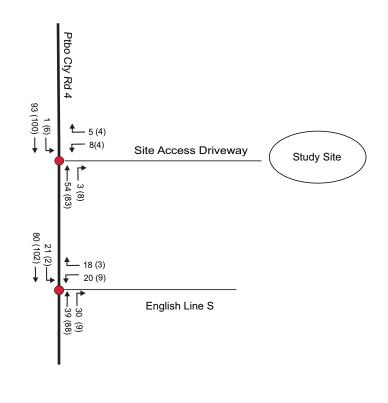
**EXHIBIT: 1.2 2033 Total Future Background Traffic Volumes** 



**EXHIBIT: 1.3 New Site Traffic** 



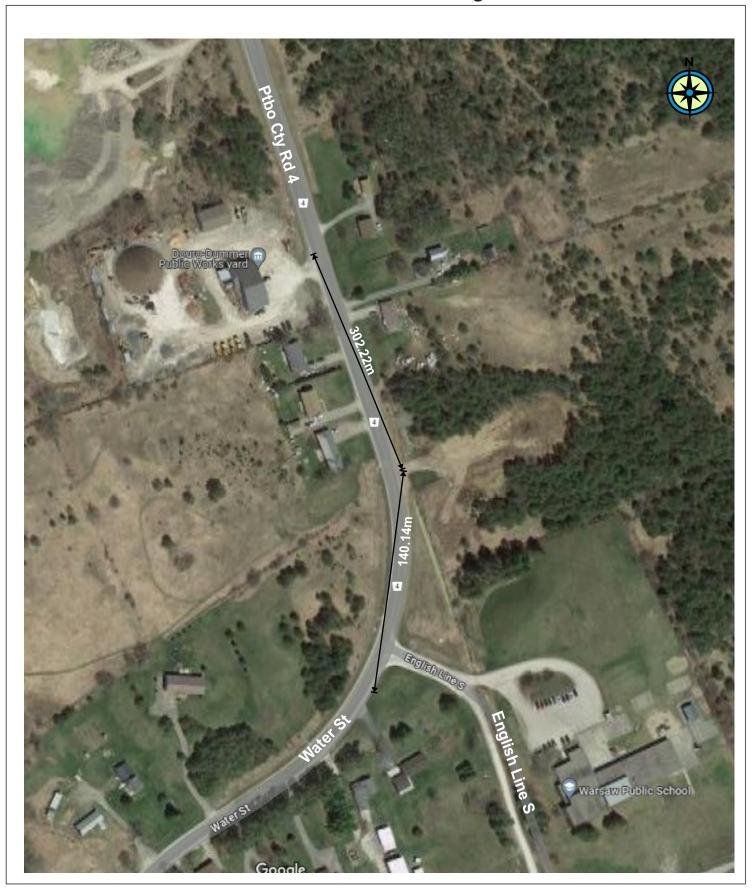
**EXHIBIT 1.4 2033 Total Traffic Volumes** 



436 Weekday AM peak hour volume (350) Weekday PM peak hour volume
Unsignalized intersection
Signalized intersection



# EXHIBIT 2 : Available Sight Lines







# **APPENDIX A1 - PEER REVIEW REPORT (STANTEC)**



June 7, 2023 File: 160900933

Attention: Ken Scullion Planner Peterborough County 470 Water Street Peterborough, ON K9H 3M3

Email: kscullion@ptbocounty.ca

Reference: Traffic Impact Study Peer Review Warsaw Residential Subdivision Peterborough

County Rd. 4 Township of Douro-Dummer, County of Peterborough, Ontario

#### INTRODUCTION

The County of Peterborough (The County) has requested Stantec Consulting Ltd. (Stantec) to conduct a peer review of the Traffic Impact Study (TIS) for the proposed development captioned above (herein referred to as the Site).

#### **SUBJECT DEVELOPMENT**

The proposed Warsaw Residential Subdivision site is located in the geographic Township of Dummer now part of the amalgamated Township of Douro-Dummer. The subdivision will be located on the northern edge of the Hamlet of Warsaw along the east side of CR 4 on a green-field site. The proposed plan of subdivision will contain up to 20 individual lots for single family residential housing. Each lot will have its own driveway accessing a new local street that will provide direct access to CR 4.

#### **DOCUMENT REVIEWED**

The following document was reviewed for purposes of the Traffic Impact Study Report peer review:

 Warsaw Residential Subdivision Peterborough County Rd. 4 - Traffic Impact Study", dated June 2018. The Terms of Reference and/or information regarding pre-consultation discussions with The County or the Township for the TIA was not made available for review. Reference:

Traffic Impact Study Peer Review Warsaw Residential Subdivision Peterborough County Rd. 4 Township of Douro-Dummer, County of Peterborough, Ontario

#### TRAFFIC REPORT REVIEW COMMENTS

This peer review report provides a review of the methodology, technical analysis, findings, and recommendations presented in the TIS and is solely based on the content of the provided report and its appendices.

While spot-checks on calculations were undertaken to confirm the study was prepared using industry accepted practice and appropriate methodology, Stantec does not take liability for any omissions/exceptions that Tranplan Associates may have made throughout their assessment.

#### **Peer Review comments**

- 1. Section 1.1: Background
  - The study uses 2017 as the existing condition that is 6 years prior to this review process.
     We request the consultant to provide support on the validity of the TIS as the TIS documents are usually considered obsolete after 3 years.
- 2. Section 2.4: Current Traffic Data
  - The detailed traffic counts should be provided in the Appendix for reference.
- 3. Section 3.1: Trip Generation Forecasts
  - When using the ITE Trip Generation Manual, it is recommended to calculate the site generated traffic based on average rate as well as fitted curve equation and choose whichever is higher for the analysis.
- 4. Section 4.1: Future Background Traffic
  - No background development was considered in this study. It is recommended to confirm if any background development exists. This is a concern especially with the 6-year span between preparation of the TIS and its peer review.
- 5. Section 4.5: Future Site Access
  - The design ISD for left turn from stop is at the verge of exceeding the available ISD. It is recommended to provide a map illustrating the actual available sightline.
- 6. Synchro Parameters
  - The Section 2.4 indicates that peak hour factor of 0.70 is used in this study but peak hour factor of 0.92 is found in the Synchro analysis.
  - It seems that the conflicting pedestrians at the English Line South and County Road 4 intersections are assumed values (AM and PM pedestrian volumes are the same). It is recommended to use actual pedestrian volumes since it is a school access.



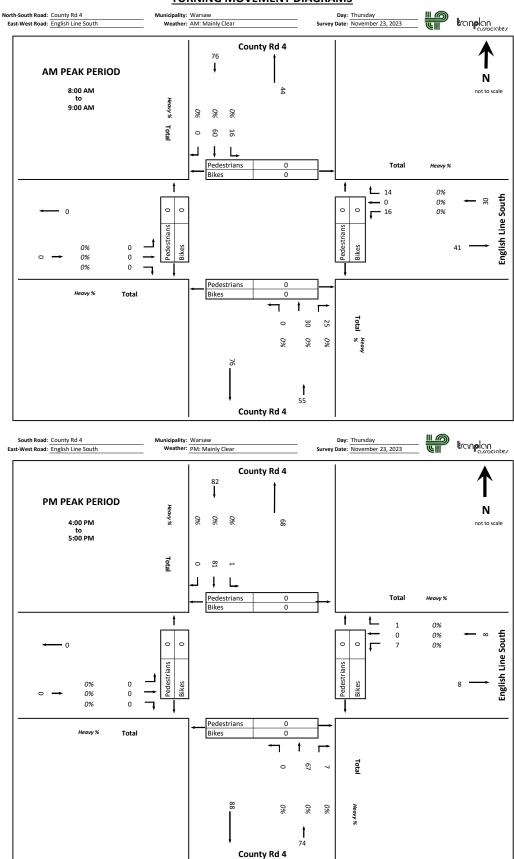
## **APPENDIX A.2 – 2023 TRAFFIC DATA**

### County Road 4/English Line South \_ November 23, 2023

Time	Noi	rth Approac	:h	So	uth Approa	ch	E	ast Approac	:h
AM Peak Hour	Left	Thru	Total	Right	Thru	Total	Left	Right	Total
8.00	1	20	21	4	6	10	2	0	2
8.15	4	16	20	5	7	12	1	1	2
8.30	7	14	21	8	9	17	10	8	18
8.45	4	10	14	8	8	16	3	5	8
<b>Grand Total</b>	16	60	76	25	30	55	16	14	30
Peak Hour Factor		0.90			0.81			0.42	

Time	Nor	th Approac	:h	So	uth Approa	ch	Ea	ast Approac	:h
PM Peak Hour	Left	Thru	Total	Right	Thru	Total	Left	Right	Total
4.00	0	31	31	2	23	25	3	1	4
4.15	0	18	18	0	12	12	1	0	1
4.30	0	21	21	1	16	17	2	0	2
4.45	1	11	12	4	16	20	1	0	1
<b>Grand Total</b>	1	81	82	7	67	74	7	1	8
Peak Hour Factor		0.66			0.74			0.50	

#### **TURNING MOVEMENT DIAGRAMS**





## **APPENDIX B - SYNCHRO REPORTS**

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	**		f)			र्स	
Traffic Volume (veh/h)	16	14	30	25	16	60	
Future Volume (Veh/h)	16	14	30	25	16	60	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.42	0.42	0.81	0.81	0.90	0.90	
Hourly flow rate (vph)	38	33	37	31	18	67	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)						106	
pX, platoon unblocked							
vC, conflicting volume	156	52			68		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	156	52			68		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	95	97			99		
cM capacity (veh/h)	826	1015			1533		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	71	68	85				
Volume Left	38	0	18				
Volume Right	33	31	0				
cSH	904	1700	1533				
Volume to Capacity	0.08	0.04	0.01				
Queue Length 95th (m)	2.0	0.0	0.3				
Control Delay (s)	9.3	0.0	1.6				
Lane LOS	Α		Α				
Approach Delay (s)	9.3	0.0	1.6				
Approach LOS	Α						
Intersection Summary							
Average Delay			3.6				
Intersection Capacity Utilization	n		20.7%	IC	U Level c	f Service	
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	M		₽			र्स	
Traffic Volume (veh/h)	7	1	67	7	1	81	
Future Volume (Veh/h)	7	1	67	7	1	81	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.50	0.50	0.74	0.74	0.66	0.66	
Hourly flow rate (vph)	14	2	91	9	2	123	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)						106	
pX, platoon unblocked							
vC, conflicting volume	222	96			100		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	222	96			100		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)							
tF (s)	3.5	3.3			2.2		
p0 queue free %	98	100			100		
cM capacity (veh/h)	765	961			1493		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	16	100	125				
Volume Left	14	0	2				
Volume Right	2	9	0				
cSH	785	1700	1493				
Volume to Capacity	0.02	0.06	0.00				
Queue Length 95th (m)	0.5	0.0	0.0				
Control Delay (s)	9.7	0.0	0.1				
Lane LOS	A	0.0	A				
Approach Delay (s)	9.7	0.0	0.1				
Approach LOS	A		<u> </u>				
Intersection Summary							
Average Delay			0.7				
Intersection Capacity Utiliza	ation		15.1%	IC	U Level o	of Service	
Analysis Period (min)			15.176	.0	2 23107		
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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1			4
Traffic Volume (veh/h)	20	17	37	30	20	73
Future Volume (Veh/h)	20	17	37	30	20	73
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.42	0.42	0.81	0.81	0.90	0.90
Hourly flow rate (vph)	48	40	46	37	22	81
Pedestrians	70	70		01		01
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)			INUITE			INOHE
						106
Upstream signal (m)						100
pX, platoon unblocked	190	64			83	
vC, conflicting volume	190	04			03	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	400	C4			00	
vCu, unblocked vol	190	64			83	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	0.5	2.0			0.0	
tF (s)	3.5	3.3			2.2	
p0 queue free %	94	96			99	
cM capacity (veh/h)	788	1000			1514	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	88	83	103			
Volume Left	48	0	22			
Volume Right	40	37	0			
cSH	872	1700	1514			
Volume to Capacity	0.10	0.05	0.01			
Queue Length 95th (m)	2.7	0.0	0.4			
Control Delay (s)	9.6	0.0	1.7			
Lane LOS	Α		Α			
Approach Delay (s)	9.6	0.0	1.7			
Approach LOS	Α					
Intersection Summary						
Average Delay			3.7			
Intersection Capacity Utiliz	zation		21.6%	IC	باميرمايا	of Service
	LauUII			10	O LEVEL	JI JEI VICE
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	N/		₽			र्स	Ī
Traffic Volume (veh/h)	9	1	82	9	1	99	
Future Volume (Veh/h)	9	1	82	9	1	99	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.50	0.50	0.74	0.74	0.66	0.66	
Hourly flow rate (vph)	18	2	111	12	2	150	
Pedestrians							
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)							
Upstream signal (m)						106	
pX, platoon unblocked							
vC, conflicting volume	271	117			123		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	271	117			123		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)	0.1	J. <u>Z</u>					
tF (s)	3.5	3.3			2.2		
p0 queue free %	97	100			100		
cM capacity (veh/h)	717	935			1464		
			05.4				
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	20	123	152				
Volume Left	18	0	2				
Volume Right	2	12	0				
cSH	735	1700	1464				
Volume to Capacity	0.03	0.07	0.00				
Queue Length 95th (m)	0.7	0.0	0.0				
Control Delay (s)	10.0	0.0	0.1				
Lane LOS	В		Α				
Approach Delay (s)	10.0	0.0	0.1				
Approach LOS	В						
Intersection Summary							
Average Delay			0.7				
Intersection Capacity Utilization	on		16.0%	IC	U Level o	of Service	
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	W		1>			र्स	-
Traffic Volume (veh/h)	20	18	39	30	21	80	
Future Volume (Veh/h)	20	18	39	30	21	80	
Sign Control	Stop		Free			Free	
Grade	0%		0%			0%	
Peak Hour Factor	0.42	0.42	0.81	0.81	0.90	0.90	
Hourly flow rate (vph)	48	43	48	37	23	89	
Pedestrians				<b>.</b>			
Lane Width (m)							
Walking Speed (m/s)							
Percent Blockage							
Right turn flare (veh)							
Median type			None			None	
Median storage veh)			110110			110/10	
Upstream signal (m)						106	
pX, platoon unblocked						100	
vC, conflicting volume	202	66			85		
vC1, stage 1 conf vol	202	00					
vC2, stage 2 conf vol							
vCu, unblocked vol	202	66			85		
tC, single (s)	6.4	6.2			4.1		
tC, 2 stage (s)	0.4	0.2			7.1		
tF (s)	3.5	3.3			2.2		
p0 queue free %	94	96			98		
cM capacity (veh/h)	775	997			1512		
					1012		
Direction, Lane #	WB 1	NB 1	SB 1				
Volume Total	91	85	112				
Volume Left	48	0	23				
Volume Right	43	37	0				
cSH	866	1700	1512				
Volume to Capacity	0.11	0.05	0.02				
Queue Length 95th (m)	2.8	0.0	0.4				
Control Delay (s)	9.6	0.0	1.6				
Lane LOS	Α		Α				
Approach Delay (s)	9.6	0.0	1.6				
Approach LOS	А						
Intersection Summary							
Average Delay			3.7				
Intersection Capacity Utilizat	tion		22.0%	IC	U Level o	of Service	
Analysis Period (min)			15				

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1>			र्स
Traffic Volume (veh/h)	8	5	54	3	1	93
Future Volume (Veh/h)	8	5	54	3	1	93
Sign Control	Stop		Free		-	Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	5	59	3	1	101
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)			110110			110/10
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	164	60			62	
vC1, stage 1 conf vol	10-1	00			02	
vC2, stage 2 conf vol						
vCu, unblocked vol	164	60			62	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)	0.7	0.2			7.1	
tF (s)	3.5	3.3			2.2	
p0 queue free %	99	100			100	
cM capacity (veh/h)	827	1005			1541	
					1041	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	14	62	102			
Volume Left	9	0	1			
Volume Right	5	3	0			
cSH	883	1700	1541			
Volume to Capacity	0.02	0.04	0.00			
Queue Length 95th (m)	0.4	0.0	0.0			
Control Delay (s)	9.1	0.0	0.1			
Lane LOS	Α		Α			
Approach Delay (s)	9.1	0.0	0.1			
Approach LOS	Α					
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utiliza	ation		15.7%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		1			4
Traffic Volume (veh/h)	4	4	83	8	6	100
Future Volume (Veh/h)	4	4	83	8	6	100
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	4	90	9	7	109
Pedestrians	<u> </u>	•			•	
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			None
Median storage veh)			INOLIC			INOLIC
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	218	94			99	
vC1, stage 1 conf vol	210	34			33	
vC2, stage 2 conf vol						
vCu, unblocked vol	218	94			99	
	6.4	6.2			4.1	
tC, single (s)	0.4	0.2			4.1	
tC, 2 stage (s)	2 5	3.3			2.2	
tF (s)	3.5 99	100			100	
p0 queue free %	767	962				
cM capacity (veh/h)	101	902			1494	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	8	99	116			
Volume Left	4	0	7			
Volume Right	4	9	0			
cSH	854	1700	1494			
Volume to Capacity	0.01	0.06	0.00			
Queue Length 95th (m)	0.2	0.0	0.1			
Control Delay (s)	9.3	0.0	0.5			
Lane LOS	Α		Α			
Approach Delay (s)	9.3	0.0	0.5			
Approach LOS	Α					
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utiliza	ation		20.2%	IC	Ulevelo	of Service
Analysis Period (min)	- COII		15	.0	5 201010	J. 00/ VIOC
Alialysis Fellou (IIIIII)			10			