Preliminary
Stormwater Management Report

Life at the Woodland
Township of Otonabee-South
Monaghan
County of Peterborough

Residential Subdivision Development

D.M. Wills Project No. 19-10874

D.M. Wills Associates Limited
Partners in Engineering, Planning and
Environmental Services
Peterborough

March 2023

Prepared for:
Life at the Woodland Inc.

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1.0 Purpose

D.M. Wills Associates Limited (Wills) has been retained by Mr. Rubal Kundra of Life at the Woodland Inc. to prepare a Preliminary Stormwater Management (SWM) Plan and Report for the proposed residential subdivision located on Part of Lot 27, Concession 10, Township of Otonabee-South Monaghan in the County of Peterborough (County).

The purpose of this report is to evaluate the impact of the proposed development on the stormwater runoff and to develop a preliminary plan for SWM that will permit the development to proceed with no adverse impacts to the receiving drainage systems. This report has been prepared specifically for the County and the Otonabee Region Conservation Authority (ORCA) to address SWM for the development and to satisfy the statutory requirements.

2.0 Site Description

The Subject Property is located in the township of Otonabee-South Monaghan with access provided from Burnham Line, situated approximately 6 kilometres east of Peterborough. The Subject Property is legally described as a Part of Lot 27, Concession 10 in the Township of Otonabee-South Monaghan in the County and is approximately 17.67 ha in area. The Subject Property is bounded by agricultural land use to the north, Burnham Line and wooded area to the west, wetland and wooded area to the east and residential lots accessed by Lancaster Road to the south. An unevaluated wetland feature is located in the western portion of the property, which drains to two separate outlets. The location of the Subject Property is shown on Figure 1.

The proposed residential subdivision consists of 27 lots adjacent to the wetland buffer. The portion of the Subject Property within the wetland buffer is to remain in an undisturbed natural state. The development of the Subject Property will require the removal of vegetated areas and grasslands. A cul-de-sac roadway off Burnham Line will provide access for the proposed lots.

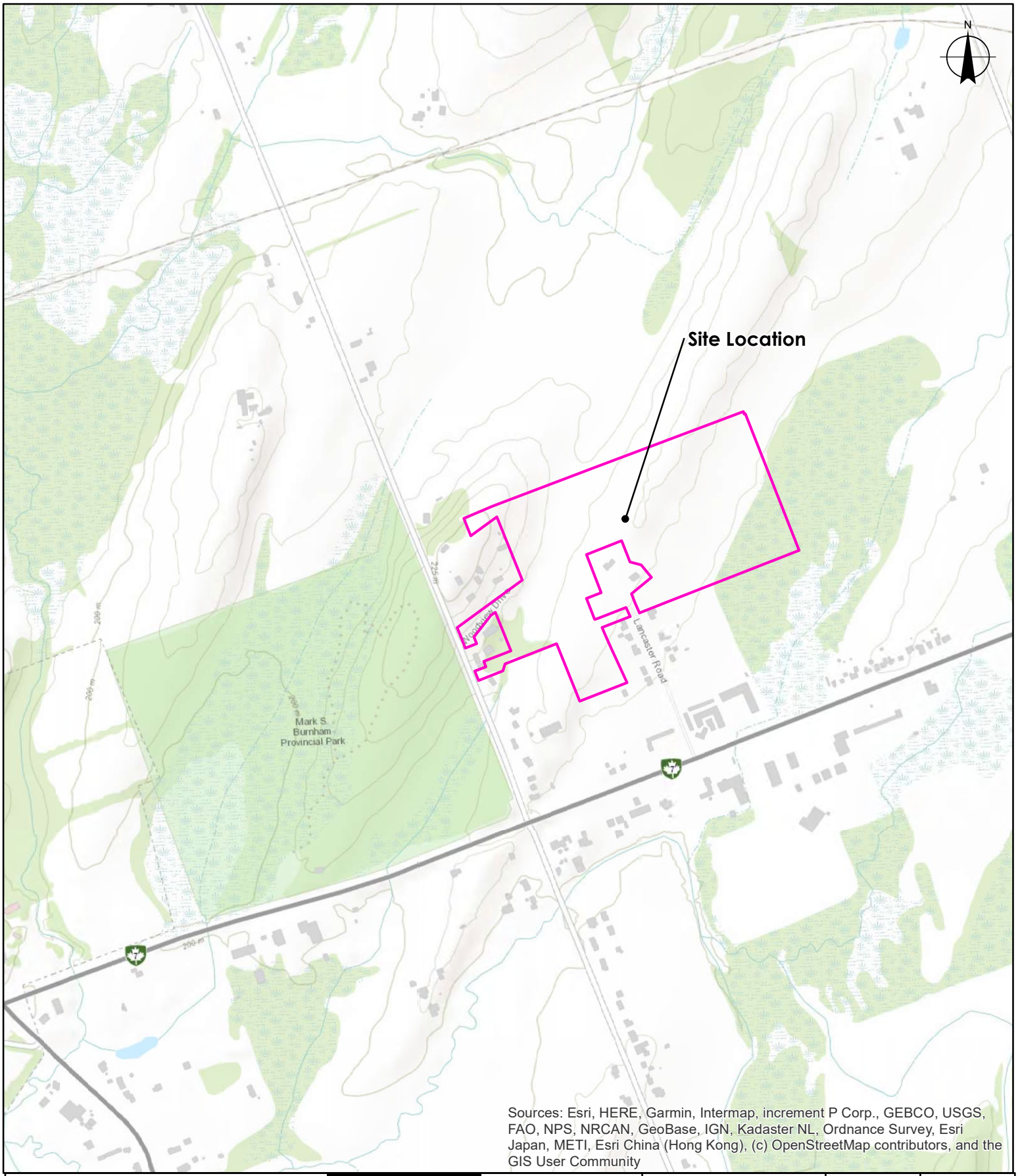

Figure 01
Location Plan

NAD83 UTM Zone 18

0 75 150 300 450 600

Meters

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Legend

— Property Limits

Drawn by: RC	Scale (Horz.) 1:10,000
Checked by: CPB	
Engineer: RC/CPB	Map Date Dec 2021
Project No. 10874	Map File No. 10874-DP01

According to the Soil Survey Complex of Ontario, the Subject Property is primarily composed of two types of surficial soils. The wetland portion of the site is composed of Foxboro Silt Loam with the remainder of the site composed of Otonabee Loam. According to the SCS method of classifying soils, these types of surficial soils correspond to Hydrologic Soils Groups C and B respectively.

A topographic survey of the Subject Property was completed by JBF Surveyors Ltd. (Project #7369) with the latest drawing version dated January 13, 2020, to determine existing elevations and the location of drainage features on the Subject Property. This information was used to determine drainage patterns and preliminary catchment area characteristics. A digital elevation model of the wetland was obtained from the South Central Ontario Orthophotography (SCOOP) under Land Information Ontario (LIO) database. The horizontal datum for the SCOOP is projected in UTM Zone 17 of the NAD83 Canadian Spatial Reference System and the vertical datum is sourced CGVD28. The SCOOP DEM raster has a resolution of 20 cm, however it was resampled into a raster cell of size 2 m. This data was produced to meet ASPRS Positional Accuracy Standards for Digital Geospatial Data (2014) with +/- 46.0cm actual VVA accuracy at the 95th percentile.

3.0 Methodology

The present hierarchy of watershed planning in Ontario can be described by the following in descending order: Watershed Plans, Sub-watershed Plans and individual SWM Plans. There is no Master Plan directing the SWM strategy for this watershed. As such, the proposed development was prepared as an individual Preliminary SWM Plan.

3.1 Site Specific Stormwater Design Criteria

A pre-consultation meeting was held to discuss the requirements of the Draft Plan Approval process. The following design criteria have been established from the pre-consultation meeting and with follow-up correspondence from the County and ORCA.

- To provide stormwater quality controls, to achieve "Enhanced" Level 1 protection as defined in the SWM Planning and Design Manual (March 2003).
- To provide stormwater quantity controls, to reduce the post development peak flow rates to the existing condition peak flow rates at each outlet location, for the 2 to 100-year design storms.
- To respect the recommended statutory setback requirements provided by the regulatory agency for the unevaluated wetland located within the property.
- To incorporate Low Impact Development (LID) features as part of the proposed stormwater management strategy.

3.2 Catchment Area Characterization

For the purpose of the preliminary SWM plan, the site will be analyzed as six catchment areas based on the site topography under existing conditions. The existing catchment areas are shown on Figure 2. There exists portions of the property, specifically the valley that traverses the site and the southeast and northwest corners, that will remain undeveloped and have subsequently omitted from the hydrologic model.

- Catchment area EX-100 consists of 2.02 ha of land and includes the northwestern portion of the subject site and neighboring lots. This catchment consists of the current gravel access to the site from Burnham Line, wooded areas and rural residential lots. Runoff from this catchment currently flows overland to the wetland, discharging to OUT-1 WEST.
- Catchment area EX-101 consists of 1.42 ha of land and includes the southwest portion of the site. This catchment is comprised of woodlots and grassed areas. Runoff from this catchment will flow overland northerly to the wetland, discharging to OUT-1 EAST.
- Catchment area EX-200 consists of 1.53 ha of land and includes the northern portion of the site. This catchment is comprised of wooded areas and neighboring residential rear yards. Runoff from this catchment will flow overland towards the wetland, discharging to OUT-2 WEST.
- Catchment area EX-201 consists of 2.44 ha of land and includes the north central portion of the Subject Property. This catchment consists of wood lots and grassed areas. Runoff from this catchment will drain northerly towards the wetland, discharging to OUT-2 EAST.
- Catchment EX-300 consists of 1.73 ha of land located in the eastern portion of the Subject Property. This catchment is comprised of grassed range type area with some treed fence lines intersecting the catchment. The runoff generated within this catchment drains southeasterly to an unidentified wetland located southeast of the Subject Property (OUT-3).
- Catchment EX-400 consists of 1.24 ha of land located in the south-central portion of the Subject Property west of Lancaster Road. This catchment is comprised of grassed range with some wood pockets. The runoff generated within this catchment drains southerly overland towards the private properties on Lancaster Road (OUT-4).

Under the proposed condition, the catchment areas subject to land use change were delineated into eight sub-catchment areas as shown on Figure 3.

- Catchment area PR-100 consists of 2.66 ha of land and includes the northwest neighboring lots and proposed access road from Burnham Line. Runoff from this catchment will flow overland towards a proposed SWM facility for treatment and will then outlet to the wetland, discharging to OUT-1 WEST.

- Catchment area PR-101 consists of 0.59 ha of land comprised of the rear lot yards located west of the proposed cul-de-sac. Runoff from this catchment will drain uncontrolled towards the wetland as in the existing condition, discharging to OUT-1 EAST.
- Catchment area PR-102 consists of 1.48 ha of land and includes the southern portion of the developed Subject Property comprised of the proposed roadway and cul-de-sac, and a portion of the proposed residential lots. Runoff from this catchment will flow overland to a proposed SWM facility for treatment and will then outlet to the wetland, discharging to OUT-1 EAST.
- Catchment area PR-200 consists of 0.75 ha of land and includes the northern portion of the Subject Property. This catchment is comprised of proposed lots and a portion of the proposed roadway. Runoff from this catchment will drain towards a proposed SWM facility and will outlet to the wetland, discharging to OUT-2 WEST.
- Catchment area PR-201 consists of 1.28 ha of land and includes a portion of the proposed roadway and residential lots. Runoff from this catchment will drain towards a proposed SWM facility and will outlet to the wetland, discharging to OUT-2 EAST.
- Catchment area PR-202 consists of 1.40 ha of land and includes the northeastern portion of the Subject Property, consisting of the proposed roadway and lots. Runoff from this catchment will flow overland towards the wetland, discharging to OUT-2 EAST.
- Catchment PR-300 consists of 1.52 ha of land that's is comprised of proposed lots. Runoff generated from this catchment will drain overland easterly to the back of the lots (OUT 3), ultimately entering the existing wetland area to the east of the Subject Property.
- Catchment PR-400 consists of 0.58 ha of land and includes the southern tip of the property, consisting of the rear yards of the proposed lots. Runoff from this catchment will drain overland, south easterly towards Lancaster Road as in the existing condition (OUT 4).

SITE AREA = 17.67 ha

OUT 1 WEST

OUT 2 WEST

15m WETLAND SETBACK

30m WETLAND SETBACK

OUT 3

OUT 2 EAST

OUT 1 EAST

OUT 4

BURNHAM LINE

WOODVIEW DR.

LANCASTER ROAD

EX-100
2.02

EX-200
1.53

EX-201
2.44

EX-300
1.73

EX-101
1.42

EX-400
1.24

EX-100
0.22
CATCHMENT ID
CATCHMENT AREA (ha)
CATCHMENT BOUNDARY

WETLAND SETBACK

SUBJECT PROPERTY LINE

OUT OUTLET LOCATION

OVERLAND FLOW DIRECTION

Sketch No.

FIGURE 2
LIFE AT THE WOODLAND
EXISTING DRAINAGE AREA PLAN

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Drawn By: GB	Scale: 1: 3000
Designed By: RC	Plot Date: JAN 2023
Checked By: CPB	Project No.: 19-10874
Engineer: RC/CPB	Drawing File No.: SWM FIGURE 2

SITE AREA = 17.67 ha

ROADSIDE DITCH SWM FACILITY
(CATCHMENT PR-100):
DITCH LENGTH = 90m
BOTTOM WIDTH = 2m
INFILTRATION LAYER DEPTH = 1m
DITCH DEPTH = 0.6m
SIDE SLOPES = 3:1

ROADSIDE DITCH SWM FACILITY
(CATCHMENT PR-200):
DITCH LENGTH=50m
BOTTOM WIDTH=2m
SIDE SLOPES=3:1
INFILTRATION LAYER DEPTH = 0.5m

OUT 2 WEST

15m WETLAND SETBACK

OUT 2 EAST

PR-200 0.75

PR-202 1.40

PR-100 2.66

PR-201 1.28

PR-300 1.52

OUT 3

BURNHAM LINE

WOODVIEW DR.

PR-102 1.48

30m WETLAND SETBACK

OUT 1 WEST

OUT 1 EAST

PR-101 0.59

LANCASTER ROAD

ROADSIDE DITCH SWM FACILITY
(CATCHMENT PR-201):
DITCH LENGTH = 107m
BOTTOM WIDTH = 2m
INFILTRATION LAYER DEPTH = 0.6m
DITCH DEPTH = 0.6m
SIDE SLOPES = 3:1

ROADSIDE DITCH SWM FACILITY
(CATCHMENT PR-102):
DITCH LENGTH = 97m
BOTTOM WIDTH = 2m
INFILTRATION LAYER DEPTH = 1m
DITCH DEPTH = 0.8m
SIDE SLOPES = 3:1

PR-400 0.58

OUT 4

PR-100 0.22 — CATCHMENT ID / CATCHMENT AREA (ha)

CATCHMENT BOUNDARY

WETLAND SETBACK

SUBJECT PROPERTY LINE

OUT — OUTLET LOCATION

OVERLAND FLOW DIRECTION

Sketch No.

FIGURE 3
LIFE AT THE WOODLAND
PROPOSED DRAINAGE AREA PLAN

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Drawn By: GB	Scale: 1:2500
Designed By: RC	Plot Date: JAN 2023
Checked By: CPB	Project No.: 19-10874
Engineer: CPB	Drawing File No.: SWM FIGURE 3

The hydrologic parameters used for each catchment area, in both existing and proposed conditions, are summarized Table 1 and documented in Appendix A.

Table 1 – Existing and Proposed Hydrologic Parameters

Standhyd[1]						
Catchment ID	Area (ha)	Impervious %	CN*[2]	Ia[3]	Pervious Slope (%)	Impervious Slope (%)
PR-102	1.48	23	60.7	5.3	5.0	2.0
PR-201	1.28	24.6	62.5	5.8	6.6	1.0

Nashyd[1]					
Catchment ID	Area (ha)	Impervious %	CN*[2]	Ia[3]	Tp[4] (hrs)
EX-100	2.02	1.9	61.5	5.3	0.17
EX-101	1.42	2.4	63.1	9.1	0.22
EX-200	1.53	0.0	61.7	8.7	0.17
EX-201	2.44	0.7	68.8	8.0	0.17
EX-300	1.73	0.0	68.6	8.0	0.17
EX-400	1.24	0	67.5	8.2	0.26
PR-100	2.66	9.7	64.8	4.8	0.17
PR-101	0.59	10.0	65.0	4.7	0.17
PR-200	0.75	16.1	68.1	7.4	0.17
PR-202	1.40	14.8	67.1	4.6	0.17
PR-300	1.52	8.9	65.4	5.1	0.17
PR-400	0.58	12.1	66.1	4.6	0.26

Notes:
1. Command Line refers to the unit hydrograph used in the VO3 hydrologic model for the respective catchment area.
2. CN* refers to the modified CN number adjusted to Antecedent Moisture Conditions II. Excludes Impervious Area for Standhyd.
3. Ia refers to Initial Abstraction. Excludes Impervious Area for Standhyd.
4. Tp refers to Time of Peak.

Hydrologic parameters such as soil infiltration properties, land use and runoff response were determined based on aerial photography, site reconnaissance and literature review. Topographic mapping and AutoCAD Civil 3D 2019 software were used to establish sub-catchment areas, land use and slopes. Rainfall data for the site is taken from the City of Peterborough Engineering Design Standards (April 2019) and is included in Appendix A.

4.0 Stormwater Management

4.1 Low Impact Development Design

As the practice of SWM has evolved, increasing emphasis has been placed on utilizing a treatment train approach to manage runoff as close to the source as possible. This design philosophy is often referred to as Low Impact Development (LID), where the goal is to maintain and mimic the natural hydrologic conditions. LID designs accomplish this by reducing the runoff volume generated by a site and implementing features that infiltrate, filter, evaporate, harvest, and detain runoff, while also preventing pollution. ORCA encourages the use of LID features as part of the water quality design for a site and, therefore, opportunities to utilize these features have been investigated.

For infiltration-based LID features, a minimum separation of 1.0 m is required from the bottom of feature to the higher of the seasonally high groundwater or bedrock elevation. As such, a geotechnical study with in-situ infiltration testing will ultimately be required to confirm the availability of minimum separation and observed infiltration rates.

LID features must also include an underdrain for infiltration rates less than 15 mm/hr. Sandy loam soils typically have infiltration rates from 30 to 50 mm/hr. and therefore underdrains are not anticipated to be required. This assumption can be confirmed once the in-situ infiltration rates have been established during detailed design.

A variety of LID features were considered for the development and evaluated based on site constraints, capital cost, maintenance considerations and water quality benefits. The preferred design selected are stone filled infiltration trenches with detention storage provided above the infiltration portion where required. These features will be incorporated as part of the roadside ditch system. The SWM facilities have been designed to achieve the necessary quality control targets.

4.2 Stormwater Quality Control

The proposed subdivision may cause additional pollutants to be conveyed off site. As such, the selection and sizing of the water quality measures are based on the procedures set out in the Stormwater Management Planning and Design Manual (MOE, March 2003) for Enhanced (Level 1) protection. As such, the goal of SWM is to preserve the natural hydrologic cycle. In addition, SWM measures should be assessed in the following order:

- Stormwater lot level controls
- Stormwater conveyance controls
- End-of-pipe SWM facilities

Stormwater lot level controls represent measures that are implemented on an individual lot basis such as soak-a-way pits, flatter grading, and reduction of the impervious footprint.

Stormwater conveyance controls represent the conveyance systems used to transport stormwater runoff from the lots to the receiving waters such as pervious pipes, catchbasin treatment and grassed swales.

End-of-pipe SWM facilities represent the common urban SWM measures used to service numerous lots or whole subdivisions including wet ponds, wetlands, dry ponds, infiltration-based facilities, Oil-Grit separators and filter systems.

4.2.1 Quality Control Summary

A SWM assessment was completed to evaluate the most appropriate measures to provide stormwater quality treatment for the proposed development. The following table summarizes the feasibility of each option for the proposed development.

Table 2 – Quality Control Feature Options Summary

Facility Description	Comments and Feasibility
Lot Level Controls • Soak-Away Pits	• Not a feasible option as a standalone water quality control for roadway runoff.
Conveyance Controls • Pervious pipes • Catchbasin treatment • Grass Swales	• Feasible option for water quality control. • Grassed swales can be considered as a viable option in combination with infiltration features to provide the necessary water quantity control.
End-of-Pipe Controls • Wetlands • Wet Ponds • Dry Ponds • Infiltration Basins • Infiltration Trenches • Filter Strips • Sand Filters • Oil-Grit separators (OGS)	• Not a feasible option due to site constraints and multiple outlets. • Wetlands are not feasible with available site area and configuration. • The increase in impervious area is not sufficient to consider a wet or dry pond facility as a viable option. • An OGS is not considered a viable option due to the outlet grade and configuration. • Infiltration basin/trench can be considered as viable options in combination with a conveyance feature to provide water quality control.

Facility Description	Comments and Feasibility
	• Filter systems are typically cumbersome, expensive and require extensive maintenance at regular intervals.

A review of the above-mentioned features indicates that a hybrid feature incorporating conveyance and infiltration is an appropriate method for achieving the necessary quality control required.

Based on Table 3.2 of the Stormwater Management Planning and Design Manual, the proposed SWM facilities were designed to provide the necessary "Enhanced" level of protection for the proposed development. Table 3 provides a summary of the proposed SWM features and demonstrates how water quality protection can be achieved for the selected sub-catchments.

Table 3 – Proposed Water Quality Treatment Summary

Description	Quality Control Feasibility
Catchment – PR-100 • Stone filled infiltration trench located within roadside ditch system.	• To achieve Level 1 (enhanced) protection, a storage volume of 66.5 m^3 is required based on Table 3.2 of the SWM Manual (2.66 ha x 25 m^3/ha). • Water quality target will be achieved by the provision of an outlet located above the dedicated stone filled infiltration layer.
Catchment – PR-102 • Stone filled infiltration trench located within roadside ditch system.	• To achieve Level 1 (enhanced) protection, a storage volume of 37 m^3 is required based on Table 3.2 of the SWM Manual (1.48 ha x 25 m^3/ha). • Water quality target will be achieved by the provision of an outlet located above the dedicated stone filled infiltration layer.
Catchment – PR-200 • Stone filled infiltration trench located within roadside ditch system.	• To achieve Level 1 (enhanced) protection, a storage volume of 19 m^3 is required based on Table 3.2 of the SWM Manual (0.75 ha x 25 m^3/ha). • Water quality target will be achieved by the provision of an outlet located above the dedicated stone filled infiltration layer.
Catchment – PR-201 • Stone filled infiltration trench located within roadside ditch system.	• To achieve Level 1 (enhanced) protection, a storage volume of 32 m^3 is required based on Table 3.2 of the SWM Manual (1.28 ha x 25 m^3/ha). • Water quality target will be achieved by the provision of an outlet located above the dedicated stone filled infiltration layer.

4.3 Stormwater Quantity Control

4.3.1 Peak Flow Calculations

Peak flows were estimated using Visual Otthymo version 3.0 (VO3) hydrologic modelling software

for each of the 2, 5, 10, 25, 50 and 100-year storm events. These calculations consider the 6-hour SCS storm duration as prescribed by the City of Peterborough Engineering Design Standards (April 2019). The schematic layout of the VO3 model and the simulation results are presented in Appendix B.

Table 4 – Existing and Uncontrolled Peak Flow Rates – OUT 1 and OUT 2

Return Period	Peak Flow Rates (m^3/s)							
	OUT 1 WEST		OUT 1 EAST		OUT 2 WEST		OUT 2 EAST	
	EX[1]	UNC[2]	EX[1]	UNC[2]	EX[1]	UNC[2]	EX[1]	UNC[2]
2-Year	0.042	0.063	0.020	0.081	0.025	0.017	0.056	0.092
5-Year	0.078	0.117	0.042	0.133	0.052	0.034	0.110	0.149
10-Year	0.107	0.159	0.060	0.171	0.073	0.046	0.151	0.196
25-Year	0.147	0.216	0.086	0.238	0.103	0.064	0.210	0.257
50-Year	0.180	0.264	0.107	0.283	0.128	0.078	0.257	0.320
100-Year	0.215	0.313	0.129	0.330	0.154	0.093	0.306	0.374

Notes: 1. EX refers to the existing development condition. (NHYD = 1, 2, 3, 4)

2. UNC refers to the proposed conditions without SWM controls. (NYHD = 6, 9, 13, 12)

As shown in Table 4, the proposed uncontrolled flows to OUT 1 WEST, OUT 1 EAST and OUT 2 EAST exceed the existing condition levels without the use of SWM controls, and therefore stormwater quantity controls will be required for these outlet locations. The proposed uncontrolled flows to OUT 2 WEST are reduced due to a reduction in drainage area discharging to that location in the proposed condition. As a result, stormwater quantity controls will not be required for OUT 2 WEST. The outlet locations are shown on Figure 2 and Figure 3.

Table 5 – Existing and Uncontrolled Peak Flow Rates – OUT 3 and OUT 4

Return Period	Peak Flow Rates (m^3/s)			
	OUT 3		OUT 4	
	EX[1]	UNC[2]		
2-Year	0.039	0.036	0.020	0.011
5-Year	0.077	0.067	0.040	0.021
10-Year	0.107	0.092	0.056	0.028
25-Year	0.148	0.125	0.078	0.038
50-Year	0.181	0.153	0.096	0.047
100-Year	0.216	0.181	0.115	0.055

Notes: 1. EX refers to the existing development condition. (NHYD = 5, 55)

2. UNC refers to the proposed condition without SWM controls. (NYHD = 14, 56)

In the post development condition for Outlet 3 (OUT 3), a portion of the overall developed Subject Property, including six residential homes and backyards, will be directed to the outlet. In the post development condition for Outlet 4 (OUT 4), a portion of the rear yards at the southern tip of the property will be directed to the outlet. A review of the hydrologic parameters and modelling indicates a reduction in drainage area to both OUT 3 and OUT 4, offsetting the increase in impervious area. As a result, and as outlined in Table 5, there is no increase in peak flow rates discharging to OUT 3 and OUT 4, and therefore no stormwater quantity controls will be required for either outlet.

4.3.2 Quantity Control Summary

In accordance with the design criteria established in Section 3.1, quantity controls are required to ensure that post-development flow rates do not exceed existing conditions, for each outlet, up to the 100-year storm. The VO3 model is used to estimate the required storage volume to provide the necessary quantity control without overtopping. A preliminary design of the combination stone filled infiltration trenches/detention ditches have been completed to confirm that sufficient stage storage can be provided. The flow regulation can be achieved by means of an outlet control structure for each facility.

4.4 SWM Facilities

SWM facilities will be required for the development to achieve the necessary quality and quantity control targets. As discussed in Table 3, stone filled infiltration trenches will provide the necessary water quality control for each outlet location. As necessary, an open detention portion will be provided above the infiltration trench as part of the

roadside ditch system and will be controlled by a dedicated outlet structure within each facility.

4.4.1 Design Overview

The preliminary selection of the SWM facilities was designed based on the required volume targets, outlet configuration and site topography. A review of Table 6 provides a summary of each SWM facility for each outlet location.

Table 6 – Proposed SWM Facility Summary

Catchment	Outlet ID	Design Summary
PR-100	OUT-1 WEST	• 1.0 m deep stone filled infiltration trench, 2.0 m in width and 90 m in length. • Required storage volume of stone filled trench for quality control is 66.5 m^3, with 72 m^3 provided. • Total storage volume required for quantity control is 261 m^3, with 278 m^3 provided at 0.6 m depth. • One stage outlet control is assumed in the preliminary calculations, multi-stage will be considered during detailed design to maximize outlet efficiency and reduce facility size.
PR-102	OUT-1 EAST	• 1.0 m deep stone filled infiltration trench, 2.0 m in width and 97 m in length. • Required storage volume of stone filled trench for quality control is 37 m^3, with 74 m^3 provided. • Total storage volume required for quantity control is 433 m^3, with 435 m^3 provided at 0.8 m depth. • One stage outlet control is assumed in the preliminary calculations, multi-stage will be considered during detailed design to maximize outlet efficiency and reduce facility size.
PR-200	OUT-2 WEST	• 0.5 m deep stone filled infiltration trench, 2.0 m in with and 50 m in length. • Required storage volume of stone filled trench for quality control is 19 m^3, with 21 m^3 provided. • Stormwater quantity control not required for PR-200.
PR-201	OUT-2 EAST	• 1.0 m deep stone filled infiltration trench, 2.0 m in width and 107 m in length. • Required storage volume of stone filled trench for quality control is 32 m^3, with 52 m^3 provided.

Catchment	Outlet ID	Design Summary
		• Total storage volume required for quantity control is 200 m^3, with 301 m^3 provided at 0.6 m depth. • One stage outlet control is assumed in the preliminary calculations, multi-stage will be considered during detailed design to maximize outlet efficiency and reduce facility size.

A review of Error! Reference source not found.6 indicates that sufficient storage volumes can be provided within the SWM facilities to meet the LID, water quality and water quantity control requirements. A geotechnical study of the site is recommended during the detailed design phase to determine actual infiltration rates and groundwater levels to confirm the final dimensions of the proposed facility and to include a shallow infiltration bottom to the proposed facilities.

4.4.2 Proposed Release Rates

The proposed peak flow rates for from the preliminary SWM facilities with respect to design storm events are shown in Table 7 below.

Table 7 – Existing and Proposed Peak Flow Summary

Return Period	Peak Flow Rates (m^3/s)							
	OUT 1 WEST		OUT 1 EAST		OUT 2 WEST		OUT 2 EAST	
	EX[1]	PR[2]	EX[1]	PR[2]	EX[1]	PR[2]	EX[1]	PR[2]
2-Year	0.042	0.020	0.020	0.020	0.025	0.017	0.056	0.056
5-Year	0.078	0.068	0.042	0.038	0.052	0.034	0.110	0.110
10-Year	0.107	0.104	0.060	0.049	0.073	0.046	0.151	0.149
25-Year	0.147	0.140	0.086	0.065	0.103	0.064	0.210	0.194
50-Year	0.180	0.165	0.107	0.077	0.128	0.078	0.257	0.232
100-Year	0.215	0.186	0.129	0.089	0.154	0.093	0.306	0.268

Notes: 1. EX refers to the existing development condition (NHYD = 1, 2, 3, 4).

2. PR refers to the proposed conditions with controlled flows from the proposed control facility (NHYD = 16, 20, 13, 24).

A review of Table 7 indicates the proposed SWM facilities will provide the required quantity controls such that the proposed development will not increase peak flow rates at each outlet location. Stage-Storage-Discharge calculations are provided for each facility in Appendix D. It is noted that these facilities have been conservatively designed using a single outlet control. During detailed design, the outlet configuration and infiltration storage depth for each facility will be optimized to reduce the storage volume requirements.

5.0 Conclusion

As the proposed residential subdivision will alter existing drainage patterns, a preliminary SWM report has been prepared to address the requirements of the County and ORCA.

LID considerations and stormwater quality controls such as stone filled infiltration trenches within the roadway corridor can be provided to achieve "Enhanced" Level 1 protection as defined in the Stormwater Management Planning and Design Manual (March 2003).

Water quantity controls can be provided by the stone filled infiltration ditches within the roadway corridor with outlet control structures providing detention control to existing condition levels.

If you require any further information, or have any questions, please do not hesitate to contact the undersigned.

Respectfully submitted,

Ken Smith, P.Eng.
Manager, Water Resources Department

Gavin Bergsma, C.E.T.
Project Designer

KS/GB/af

Statement of Limitations

This report has been prepared by D.M. Wills Associates Limited on behalf of Mr. Rubal Kundra of Life at the Woodland Inc. to address the requirements of the County of Peterborough and ORCA.

The conclusions and recommendations in this report are based on available background documentation and discussions with applicable agencies at the time of preparation.

The report is intended to determine the feasibility of the proposed development with respect to Stormwater Management of the Site. The design information provided in this report is preliminary in nature and should not be used for site plan application or construction purposes.

Any use that a third party makes of this report other than a Preliminary Stormwater Management Report for the proposed development is the responsibility of such third parties. D.M. Wills Associates Limited accepts no responsibility for damages, if any, suffered by a third party as a result of decisions made or action taken based on using this report for purposes other than a Preliminary Stormwater Management Report for the proposed Life at the Woodland residential subdivision.

Appendix A

Rainfall Data and Hydrology

B.1 Storm Sewer System

City of Peterborough Standard Drawings and Specifications shall be adhered to at all times. Where the City has no standard or specification, the City of Peterborough recognizes the July 1984 release of the Ministry of Environment Guidelines for the Design of Storm Sewer Systems as the guidelines by-which all storm sewer work shall conform to. Otherwise, OPSS or OPSD shall apply.

B.1.1 Definition and Use

Storm systems may consist of one or any combination of pipes, ditches, culverts, open channels and storm water management facilities that convey storm water.

Storm sewers shall be designed to collect storm water discharge from pervious and impervious areas both on private and public lands. Storm drainage connected to buildings on private property requires a building permit before installation.

B.1.2 Location and Alignment

All works to be constructed within a City road allowance are to be located in accordance with the appropriate City of Peterborough Drawing from standards USD100.01 thru USD100.13. The City must approve locating works in non-standard locations.

Storm sewers connected to buildings on private property are regulated by Part 7 of the OBC. Where there are no specific regulations in the OBC, details from this document shall apply.

Connections to existing storm sewer systems shall be made at manholes. Where no manhole is present a new manhole is required on the City's main. Connections between storm sewer pipes on private property, where not at a manhole or catchbasin, shall be made with manufactured 'Tees" and only with approval from the City engineering department and only if sewer is not defined as a *building sewer* 200mm or greater. Control manholes for private sewers shall be placed on the front property line or just inside the property if placement on the property line is not possible.

B.1.3 Drainage / Sub Drainage Area Plans

Drainage/sub-drainage area limits for sewer designs are to be in accordance with approved grading plans to the proposed maintenance holes (or catchbasins if applicable) located on the R.O.W.

Note: All areas and runoff coefficients are to be shown for each drainage/sub-drainage areas.

B.1.4 External Watershed Limits and Drainage Areas

When design abuts undeveloped areas, identify the external watershed limit to be designed for, typically following contour lines. Developed external areas should encompass the entire sewershed.

Note: All areas, runoff coefficients and time of concentrations are to be shown for all drainage areas within external watershed limits.

B.1.5 Design Chart

Storm sewer design calculations are to be completed on an appropriate Storm Sewer Design chart.

B.1.6 Design Requirements and Location

The City of Peterborough rainfall data based upon 2002 AES Peterborough Airport IDF shall be used for the development of rainfall intensities for storm sewer design. Alternatively, IDF curve parameters A, B, and C provided in Table B.1.7.1 may be used.

Storm sewers shall be designed as a separate sewer system. Effluent from sanitary sewers or any potentially contaminated drainage from industrial, agricultural or commercial operations shall not be discharged into storm sewers. Contaminated drainage means, the introduction of any foreign, undesirable physical, chemical or biological substance into the environment, which results or is likely to result in deleterious effects.

Storm sewers shall be designed as deep storm sewers with approximately 3.0m of cover where private gravity service connections are required. An acceptable alternative to the deep sewer system is the provision of a separate foundation drainage system (generally with smaller pipes) with gravity service connections as well as a traditional storm sewer system with shallow pipes. Where physical constraints do not allow for either above noted system, a shallow system may be approved with pipe typically 1.25m deep.

The alignment of the storm varies depending on the type of road and whether or not deep storm sewers are to be implemented. Typically, deep storm sewers are offset 1.5m on one side of the centreline with the sanitary sewer offset 1.5m from the other side of the centreline. Shallow storm sewers, particularly those less than 450mm in diameter, are typically located 0.31 metres within the curb face. Storm sewers are generally to the north or west of the centerline.

MOE Guidelines require 0.5m vertical or 2.5m horizontal clearances between sewers and watermains. In some circumstances it may be prudent for the PUC/City jointly to decide by what means/best practice one main crosses the other in addition to MOE

design guidelines. Watermain bends may be required at sewer/watermain pipe crossings while sometimes it may be more appropriate to change the grade of a sewer, depending on the circumstances.

The capacity of the minor storm sewer systems shall typically be designed to carry the peak flow resulting from a one (1) in five (5) year rainfall event. Where gravity foundation service connections exist, the five year design flow must not exceed 80% of the just full pipe capacity.

Where gravity service connections exist or are proposed, a 100 year hydraulic grade line analysis must be undertaken, to determine peak 100 year water levels within a potentially surcharged storm sewer system. A minimum freeboard of 0.50m must be provided between the computed 100 year hydraulic grade line and the minimum basement floor level in conjunction with the use of backflow preventers. Where the change in grade has the potential for hydraulic jumps, additional freeboard shall be provided. The implementation of flow regulating ICDs may be permitted by the City Engineer to reduce 100 year storm sewer flows on a case by case basis only at low points in the road, however in all instances, the 5 year flow must pass through the ICD unencumbered with minimal backwater head and no surface ponding. Pipe sizes should be increased elsewhere to accommodate the 100 year flow with appropriate hydraulic grade line elevations.

The major system design shall be based on a one in 100 year rainfall event and should include assessment of road sags and boulevard overflows into stormwater management ponds and watercourses. The maximum ponding depth shall not exceed 300mm as measure to the centerline of the road.

B.1.7 Peak Flow Calculation and Storm Sewer Design

The design area shall include all areas, which reasonably or naturally drain to the system. To calculate the peak rate of runoff from an area, the Rational Method shall be used as follows:

$$Q = K \cdot A \cdot i \cdot C$$

Where:

‘Q’ - is peak flow (l/s)
‘K’ - is 2.78
‘A’ - is the area (hectares)
‘i’ - is the rainfall intensity (mm/hour) calculated as follows:

$$i = \frac{A}{(Tc + B)^C}$$

or as directly derived from 2002 Peterborough Airport IDF curves

Where: A, B and C are per Table B.1.7.1.

Table B.1.7.1

	A	B	C
2 Year	662	7.5	0.79
5 Year	1098	10.1	0.83
10 Year	1560	13	0.86
25 Year	2010	14	0.88
50 Year	2200	14.6	0.87
100 Year	2507	14.8	0.88

Please note that the above A, B, C values shall be used for storm sewer design only, or other calculations design storms of less than 3 hours duration. Stormwater Management Reports should use rainfall directly from the 2006 Peterborough Airport IDF curves.

The time of concentration (Tc) should be calculated rather than relying upon arbitrary minimum and maximum times. Where this is not practical, a ten (10) minute time of concentration (Tc) shall be used except when the zoning requires the use of a runoff coefficient of 0.75 or higher in which case five (5) minute time of concentration (Tc) shall be used.
'C' - is the co-efficient of run-off

The runoff co-efficient or "C" for storm drainage unless otherwise specified or adequately proven through calculation, shall be per Table B.1.7.2 for the 5 year design.

Table B.1.7.2

Parks – over 4.0 ha	0.20
Parks – 4.0 ha and under	0.25
Single family residential-15m lots	0.55
Single family residential-12m lots	0.65
Single family residential-9m lots	0.75
Semi-detached	0.80
Townhouses	0.85
Apartments	0.90
Schools and Churches / Industrial	Varies
Commercial	0.90
Heavily Developed Areas	0.90
Asphalt, Concrete, Roofed Areas	0.95

25 to 100 year runoff co-efficients shall be increased to account for soil saturation. Increase co-efficients for the 25, 50, and 100 year storms by 10%, 20%, and 25% respectively up to a maximum value of 0.95.

B.1.7.1 Flow Velocities and Minimum Slope

Storm sewer flow velocities shall not be less than 0.8 m/s when flowing full or 0.6m/s at design depth. Minimum longitudinal slope shall be 0.5%. The preferred maximum velocity is 3.0 m/s with an absolute maximum acceptable velocity of 6.0 m/s.

B.1.7.2 Pipe Cover

The minimum depth of cover to pipe crown shall be 1.2 m. Excessive cover should be avoided except under special circumstances.

Minimum pipe cover for deep sewers shall be based upon providing gravity connections including hydraulic grade line freeboard for storm services.

For concrete pipe, the maximum allowable cover permitted on concrete pipe to be constructed is to be based on OPSD 807.010, 807.030, 807.040 and 807.050. Where the pipe required exceeds the OPSD charts, a pipe design sealed by a Professional Engineer must be submitted to the City for approval.

For flexible pipe, the maximum allowable cover permitted shall be as per OPSD or manufacturers specifications.

B.1.7.3 Gravity Pipe Design

The minimum pipe diameter for storm sewers shall be 250 mm. Minimum pipe diameter for catchbasins leads shall be 250mm with the exception of double catchbasins which shall be 300mm.

The obvert of the inlet pipe at all maintenance holes shall be higher than or equal to the obvert of the outlet pipe. Appropriate invert elevation drops to account for the velocity head, transition and bend losses within manholes should be provided.

An outlet pipe from a manhole is not permitted to be smaller than the incoming pipe even if the outlet pipe has adequate capacity due to greater slope. Allowances may be granted by the City Engineer in special circumstances to allow a smaller outlet pipes on privately owned property in the event other regulatory agencies (e.g. MTO) require this for stormwater retention.

The Manning equation shall be used to calculate the required hydraulic capacity of a gravity sewer as follows:

6 Hour SCS Type II Intensity Hyetographs
2006 Peterborough Airport Weather Station
(mm/hr)

Time (min.)	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
0	0	0	0	0	0	0
15	1.6	2.1	2.5	2.9	3.3	3.6
30	1.6	2.1	2.5	2.9	3.3	3.6
45	2.3	3.2	3.7	4.4	4.9	5.4
60	2.3	3.2	3.7	4.4	4.9	5.4
75	2.3	3.2	3.7	4.4	4.9	5.4
90	2.3	3.2	3.7	4.4	4.9	5.4
105	3.9	5.2	6.2	7.3	8.1	9.0
120	3.9	5.2	6.2	7.3	8.1	9.0
135	4.6	6.3	7.4	8.8	9.8	10.8
150	4.6	6.3	7.4	8.8	9.8	10.8
165	23.2	31.4	36.9	43.7	48.9	53.9
180	60.4	81.78	95.9	113.7	127.0	140.2
195	8.5	11.5	13.5	16.0	17.9	19.8
210	8.5	11.5	13.5	16.0	17.9	19.8
225	3.9	5.2	6.2	7.3	8.1	9.0
240	3.9	5.2	6.2	7.3	8.1	9.0
255	3.1	4.2	4.9	5.8	6.5	7.2
270	3.1	4.2	4.9	5.8	6.5	7.2
285	2.3	3.2	3.7	4.4	4.9	5.4
300	2.3	3.2	3.7	4.4	4.9	5.4
315	1.6	2.1	2.5	2.9	3.3	3.6
330	1.6	2.1	2.5	2.9	3.3	3.6
345	1.6	2.1	2.5	2.9	3.3	3.6
360	1.6	2.1	2.5	2.9	3.3	3.6

Hydrologic Parameters for EX-100

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: MW / CPB
Date: 22-Feb-22

Land Use

	Internal	External	
Agriculture	0.00	0.00	ha
Range	0.00	0.00	ha
Grass	0.35	1.47	ha
Woods	0.10	0.06	ha
Wetland	0.00	0.00	ha
Gravel	0.00	0.00	ha
Impervious	0.00	0.04	ha
SUM	**0.45**	**1.57**	
Hydrologic Soil Group[1]	B	B	
Soil Type	0	0	
C	0.21	0.21	
CN (Nashyd)	60.3	61.8	

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**2.02**	ha
Impervious Area	**0.04**	ha
Percent Impervious	**1.9%**	
Connected Impervious	1.9%	

	Pervious	Impervious	
Length	125	5	m
US Elev	226.5	207.5	m
DS Elev	207.5	207.4	m
Slope	15.2	2.0	%
	Steep	Flat	

Parameter	Soil Group	Land Use							Weighted Value	
		Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Incl. Imperv. NASHYD	Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	**B**	0.57	0.35	0.19	0.29	0.05	0.76	0.90	0.21	
	B	0.57	0.35	0.19	0.29	0.05	0.76	0.90	0.21	n.a.
SCS Curve No.[3]**, CN**	**B**	74	65	61	58	50	85	98	60.3	60.3
	B	74	65	61	58	50	85	98	61.8	60.9
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	5.3	5.4

Time of Concentration[6]

Pervious Length	125	m	
Slope	15.2	%	
Airport	13.2	min.	
Bransby - Williams	3.9	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	2.02 ha	
Runoff Coefficient	0.21	
SCS Curve No.	61.5	60.8
Modified Curve No.[4]**, CN***	**61.5**	60.7
Initial Abstraction.	**5.3**	5.4

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for EX-101

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: MW / CPB
Date: 22-Feb-22

Land Use

Agriculture	0.00	0.00	ha
Range	0.21	0.07	ha
Grass	0.08	0.00	ha
Woods	0.96	0.07	ha
Wetland	0.00	0.00	ha
Gravel	0.00	0.00	ha
Impervious	0.03	0.00	ha
SUM	**1.27**	**0.14**	
Hydrologic Soil Group[1]	B	B	
Soil Type	0	0	
C	0.15	0.17	
CN (Nashyd)	60.4	61.5	

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**1.42**	ha
Impervious Area	**0.03**	ha
Percent Impervious	**2.4%**	
Connected Impervious	0.2%	

Pervious

Length	110	m
US Elev	212.2	m
DS Elev	207.5	m
Slope	4.3	%
	Rolling	

Parameter	Soil Group	Land Use: Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Weighted Value: Incl. Imperv. NASHYD	Weighted Value: Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	**B**	0.32	0.22	0.13	0.11	0.05	0.76	0.90	0.15	
	B	0.32	0.22	0.13	0.11	0.05	0.76	0.90	0.17	n.a.
SCS Curve No.[3]**, CN**	**B**	74	65	61	58	50	85	98	60.4	59.4
	B	74	65	61	58	50	85	98	61.5	61.5
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	9.1	9.3

Time of Concentration[6]

Total Length	110	m	
Average Slope	4.3	%	
Airport	**20.1**	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Bransby - Williams	4.5	min.	
Applicable Minimum[7]	15.0	min.	
Time to Peak	13.5	min.	
	0.22	**hr.**	

Composite Parameters

Drainage Area	1.42 ha	
Runoff Coefficient	0.15	
SCS Curve No.	60.5	59.6
Modified Curve No.[4]**, CN***	**63.1**	62.7
Initial Abstraction.	**9.1**	9.3

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. Connected Impervious is estimated using the Sutherland Equation with a Watershed Selection Criteria of Somewhat Connected

Hydrologic Parameters for EX-200

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: MW / CPB
Date: 22-Feb-22

Land Use

	Internal	External	
Agriculture	0.00	0.00	ha
Range	0.00	0.00	ha
Grass	0.27	0.13	ha
Woods	1.04	0.09	ha
Wetland	0.00	0.00	ha
Gravel	0.00	0.00	ha
Impervious	0.00	0.00	ha
SUM	**1.31**	**0.22**	
Hydrologic Soil Group[1]	B	B	
Soil Type	0	0	
C	0.27	0.23	
CN (Nashyd)	58.6	59.8	

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**1.53**	ha
Impervious Area	**0.00**	ha
Percent Impervious	**0.0%**	
Connected Impervious	0.0%	
	Pervious	
Length	130	m
US Elev	225.9	m
DS Elev	211.6	m
Slope	11.0	%
	Steep	

Parameter	Soil Group	Land Use							Weighted Value	
		Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Incl. Imperv. NASHYD	Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	**B**	0.57	0.35	0.19	0.29	0.05	0.76	0.90	0.27	
	B	0.57	0.35	0.19	0.29	0.05	0.76	0.90	0.23	n.a.
SCS Curve No.[3]**, CN**	**B**	74	65	61	58	50	85	98	58.6	58.6
	B	74	65	61	58	50	85	98	59.8	59.8
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	8.7	8.7

Time of Concentration[6]

Total Length	130	m	
Average Slope	11.0	%	
Airport	14.1	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Bransby - Williams	4.4	min.	
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	1.53 ha	
Runoff Coefficient	0.26	
SCS Curve No.	58.8	58.8
Modified Curve No.[4]**, CN***	**61.7**	61.7
Initial Abstraction.	**8.7**	8.7

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for EX-201

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: MW / CPB
Date: 22-Feb-22

Land Use

	Internal	External	
Agriculture	0.00	0.00	ha
Range	2.16	0.24	ha
Grass	0.00	0.00	ha
Woods	0.02	0.00	ha
Wetland	0.00	0.00	ha
Gravel	0.00	0.00	ha
Impervious	0.00	0.02	ha
SUM	**2.18**	**0.26**	
Hydrologic Soil Group[1]	B	B	
Soil Type	0	0	
C	0.30	0.34	
CN (Nashyd)	64.9	67.3	

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**2.44**	ha
Impervious Area	**0.02**	ha
Percent Impervious	**0.7%**	
Connected Impervious	0.7%	

Pervious

Length	100	m
US Elev	213.2	m
DS Elev	206.6	m
Slope	6.6	%
	Hilly	

Parameter	Soil Group	Land Use							Weighted Value	
		Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Incl. Imperv. NASHYD	Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	**B**	0.40	0.30	0.19	0.14	0.05	0.76	0.90	0.30	
	B	0.40	0.30	0.19	0.14	0.05	0.76	0.90	0.34	n.a.
SCS Curve No.[3]**, CN**	**B**	74	65	61	58	50	85	98	64.9	64.9
	B	74	65	61	58	50	85	98	67.3	65.0
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	8.0	8.0

Time of Concentration[6]

Total Length	100	m	
Average Slope	6.6	%	
Airport	13.9	min.	
Bransby - Williams	3.6	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	2.44 ha	
Runoff Coefficient	0.30	
SCS Curve No.	65.2	64.9
Modified Curve No.[4]**, CN***	**68.8**	68.5
Initial Abstraction.	**8.0**	8.0

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for EX-300

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: GB/KS
Date: 22-Feb-22

Land Use

	Internal	
Agriculture	0.00	ha
Range	1.73	ha
Grass	0.00	ha
Woods	0.00	ha
Wetland	0.00	ha
Gravel	0.00	ha
Impervious	0.00	ha
SUM	**1.73**	

Hydrologic Soil Group[1]	B
Soil Type	0
C	0.30
CN (Nashyd)	65.0

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**1.73**	ha
Impervious Area	**0.00**	ha
Percent Impervious	**0.0%**	
Connected Impervious	0.0%	

Pervious

Length	55	m
US Elev	214.0	m
DS Elev	210.0	m
Slope	7.3	%
	Hilly	

Parameter	Soil Group	Land Use							Weighted Value	
		Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Incl. Imperv. NASHYD	Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	**B**	0.40	0.30	0.19	0.14	0.05	0.76	0.90	0.30	n.a.
SCS Curve No.[3]**, CN**	**B**	74	65	61	58	50	85	98	65.0	65.0
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	8.0	8.0

Time of Concentration[6]

Total Length	55	m	
Average Slope	7.3	%	
Airport	10.0	min.	
Bransby - Williams	2.0	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	1.73 ha	
Runoff Coefficient	0.30	
SCS Curve No.	65.0	65.0
Modified Curve No.[4]**, CN***	**68.6**	68.6
Initial Abstraction.	**8.0**	8.0

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for EX-400

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: RC/CPB
Date: 22-Feb-22

Land Use

	Internal	
Agriculture	0.00	ha
Range	1.09	ha
Grass	0.00	ha
Woods	0.15	ha
Wetland	0.00	ha
Gravel	0.00	ha
Impervious	0.00	ha
SUM	**1.24**	

Hydrologic Soil Group[1]	B
Soil Type	0
C	0.21
CN (Nashyd)	64.2

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**1.24**	ha
Impervious Area	**0.00**	ha
Percent Impervious	**0.0%**	
Connected Impervious	0.0%	

Pervious

Length	165	m
US Elev	211.6	m
DS Elev	204.8	m
Slope	4.1	%
	Rolling	

Parameter	Soil Group	Land Use							Weighted Value	
		Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Incl. Imperv. NASHYD	Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	**B**	0.32	0.22	0.13	0.11	0.05	0.76	0.90	0.21	n.a.
SCS Curve No.[3]**, CN**	**B**	74	65	61	58	50	85	98	64.2	64.2
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	8.2	8.2

Time of Concentration[6]

Total Length	165	m	
Average Slope	4.1	%	
Airport	**23.5**	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Bransby - Williams	6.9	min.	
Applicable Minimum[7]	15.0	min.	
Time to Peak	15.7	min.	
	0.26	**hr.**	

Composite Parameters

Drainage Area	1.24 ha	
Runoff Coefficient	0.21	
SCS Curve No.	64.2	64.2
Modified Curve No.[4]**, CN***	**67.5**	67.5
Initial Abstraction.	**8.2**	8.2

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for PR-100

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: RC/CPB
Date: 22-Feb-22

Land Use

	Internal	External	
Agriculture	0.00	0.00	ha
Range	0.00	0.00	ha
Grass	0.87	1.47	ha
Woods	0.00	0.06	ha
Wetland	0.00	0.00	ha
Gravel	0.00	0.00	ha
Impervious	0.22	0.04	ha
SUM	**1.09**	**1.57**	
Hydrologic Soil Group[1]	B	B	
Soil Type	0	0	
C	0.33	0.21	
CN (Nashyd)	68.5	61.8	

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**2.66**	ha
Impervious Area	**0.26**	ha
Percent Impervious	**9.7%**	
Connected Impervious	9.7%	

	Pervious	Impervious	
Length	125	8	m
US Elev	226.5	207.5	m
DS Elev	207.5	207.3	m
Slope	15.2	2.0	%
	Steep	Flat	

Parameter	Soil Group	Land Use							Weighted Value	
		Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Incl. Imperv. NASHYD	Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	B	0.57	0.35	0.19	0.29	0.05	0.76	0.90	0.33	
	B	0.57	0.35	0.19	0.29	0.05	0.76	0.90	0.21	n.a.
SCS Curve No.[3]**, CN**	B	74	65	61	58	50	85	98	68.5	61.0
	B	74	65	61	58	50	85	98	61.8	60.9
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	4.8	5.1

Time of Concentration[6]

Pervious Length	125	m	
Slope	15.2	%	
Airport	12.5	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Bransby - Williams	3.7	min.	
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	2.66 ha	
Runoff Coefficient	0.26	
SCS Curve No.	64.5	60.9
Modified Curve No.[4]**, CN***	**64.8**	60.6
Initial Abstraction.	**4.8**	5.1

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for PR-101

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: RC/CPB
Date: 22-Feb-22

Land Use

	Controlled Lots	Uncontrolled Lots	Roadway	External
Agriculture	0.00	0.00	ha	
Range	0.00	0.00	ha	
Grass	0.00	0.53	ha	
Woods	0.00	0.00	ha	
Wetland	0.00	0.00	ha	
Gravel	0.00	0.00	ha	
Impervious	0.00	0.06	ha	
SUM	**0.00**	**0.59**		
Hydrologic Soil Group[1]	B	B		
Soil Type	0	0		
C		0.21		
CN (Nashyd)		64.7		

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**0.59**	ha
Impervious Area	**0.06**	ha
Percent Impervious	**10.0%**	
Connected Impervious	10.0%	

	Pervious	Impervious	
Length	40	5	m
US Elev	212.2	211.0	m
DS Elev	210.2	210.9	m
Slope	5.0	2.0	%
	Rolling	Flat	

Parameter	Soil Group	Land Use: Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Weighted Value: Incl. Imperv. NASHYD	Weighted Value: Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	B	0.32	0.22	0.13	0.11	0.05	0.76	0.90		
	B	0.32	0.22	0.13	0.11	0.05	0.76	0.90	0.21	n.a.
SCS Curve No.[3]**, CN**	B	74	65	61	58	50		98		
	B	74	65	61	58	50	85	98	64.7	61.0
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	4.7	5.0

Time of Concentration[6]

Total Length	45	m	
Average Slope	4.7	%	
Airport	11.7	min.	
Bransby - Williams	2.0	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	0.59 ha	
Runoff Coefficient	0.21	
SCS Curve No.	64.7	61.0
Modified Curve No.[4]**, CN***	**65.0**	60.5
Initial Abstraction.	**4.7**	5.0

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. Connected Impervious does not include areas that discharge to pervious surfaces.

Hydrologic Parameters for PR-102

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: RC/CPB
Date: 22-Feb-22

Land Use

	Controlled Lots	Roadway	External	
Agriculture	0.00	0.00	0.00	ha
Range	0.00	0.00	0.00	ha
Grass	0.70	0.29	0.08	ha
Woods	0.00	0.00	0.07	ha
Wetland	0.00	0.00	0.00	ha
Gravel	0.00	0.00	0.00	ha
Impervious	0.08	0.23	0.03	ha
SUM	**0.78**	**0.52**	**0.18**	
Hydrologic Soil Group[1]	B	B	B	
Soil Type	0	0	0	
C	0.21	0.48	0.26	
CN (Nashyd)	64.7	77.7	66.6	

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**1.48**	ha
Impervious Area	**0.35**	ha
Percent Impervious	**23.3%**	
Connected Impervious	23.3%	

	Pervious	Impervious	
Length	40	5	m
US Elev	212.2	211.0	m
DS Elev	210.2	210.9	m
Slope	5.0	2.0	%
	Rolling	Flat	

Parameter	Soil Group	Land Use: Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Weighted Value: Incl. Imperv. NASHYD	Weighted Value: Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	**B**	0.32	0.22	0.13	0.11	0.05	0.76	0.90	0.21	n.a.
	B	0.32	0.22	0.13	0.11	0.05	0.76	0.90	0.48	
	B	0.32	0.22	0.13	0.11	0.05	0.76	0.90	0.26	
SCS Curve No.[3]**, CN**	**B**	74	65	61	58	50	85	98	64.7	61.0
	B	74	65	61	58	50	85	98	77.7	61.0
	B	74	65	61	58	50	85	98	66.6	59.6
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	4.5	5.3

Time of Concentration[6]

Total Length	45	m	
Average Slope	4.7	%	
Airport	10.4	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Bransby - Williams	1.8	min.	
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	1.48 ha	
Runoff Coefficient	0.31	
SCS Curve No.	69.5	60.8
Modified Curve No.[4]**, CN***	70.8	**60.7**
Initial Abstraction.	4.5	**5.3**

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for PR-200

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: RC/CPB
Date: 22-Feb-22

Land Use

	Internal	External	
Agriculture	0.00		ha
Range	0.00		ha
Grass	0.20		ha
Woods	0.43		ha
Wetland	0.00		ha
Gravel	0.00		ha
Impervious	0.12		ha
SUM	**0.75**		

Hydrologic Soil Group[1]	B
Soil Type	0
C	0.36
CN (Nashyd)	65.2

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**0.75**	ha
Impervious Area	**0.12**	ha
Percent Impervious	**16.1%**	
Connected Impervious	16.1%	

	Pervious	Impervious	
Length	130	8	m
US Elev	225.9	210.0	m
DS Elev	211.6	209.8	m
Slope	11.0	2.0	%
	Steep	Flat	

Parameter	Soil Group	Land Use							Weighted Value	
		Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Incl. Imperv. NASHYD	Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	**B**	0.57	0.35	0.19	0.29	0.05	0.76	0.90	0.36	n.a.
SCS Curve No.[3]**, CN**	**B**	74	65	61	58	50	85	98	65.2	59.0
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	7.4	8.4

Time of Concentration[6]

Total Length	138	m	
Average Slope	10.5	%	
Airport	13.0	min.	
Bransby - Williams	5.1	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	0.75 ha	
Runoff Coefficient	0.36	
SCS Curve No.	65.2	59.0
Modified Curve No.[4]**, CN***	**68.1**	61.7
Initial Abstraction.	**7.4**	8.4

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for PR-201

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: RC/CPB
Date: 22-Feb-22

Land Use

	Controlled Lots	Controlled Roadway	External	
Agriculture	0.00	0.00	0.00	ha
Range	0.00	0.00	0.24	ha
Grass	0.41	0.30	0.00	ha
Woods	0.00	0.00	0.00	ha
Wetland	0.00	0.00	0.00	ha
Gravel	0.00	0.00	0.00	ha
Impervious	0.05	0.25	0.02	ha
SUM	**0.46**	**0.55**	**0.26**	
Hydrologic Soil Group[1]	B	B	B	
Soil Type	0	0	0	
C	0.26	0.51	0.34	
CN (Nashyd)	64.7	77.7	67.3	

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**1.28**	ha
Impervious Area	**0.31**	ha
Percent Impervious	**24.6%**	
Connected Impervious	24.6%	

Pervious

Length	100	m
US Elev	213.2	m
DS Elev	206.6	m
Slope	6.6	%
	Hilly	

Parameter	Soil Group	Land Use: Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Weighted Value: Incl. Imperv. NASHYD	Weighted Value: Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	**B**	0.40	0.30	0.19	0.14	0.05	0.76	0.90	0.26	
	B	0.40	0.30	0.19	0.14	0.05	0.76	0.90	0.51	n.a.
	B	0.40	0.30	0.19	0.14	0.05	0.76	0.90	0.34	
SCS Curve No.[3]**, CN**	**B**	74	65	61	58	50	85	98	64.7	61.0
	B	74	65	61	58	50	85	98	77.7	61.0
	B	74	65	61	58	50	85	98	67.3	65.0
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	4.8	5.8

Time of Concentration[6]

Total Length	100	m	
Average Slope	6.6	%	
Airport	12.5	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Bransby - Williams	3.8	min.	
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	1.28 ha	
Runoff Coefficient	0.39	
SCS Curve No.	70.9	62.0
Modified Curve No.[4]**, CN***	72.2	**62.5**
Initial Abstraction.	4.8	**5.8**

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for PR-202

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: RC/CPB
Date: 22-Feb-22

Land Use

	Uncontrolled Lots	Uncontrolled Roadway	
Agriculture	0.00	0.00	ha
Range	0.00	0.00	ha
Grass	1.09	0.10	ha
Woods	0.00	0.00	ha
Wetland	0.00	0.00	ha
Gravel	0.00	0.00	ha
Impervious	0.12	0.09	ha
SUM	**1.21**	**0.19**	
Hydrologic Soil Group[1]	B	B	
Soil Type	0	0	
C	0.26	0.51	
CN (Nashyd)	64.7	77.7	

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**1.40**	ha
Impervious Area	**0.21**	ha
Percent Impervious	**14.8%**	
Connected Impervious	14.8%	

Pervious

Length	100	m
US Elev	213.2	m
DS Elev	206.6	m
Slope	6.6	%
	Hilly	

Parameter	Soil Group	Land Use							Weighted Value	
		Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Incl. Imperv. NASHYD	Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	B	0.40	0.30	0.19	0.14	0.05	0.76	0.90	0.26	
	B	0.40	0.30	0.19	0.14	0.05	0.76	0.90	0.51	n.a.
SCS Curve No.[3]**, CN**	B	74	65	61	58	50	85	98	64.7	61.0
	B	74	65	61	58	50	85	98	77.7	61.0
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	4.6	5.0

Time of Concentration[6]

Total Length	100	m	
Average Slope	6.6	%	
Airport	14.1	min.	
Bransby - Williams	3.8	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	1.40 ha	
Runoff Coefficient	0.29	
SCS Curve No.	66.5	61.0
Modified Curve No.[4]**, CN***	**67.1**	60.5
Initial Abstraction.	**4.6**	5.0

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for PR-300

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: RC/CPB
Date: 22-Feb-22

Land Use

	Internal	
Agriculture	0.00	ha
Range	0.17	ha
Grass	1.22	ha
Woods	0.00	ha
Wetland	0.00	ha
Gravel	0.00	ha
Impervious	0.13	ha
SUM	**1.52**	

Hydrologic Soil Group[1]	B
Soil Type	0
C	0.27
CN (Nashyd)	64.7

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**1.52**	ha
Impervious Area	**0.13**	ha
Percent Impervious	**8.9%**	
Connected Impervious	8.9%	

Pervious

Length	55	m
US Elev	214.0	m
DS Elev	210.0	m
Slope	7.3	%
	Hilly	

Parameter	Soil Group	Land Use							Weighted Value	
		Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Incl. Imperv. NASHYD	Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	**B**	0.40	0.30	0.19	0.14	0.05	0.76	0.90	0.27	n.a.
SCS Curve No.[3]**, CN**	**B**	74	65	61	58	50	85	98	64.7	61.5
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	5.1	5.4

Time of Concentration[6]

Total Length	55	m	
Average Slope	7.3	%	
Airport	10.5	min.	
Bransby - Williams	2.0	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Applicable Minimum[7]	**15.0**	min.	
Time to Peak	10.1	min.	
	0.17	**hr.**	

Composite Parameters

Drainage Area	1.52 ha	
Runoff Coefficient	0.27	
SCS Curve No.	64.7	61.5
Modified Curve No.[4]**, CN***	**65.4**	61.6
Initial Abstraction.	**5.1**	5.4

Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Hydrologic Parameters for PR-400

Sheet 1 of 1

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: RC/CPB
Date: 22-Feb-22

Land Use

	Internal	
Agriculture	0.00	ha
Range	0.00	ha
Grass	0.51	ha
Woods	0.00	ha
Wetland	0.00	ha
Gravel	0.00	ha
Impervious	0.07	ha
SUM	**0.58**	

Hydrologic Soil Group[1]	B
Soil Type	0
C	0.22
CN (Nashyd)	65.5

Rainfall Data

Gauging Station = Peterborough
12 hr, 100 Yr Rainfall = 90.4 mm

Drainage Area	**0.58**	ha
Impervious Area	**0.07**	ha
Percent Impervious	**12.1%**	
Connected Impervious	12.1%	

Pervious

Length	165	m
US Elev	211.6	m
DS Elev	204.8	m
Slope	4.1	%
	Rolling	

Parameter	Soil Group	Land Use							Weighted Value	
		Agriculture	Range	Grass	Woods	Wetland	Gravel	Imperv.	Incl. Imperv. NASHYD	Not Incl. Imperv. STANDHYD
Runoff Coefficient[2]**, C**	B	0.32	0.22	0.13	0.11	0.05	0.76	0.90	0.22	n.a.
SCS Curve No.[3]**, CN**	B	74	65	61	58	50	85	98	65.5	61.0
Initial Abstraction[5]**, mm**		6.0	8.0	5.0	10.0	10.0	2.5	2.0	4.6	5.0

Time of Concentration[6]

Total Length	165	m	
Average Slope	4.1	%	
Airport	**23.0**	min.	Flat: 0-2% Slopes Rolling: 2-6% Slopes Hilly: >6% Slopes
Bransby - Williams	7.5	min.	
Applicable Minimum[7]	15.0	min.	
Time to Peak	15.4	min.	
	0.26	**hr.**	

Composite Parameters

Drainage Area	0.58 ha	
Runoff Coefficient	0.22	
SCS Curve No.	65.5	61.0
Modified Curve No.[4]**, CN***	**66.1**	60.5
Initial Abstraction.	**4.6**	5.0

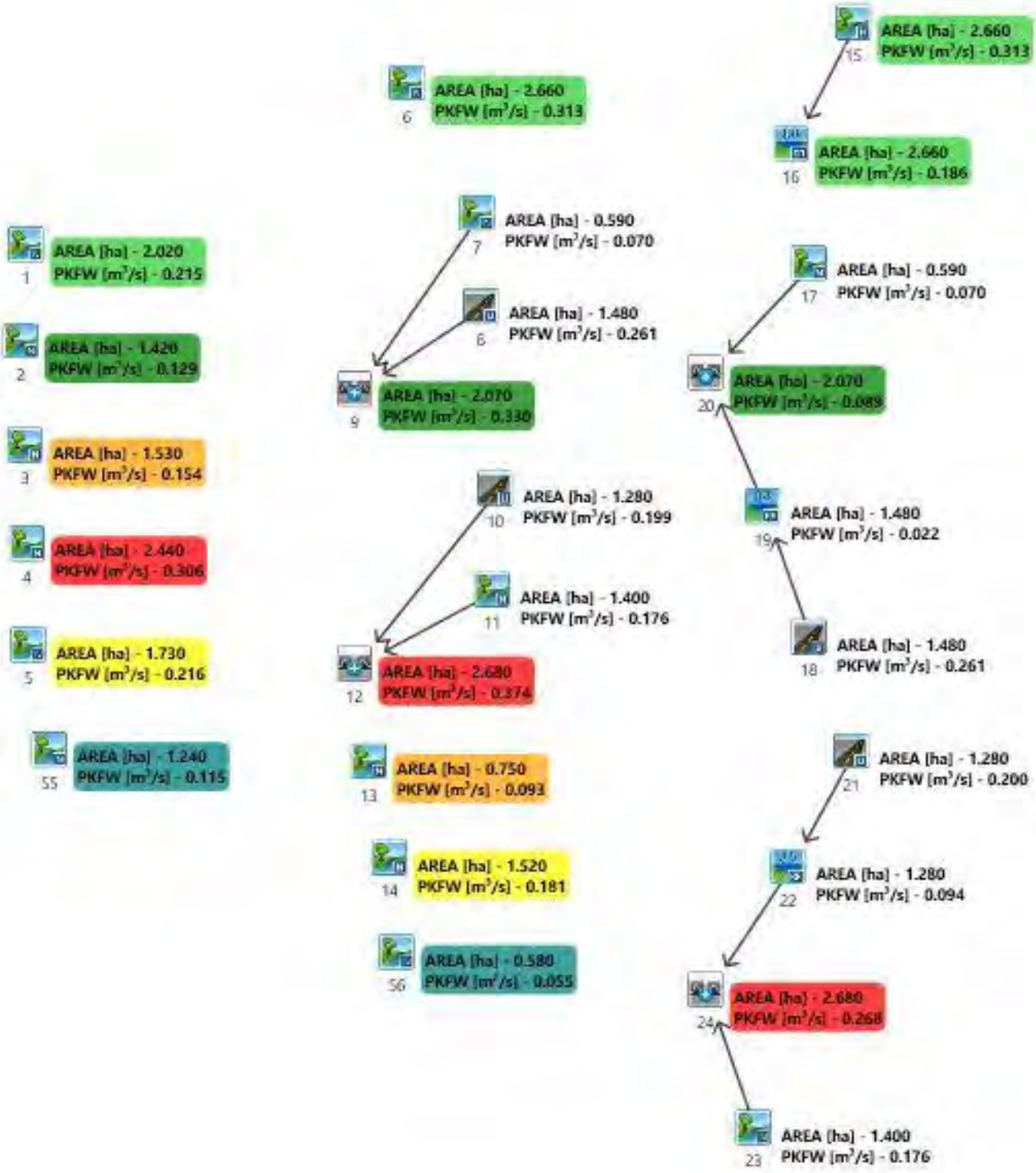
Notes:

1. Hydrologic Soil Group obtained from Design Chart H2-6A, M.T.O. Drainage Manual, 1980.
2. Runoff coefficient obtained from M.T.O. Design Chart 1.07, M.T.O. Drainage Management Manual, 1997, Hydrologic Analysis and Design, McCuen 2004 and New Jersey Technical Manual for Stream Encroachment, 1984.
3. SCS Curve No. obtained from M.T.O. Design Chart 1.09, M.T.O. Drainage Management Manual, 1997, and Table 2-2a, TR-55, page 2-5.
4. The modified curve number is adjusted as per Paul Wisner & Associates (1982) and represents anticedent moisture conditions Type II
5. Initial Abstraction values taken from the Environmental and Engineering Services Department, The Corporation of the City of London, Dec 2005
6. Use Airport Equation to calculate time of concentration for C <= 0.4, and Bransby-Williams for C > 0.4.
7. Minimum Time of Concentration for use in the Rational Method and Hydrologic Model has been set to 15 minutes
8. All impervious areas have been assumed to be directly connected.

Appendix B

Hydrologic Modelling

VO3 Analysis



```
================================================================================
======================

       V    V   I    SSSSS  U   U    A    L
       V    V   I    SS     U   U   A A   L
        V  V    I     SS    U   U  AAAAA  L
        V  V    I      SS   U   U  A   A  L
         VV     I    SSSSS  UUUUU  A   A  LLLLL

        OOO   TTTTT  TTTTT  H   H  Y   Y  M   M   OOO    TM
       O   O    T      T    H   H   Y Y   MM MM  O   O
       O   O    T      T    H   H    Y    M  M  O   O
        OOO     T      T    H   H    Y    M   M   OOO
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                  *****  D E T A I L E D   O U T P U T *****


  Input   filename: C:\Program Files (x86)\VO Suite 3.0\VO2\voin.dat

  Output  filename:
C:\Users\gbergsma\AppData\Local\Temp\2e9de3a4-1259-4d17-ae23-36803a62f8c1\Scenario.
out
  Summary filename:
C:\Users\gbergsma\AppData\Local\Temp\2e9de3a4-1259-4d17-ae23-36803a62f8c1\Scenario.
sum


DATE: 02/09/2023                          TIME: 11:25:53

USER:



COMMENTS: _____________________________________________________


--------------------------------------------------------------------------------
-------------------------
  ****************************
  ** SIMULATION NUMBER:   1 **
  ****************************


--------------------
|    READ STORM    |    Filename: C:\Users\gbergsma\AppD
|                  |              ata\Local\Temp\
|                  |              2e9de3a4-1259-4d17-ae23-36803a62f8c1\227bdd08
| Ptotal= 38.75 mm |    Comments: 2-Year, 6 hour SCS Type II - Peterboroug
--------------------
```

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	1.60	1.75	3.90	3.25	8.50	4.75	2.30
0.50	1.60	2.00	3.90	3.50	8.50	5.00	2.30
0.75	2.30	2.25	4.60	3.75	3.90	5.25	1.60
1.00	2.30	2.50	4.60	4.00	3.90	5.50	1.60
1.25	2.30	2.75	23.20	4.25	3.10	5.75	1.60
1.50	2.30	3.00	60.40	4.50	3.10	6.00	1.60

```
--------------------------------------------------------------------------------

--------------------
| CALIB            |
| NASHYD    (0002) |   Area    (ha)=   1.42   Curve Number   (CN)= 63.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   9.10   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.22

        NOTE:  RAINFALL WAS TRANSFORMED TO   5.0 MIN. TIME STEP.

                          ---- TRANSFORMED HYETOGRAPH ----
```

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.60	1.583	3.90	3.083	8.50	4.58	2.30
0.167	1.60	1.667	3.90	3.167	8.50	4.67	2.30
0.250	1.60	1.750	3.90	3.250	8.50	4.75	2.30
0.333	1.60	1.833	3.90	3.333	8.50	4.83	2.30
0.417	1.60	1.917	3.90	3.417	8.50	4.92	2.30
0.500	1.60	2.000	3.90	3.500	8.50	5.00	2.30
0.583	2.30	2.083	4.60	3.583	3.90	5.08	1.60
0.667	2.30	2.167	4.60	3.667	3.90	5.17	1.60
0.750	2.30	2.250	4.60	3.750	3.90	5.25	1.60
0.833	2.30	2.333	4.60	3.833	3.90	5.33	1.60
0.917	2.30	2.417	4.60	3.917	3.90	5.42	1.60
1.000	2.30	2.500	4.60	4.000	3.90	5.50	1.60
1.083	2.30	2.583	23.20	4.083	3.10	5.58	1.60
1.167	2.30	2.667	23.20	4.167	3.10	5.67	1.60
1.250	2.30	2.750	23.20	4.250	3.10	5.75	1.60
1.333	2.30	2.833	60.40	4.333	3.10	5.83	1.60
1.417	2.30	2.917	60.40	4.417	3.10	5.92	1.60
1.500	2.30	3.000	60.40	4.500	3.10	6.00	1.60

```
    Unit Hyd Qpeak  (cms)=   0.247

    PEAK FLOW       (cms)=   0.020 (i)
    TIME TO PEAK    (hrs)=   3.083
    RUNOFF VOLUME    (mm)=   4.927
    TOTAL RAINFALL   (mm)=  38.750
    RUNOFF COEFFICIENT   =   0.127

    (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0001) |   Area    (ha)=   2.02   Curve Number   (CN)= 61.5
|ID= 1 DT= 5.0 min |   Ia      (mm)=   5.30   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

    Unit Hyd Qpeak  (cms)=   0.454

    PEAK FLOW       (cms)=   0.042 (i)
    TIME TO PEAK    (hrs)=   3.083
    RUNOFF VOLUME    (mm)=   5.793
    TOTAL RAINFALL   (mm)=  38.750
    RUNOFF COEFFICIENT   =   0.149

    (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0015) |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

    Unit Hyd Qpeak  (cms)=   0.598

    PEAK FLOW       (cms)=   0.063 (i)
    TIME TO PEAK    (hrs)=   3.083
    RUNOFF VOLUME    (mm)=   6.680
    TOTAL RAINFALL   (mm)=  38.750
    RUNOFF COEFFICIENT   =   0.172

    (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| RESERVOIR (0016) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE    |  OUTFLOW    STORAGE
--------------------     (cms)     (ha.m.)    |   (cms)     (ha.m.)
    **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                       0.0001     0.0072    |   0.1306      0.0171
                       0.0065     0.0083    |   0.1492      0.0194
                       0.0242     0.0096    |   0.1656      0.0220
                       0.0501     0.0112    |   0.1806      0.0248
                       0.0806     0.0129    |   0.1944      0.0278
                       0.1098     0.0149    |   0.0000      0.0000

                           AREA      QPEAK      TPEAK       R.V.
                           (ha)      (cms)      (hrs)       (mm)
    INFLOW : ID= 2 (0015)  2.660      0.063      3.08        6.68
    OUTFLOW: ID= 1 (0016)  2.660      0.020      3.58        3.97

                  PEAK   FLOW   REDUCTION [Qout/Qin](%)= 31.42
                  TIME SHIFT OF PEAK FLOW        (min)= 30.00
                  MAXIMUM  STORAGE   USED       (ha.m.)=  0.0093

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0004) |   Area    (ha)=   2.44   Curve Number   (CN)= 68.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

    Unit Hyd Qpeak  (cms)=   0.548

    PEAK FLOW       (cms)=   0.056 (i)
    TIME TO PEAK    (hrs)=   3.083
    RUNOFF VOLUME    (mm)=   6.456
    TOTAL RAINFALL   (mm)=  38.750
    RUNOFF COEFFICIENT   =   0.167

    (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0003) |   Area    (ha)=   1.53   Curve Number   (CN)= 61.7
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.70   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

    Unit Hyd Qpeak  (cms)=   0.344

    PEAK FLOW       (cms)=   0.025 (i)
    TIME TO PEAK    (hrs)=   3.083
    RUNOFF VOLUME    (mm)=   4.793
    TOTAL RAINFALL   (mm)=  38.750
    RUNOFF COEFFICIENT   =   0.124

    (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0007) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
```

```
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.014 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=   6.762
     TOTAL RAINFALL   (mm)=  38.750
     RUNOFF COEFFICIENT   =   0.175

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| STANDHYD  (0008) |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
-------------------
                          IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.34         1.14
     Dep. Storage     (mm)=       1.00         5.30
     Average Slope     (%)=       2.00         5.00
     Length            (m)=      99.33        40.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      60.40         9.81
                 over (min)        5.00        20.00
     Storage Coeff.  (min)=       2.53 (ii)   16.10 (ii)
     Unit Hyd. Tpeak (min)=       5.00        20.00
     Unit Hyd. peak  (cms)=       0.29         0.06
                                                        *TOTALS*
     PEAK FLOW       (cms)=       0.06         0.02          0.067 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.25           3.00
     RUNOFF VOLUME    (mm)=      37.75         5.65          13.03
     TOTAL RAINFALL   (mm)=      38.75        38.75          38.75
     RUNOFF COEFFICIENT   =       0.97         0.15           0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------

-------------------
| ADD HYD   (0009) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------          (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0007):     0.59   0.014     3.08     6.76
        + ID2= 2 (0008):     1.48   0.067     3.00    13.03
          ================================================
          ID = 3 (0009):     2.07   0.081     3.00    11.24

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0011) |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.037 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=   7.323
     TOTAL RAINFALL   (mm)=  38.750
     RUNOFF COEFFICIENT   =   0.189

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| STANDHYD  (0010) |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min |   Total Imp(%)=  24.60   Dir. Conn.(%)=  24.60
-------------------
                          IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.31         0.97
     Dep. Storage     (mm)=       1.00         5.80
     Average Slope     (%)=       1.00         6.60
     Length            (m)=      92.38       100.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      60.40         6.94
                 over (min)        5.00        30.00
     Storage Coeff.  (min)=       2.98 (ii)   27.82 (ii)
     Unit Hyd. Tpeak (min)=       5.00        30.00
     Unit Hyd. peak  (cms)=       0.28         0.04
                                                        *TOTALS*
     PEAK FLOW       (cms)=       0.05         0.01          0.056 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.42           3.00
     RUNOFF VOLUME    (mm)=      37.75         5.86          13.69
     TOTAL RAINFALL   (mm)=      38.75        38.75          38.75
     RUNOFF COEFFICIENT   =       0.97         0.15           0.35

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------

-------------------
| ADD HYD   (0012) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------          (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0010):     1.28   0.056     3.00    13.69
        + ID2= 2 (0011):     1.40   0.037     3.08     7.32
          ================================================
          ID = 3 (0012):     2.68   0.092     3.00    10.36

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0005) |   Area    (ha)=   1.73   Curve Number   (CN)= 68.6
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.389

     PEAK FLOW       (cms)=   0.039 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=   6.409
     TOTAL RAINFALL   (mm)=  38.750
     RUNOFF COEFFICIENT   =   0.165

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0014) |   Area    (ha)=   1.52   Curve Number   (CN)= 65.4
|ID= 1 DT= 5.0 min |   Ia      (mm)=   5.10   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.342

     PEAK FLOW       (cms)=   0.036 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=   6.715
     TOTAL RAINFALL   (mm)=  38.750
     RUNOFF COEFFICIENT   =   0.173

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0006) |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.598

     PEAK FLOW       (cms)=   0.063 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=   6.680
     TOTAL RAINFALL   (mm)=  38.750
     RUNOFF COEFFICIENT   =   0.172

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0013) |   Area    (ha)=   0.75   Curve Number   (CN)= 68.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   7.40   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.169

     PEAK FLOW       (cms)=   0.017 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=   6.514
     TOTAL RAINFALL   (mm)=  38.750
     RUNOFF COEFFICIENT   =   0.168

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0017) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.014 (i)
```

```
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=   6.762
     TOTAL RAINFALL   (mm)=  38.750
     RUNOFF COEFFICIENT   =   0.175

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| STANDHYD  (0018)  |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min  |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
-------------------
                          IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.34         1.14
     Dep. Storage     (mm)=       1.00         5.30
     Average Slope     (%)=       2.00         5.00
     Length            (m)=      99.33        40.00
     Mannings n            =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      60.40         9.81
               over (min)        5.00        20.00
     Storage Coeff.  (min)=       2.53 (ii)   16.10 (ii)
     Unit Hyd. Tpeak (min)=       5.00        20.00
     Unit Hyd. peak  (cms)=       0.29         0.06
                                                      *TOTALS*
     PEAK FLOW       (cms)=       0.06         0.02          0.067 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.25           3.00
     RUNOFF VOLUME    (mm)=      37.75         5.65          13.03
     TOTAL RAINFALL   (mm)=      38.75        38.75          38.75
     RUNOFF COEFFICIENT   =       0.97         0.15           0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| RESERVOIR (0019) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE    |  OUTFLOW    STORAGE
-------------------      (cms)     (ha.m.)    |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                          0.0001     0.0074    |   0.0165      0.0238
                          0.0045     0.0089    |   0.0180      0.0280
                          0.0084     0.0111    |   0.0193      0.0326
                          0.0110     0.0137    |   0.0206      0.0376
                          0.0131     0.0166    |   0.0218      0.0435
                          0.0149     0.0200    |   0.0000      0.0000

                               AREA     QPEAK     TPEAK       R.V.
                               (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0018)      1.480      0.067      3.00      13.03
     OUTFLOW: ID= 1 (0019)      1.480      0.010      3.83       8.00

                    PEAK   FLOW   REDUCTION [Qout/Qin](%)= 14.63
                    TIME SHIFT OF PEAK FLOW         (min)= 50.00
                    MAXIMUM  STORAGE   USED       (ha.m.)=  0.0125

--------------------------------------------------------------------------------

-------------------
| ADD HYD   (0020) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0017):     0.59   0.014     3.08     6.76
        + ID2= 2 (0019):     1.48   0.010     3.83     8.00
          ================================================
          ID = 3 (0020):     2.07   0.020     3.08     7.65

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0023)  |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.037 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=   7.323
     TOTAL RAINFALL   (mm)=  38.750
     RUNOFF COEFFICIENT   =   0.189

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| STANDHYD  (0021)  |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min  |   Total Imp(%)=  25.00   Dir. Conn.(%)=  25.00
-------------------
                          IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.32         0.96
     Dep. Storage     (mm)=       1.00         5.80
     Average Slope     (%)=       1.00         6.60
     Length            (m)=      92.38       100.00
     Mannings n            =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      60.40         6.94
               over (min)        5.00        30.00
     Storage Coeff.  (min)=       2.98 (ii)   27.82 (ii)
     Unit Hyd. Tpeak (min)=       5.00        30.00
     Unit Hyd. peak  (cms)=       0.28         0.04
                                                      *TOTALS*
     PEAK FLOW       (cms)=       0.05         0.01          0.057 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.42           3.00
     RUNOFF VOLUME    (mm)=      37.75         5.86          13.82
     TOTAL RAINFALL   (mm)=      38.75        38.75          38.75
     RUNOFF COEFFICIENT   =       0.97         0.15           0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| RESERVOIR (0022) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE    |  OUTFLOW    STORAGE
-------------------      (cms)     (ha.m.)    |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                          0.0001     0.0052    |   0.0843      0.0172
                          0.0056     0.0066    |   0.0945      0.0200
                          0.0202     0.0082    |   0.1036      0.0231
                          0.0402     0.0101    |   0.1121      0.0265
                          0.0595     0.0122    |   0.1199      0.0301
                          0.0728     0.0146    |   0.0000      0.0000

                               AREA     QPEAK     TPEAK       R.V.
                               (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0021)      1.280      0.057      3.00      13.82
     OUTFLOW: ID= 1 (0022)      1.280      0.019      3.08       9.73

                    PEAK   FLOW   REDUCTION [Qout/Qin](%)= 33.86
                    TIME SHIFT OF PEAK FLOW         (min)=  5.00
                    MAXIMUM  STORAGE   USED       (ha.m.)=  0.0081

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-------------------
| ADD HYD   (0024) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0022):     1.28   0.019     3.08     9.73
        + ID2= 2 (0023):     1.40   0.037     3.08     7.32
          ================================================
          ID = 3 (0024):     2.68   0.056     3.08     8.47

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0055)  |   Area    (ha)=   1.24   Curve Number   (CN)= 67.5
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   8.20   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.26

     Unit Hyd Qpeak  (cms)=   0.182

     PEAK FLOW       (cms)=   0.020 (i)
     TIME TO PEAK    (hrs)=   3.167
     RUNOFF VOLUME    (mm)=   6.101
     TOTAL RAINFALL   (mm)=  38.750
     RUNOFF COEFFICIENT   =   0.157

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0056)  |   Area    (ha)=   0.58   Curve Number   (CN)= 66.1
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.26

     Unit Hyd Qpeak  (cms)=   0.085

     PEAK FLOW       (cms)=   0.011 (i)
     TIME TO PEAK    (hrs)=   3.167
     RUNOFF VOLUME    (mm)=   7.087
     TOTAL RAINFALL   (mm)=  38.750
     RUNOFF COEFFICIENT   =   0.183

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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  ****************************
  ** SIMULATION NUMBER:   2 **
  ****************************
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-------------------
|    READ STORM    |    Filename: C:\Users\gbergsma\AppD
|                  |              ata\Local\Temp\
|                  |              2e9de3a4-1259-4d17-ae23-36803a62f8c1\fcd4f25c
| Ptotal= 52.45 mm |    Comments: 5-Year, 6 hour SCS Type II - Peterboroug
-------------------
```

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	2.10	1.75	5.20	3.25	11.50	4.75	3.20
0.50	2.10	2.00	5.20	3.50	11.50	5.00	3.20
0.75	3.20	2.25	6.30	3.75	5.20	5.25	2.10
1.00	3.20	2.50	6.30	4.00	5.20	5.50	2.10
1.25	3.20	2.75	31.40	4.25	4.20	5.75	2.10
1.50	3.20	3.00	81.80	4.50	4.20	6.00	2.10

```
-------------------
| CALIB             |
| NASHYD    (0002) |   Area    (ha)=   1.42   Curve Number   (CN)= 63.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   9.10   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.22

        NOTE:  RAINFALL WAS TRANSFORMED TO   5.0 MIN. TIME STEP.
```

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.10	1.583	5.20	3.083	11.50	4.58	3.20
0.167	2.10	1.667	5.20	3.167	11.50	4.67	3.20
0.250	2.10	1.750	5.20	3.250	11.50	4.75	3.20
0.333	2.10	1.833	5.20	3.333	11.50	4.83	3.20
0.417	2.10	1.917	5.20	3.417	11.50	4.92	3.20
0.500	2.10	2.000	5.20	3.500	11.50	5.00	3.20
0.583	3.20	2.083	6.30	3.583	5.20	5.08	2.10
0.667	3.20	2.167	6.30	3.667	5.20	5.17	2.10
0.750	3.20	2.250	6.30	3.750	5.20	5.25	2.10
0.833	3.20	2.333	6.30	3.833	5.20	5.33	2.10
0.917	3.20	2.417	6.30	3.917	5.20	5.42	2.10
1.000	3.20	2.500	6.30	4.000	5.20	5.50	2.10
1.083	3.20	2.583	31.40	4.083	4.20	5.58	2.10
1.167	3.20	2.667	31.40	4.167	4.20	5.67	2.10
1.250	3.20	2.750	31.40	4.250	4.20	5.75	2.10
1.333	3.20	2.833	81.80	4.333	4.20	5.83	2.10
1.417	3.20	2.917	81.80	4.417	4.20	5.92	2.10
1.500	3.20	3.000	81.80	4.500	4.20	6.00	2.10

```
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                          0.0001     0.0072    |   0.1306      0.0171
                          0.0065     0.0083    |   0.1492      0.0194
                          0.0242     0.0096    |   0.1656      0.0220
                          0.0501     0.0112    |   0.1806      0.0248
                          0.0806     0.0129    |   0.1944      0.0278
                          0.1098     0.0149    |   0.0000      0.0000

                               AREA     QPEAK     TPEAK       R.V.
                               (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0015)     2.660      0.117     3.08       12.19
     OUTFLOW: ID= 1 (0016)     2.660      0.068     3.25        9.47

                  PEAK   FLOW   REDUCTION [Qout/Qin](%)= 58.45
                  TIME SHIFT OF PEAK FLOW         (min)= 10.00
                  MAXIMUM  STORAGE   USED       (ha.m.)=  0.0123

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0004) |   Area    (ha)=   2.44   Curve Number   (CN)= 68.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.548

     PEAK FLOW       (cms)=   0.110 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  12.333
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.235

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0003) |   Area    (ha)=   1.53   Curve Number   (CN)= 61.7
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.70   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.344

     PEAK FLOW       (cms)=   0.052 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=   9.469
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.181

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

     Unit Hyd Qpeak  (cms)=   0.247

     PEAK FLOW       (cms)=   0.042 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=   9.780
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.186

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0001) |   Area    (ha)=   2.02   Curve Number   (CN)= 61.5
|ID= 1 DT= 5.0 min |   Ia      (mm)=   5.30   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.454

     PEAK FLOW       (cms)=   0.078 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  10.745
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.205

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0015) |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.598

     PEAK FLOW       (cms)=   0.117 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  12.188
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.232

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| RESERVOIR (0016) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE   |  OUTFLOW    STORAGE
-------------------      (cms)     (ha.m.)    |   (cms)     (ha.m.)

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0007) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.026 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  12.312
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.235

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| STANDHYD  (0008) |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
-------------------
```

		IMPERVIOUS	PERVIOUS (i)	
Surface Area	(ha)=	0.34	1.14	
Dep. Storage	(mm)=	1.00	5.30	
Average Slope	(%)=	2.00	5.00	
Length	(m)=	99.33	40.00	
Mannings n	=	0.013	0.250	
Max.Eff.Inten.(mm/hr)=		81.80	18.50	
over (min)		5.00	15.00	
Storage Coeff. (min)=		2.24 (ii)	12.77 (ii)	
Unit Hyd. Tpeak (min)=		5.00	15.00	
Unit Hyd. peak (cms)=		0.30	0.08	
				TOTALS
PEAK FLOW	(cms)=	0.08	0.04	0.107 (iii)
TIME TO PEAK	(hrs)=	3.00	3.08	3.00
RUNOFF VOLUME	(mm)=	51.45	10.51	19.92
TOTAL RAINFALL	(mm)=	52.45	52.45	52.45
RUNOFF COEFFICIENT	=	0.98	0.20	0.38

```
***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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--------------------
| ADD HYD   (0009) |
|   1 +  2 =  3    |          AREA    QPEAK    TPEAK     R.V.
--------------------          (ha)    (cms)    (hrs)     (mm)
         ID1= 1 (0007):     0.59   0.026     3.08    12.31
       + ID2= 2 (0008):     1.48   0.107     3.00    19.92
         ================================================
         ID = 3 (0009):     2.07   0.133     3.00    17.75

    NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0011) |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.067 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  13.234
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.252

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| STANDHYD  (0010) |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min |   Total Imp(%)=  24.60   Dir. Conn.(%)=  24.60
--------------------

                           IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.31         0.97
     Dep. Storage     (mm)=       1.00         5.80
     Average Slope     (%)=       1.00         6.60
     Length            (m)=      92.38       100.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      81.80        15.49
               over (min)        5.00        25.00
     Storage Coeff.  (min)=       2.64 (ii)   20.66 (ii)
     Unit Hyd. Tpeak (min)=       5.00        25.00
     Unit Hyd. peak  (cms)=       0.29         0.05
                                                        *TOTALS*

     Unit Hyd Qpeak  (cms)=   0.342

     PEAK FLOW       (cms)=   0.067 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  12.293
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.234

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0006) |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.598

     PEAK FLOW       (cms)=   0.117 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  12.188
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.232

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0013) |   Area    (ha)=   0.75   Curve Number   (CN)= 68.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   7.40   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.169

     PEAK FLOW       (cms)=   0.034 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  12.328
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.235

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0017) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.026 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  12.312
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.235

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| STANDHYD  (0018) |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
--------------------

                           IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.34         1.14
     Dep. Storage     (mm)=       1.00         5.30
     Average Slope     (%)=       2.00         5.00
     Length            (m)=      99.33        40.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      81.80        18.50
               over (min)        5.00        15.00
     Storage Coeff.  (min)=       2.24 (ii)   12.77 (ii)
     Unit Hyd. Tpeak (min)=       5.00        15.00
     Unit Hyd. peak  (cms)=       0.30         0.08
                                                        *TOTALS*
     PEAK FLOW       (cms)=       0.08         0.04          0.107 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.08          3.00
     RUNOFF VOLUME    (mm)=      51.45        10.51         19.92
     TOTAL RAINFALL   (mm)=      52.45        52.45         52.45
     RUNOFF COEFFICIENT   =       0.98         0.20          0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| RESERVOIR (0019) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE   |  OUTFLOW    STORAGE
```

```
     PEAK FLOW       (cms)=       0.07         0.02          0.083 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.33           3.00
     RUNOFF VOLUME    (mm)=      51.45        10.93          20.89
     TOTAL RAINFALL   (mm)=      52.45        52.45          52.45
     RUNOFF COEFFICIENT   =       0.98         0.21           0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| ADD HYD   (0012) |
|   1 +  2 =  3    |          AREA    QPEAK    TPEAK     R.V.
--------------------          (ha)    (cms)    (hrs)     (mm)
         ID1= 1 (0010):     1.28   0.083     3.00    20.89
       + ID2= 2 (0011):     1.40   0.067     3.08    13.23
         ================================================
         ID = 3 (0012):     2.68   0.149     3.00    16.89

    NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0005) |   Area    (ha)=   1.73   Curve Number   (CN)= 68.6
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.389

     PEAK FLOW       (cms)=   0.077 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  12.250
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.234

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0014) |   Area    (ha)=   1.52   Curve Number   (CN)= 65.4
|ID= 1 DT= 5.0 min |   Ia      (mm)=   5.10   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17
```

```
--------------------      (cms)     (ha.m.)   |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                           0.0001     0.0074   |   0.0165      0.0238
                           0.0045     0.0089   |   0.0180      0.0280
                           0.0084     0.0111   |   0.0193      0.0326
                           0.0110     0.0137   |   0.0206      0.0376
                           0.0131     0.0166   |   0.0218      0.0435
                           0.0149     0.0200   |   0.0000      0.0000

                                AREA     QPEAK     TPEAK       R.V.
                                (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0018)      1.480      0.107      3.00      19.92
     OUTFLOW: ID= 1 (0019)      1.480      0.014      3.83      14.89

                    PEAK   FLOW   REDUCTION [Qout/Qin](%)= 13.40
                    TIME SHIFT OF PEAK FLOW         (min)= 50.00
                    MAXIMUM  STORAGE   USED       (ha.m.)=  0.0190

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-------------------
| ADD HYD   (0020) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
--------------------         (ha)    (cms)    (hrs)     (mm)
         ID1= 1 (0017):     0.59   0.026     3.08    12.31
       + ID2= 2 (0019):     1.48   0.014     3.83    14.89
         ================================================
         ID = 3 (0020):     2.07   0.038     3.08    14.15

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0023) |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.067 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  13.234
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.252

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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| CALIB             |
| STANDHYD  (0021) |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min |   Total Imp(%)=  25.00   Dir. Conn.(%)=  25.00
-------------------

                                IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.32         0.96
     Dep. Storage     (mm)=       1.00         5.80
     Average Slope     (%)=       1.00         6.60
     Length            (m)=      92.38       100.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      81.80        15.49
               over (min)        5.00        25.00
     Storage Coeff.  (min)=       2.64 (ii)   20.66 (ii)
     Unit Hyd. Tpeak (min)=       5.00        25.00
     Unit Hyd. peak  (cms)=       0.29         0.05
                                                          *TOTALS*
     PEAK FLOW       (cms)=       0.07         0.02          0.084 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.33           3.00
     RUNOFF VOLUME    (mm)=      51.45        10.93          21.05
     TOTAL RAINFALL   (mm)=      52.45        52.45          52.45
     RUNOFF COEFFICIENT   =       0.98         0.21           0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| RESERVOIR (0022) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE   |  OUTFLOW    STORAGE
-------------------      (cms)     (ha.m.)   |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                           0.0001     0.0052   |   0.0843      0.0172
                           0.0056     0.0066   |   0.0945      0.0200
                           0.0202     0.0082   |   0.1036      0.0231
                           0.0402     0.0101   |   0.1121      0.0265
                           0.0595     0.0122   |   0.1199      0.0301
                           0.0728     0.0146   |   0.0000      0.0000

                                AREA     QPEAK     TPEAK       R.V.
                                (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0021)      1.280      0.084      3.00      21.05
     OUTFLOW: ID= 1 (0022)      1.280      0.043      3.08      16.96
```

```
                    PEAK   FLOW   REDUCTION [Qout/Qin](%)= 50.74
                    TIME SHIFT OF PEAK FLOW         (min)=  5.00
                    MAXIMUM  STORAGE   USED       (ha.m.)=  0.0105

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-------------------
| ADD HYD   (0024) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
--------------------         (ha)    (cms)    (hrs)     (mm)
         ID1= 1 (0022):     1.28   0.043     3.08    16.96
       + ID2= 2 (0023):     1.40   0.067     3.08    13.23
         ================================================
         ID = 3 (0024):     2.68   0.110     3.08    15.01

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-------------------
| CALIB             |
| NASHYD    (0055) |   Area    (ha)=   1.24   Curve Number   (CN)= 67.5
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.20   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.26

     Unit Hyd Qpeak  (cms)=   0.182

     PEAK FLOW       (cms)=   0.040 (i)
     TIME TO PEAK    (hrs)=   3.167
     RUNOFF VOLUME    (mm)=  11.748
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.224

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| CALIB             |
| NASHYD    (0056) |   Area    (ha)=   0.58   Curve Number   (CN)= 66.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.26

     Unit Hyd Qpeak  (cms)=   0.085

     PEAK FLOW       (cms)=   0.021 (i)
     TIME TO PEAK    (hrs)=   3.167
     RUNOFF VOLUME    (mm)=  12.845
     TOTAL RAINFALL   (mm)=  52.450
     RUNOFF COEFFICIENT   =   0.245

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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  ***************************
  ** SIMULATION NUMBER:   3 **
  ***************************


-------------------
|    READ STORM    |    Filename: C:\Users\gbergsma\AppD
|                  |              ata\Local\Temp\
|                  |              2e9de3a4-1259-4d17-ae23-36803a62f8c1\b881660d
| Ptotal= 61.60 mm |   Comments: 10-Year, 6 hour SCS Type II - Peterborou
-------------------

                TIME    RAIN |  TIME    RAIN |'  TIME    RAIN |  TIME    RAIN
                 hrs   mm/hr |   hrs   mm/hr |'   hrs   mm/hr |   hrs   mm/hr
                0.25    2.50 |  1.75    6.20 |'  3.25   13.50 |  4.75    3.70
                0.50    2.50 |  2.00    6.20 |'  3.50   13.50 |  5.00    3.70
                0.75    3.70 |  2.25    7.40 |'  3.75    6.20 |  5.25    2.50
                1.00    3.70 |  2.50    7.40 |'  4.00    6.20 |  5.50    2.50
                1.25    3.70 |  2.75   36.90 |'  4.25    4.90 |  5.75    2.50
                1.50    3.70 |  3.00   95.90 |'  4.50    4.90 |  6.00    2.50

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-------------------
| CALIB             |
| NASHYD    (0002) |   Area    (ha)=   1.42   Curve Number   (CN)= 63.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   9.10   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.22

        NOTE:  RAINFALL WAS TRANSFORMED TO   5.0 MIN. TIME STEP.

                          ---- TRANSFORMED HYETOGRAPH ----
                TIME    RAIN |  TIME    RAIN |'  TIME    RAIN |  TIME    RAIN
                 hrs   mm/hr |   hrs   mm/hr |'   hrs   mm/hr |   hrs   mm/hr
               0.083    2.50 | 1.583    6.20 |' 3.083   13.50 |  4.58    3.70
               0.167    2.50 | 1.667    6.20 |' 3.167   13.50 |  4.67    3.70
               0.250    2.50 | 1.750    6.20 |' 3.250   13.50 |  4.75    3.70
               0.333    2.50 | 1.833    6.20 |' 3.333   13.50 |  4.83    3.70
               0.417    2.50 | 1.917    6.20 |' 3.417   13.50 |  4.92    3.70
               0.500    2.50 | 2.000    6.20 |' 3.500   13.50 |  5.00    3.70
               0.583    3.70 | 2.083    7.40 |' 3.583    6.20 |  5.08    2.50
               0.667    3.70 | 2.167    7.40 |' 3.667    6.20 |  5.17    2.50
               0.750    3.70 | 2.250    7.40 |' 3.750    6.20 |  5.25    2.50
               0.833    3.70 | 2.333    7.40 |' 3.833    6.20 |  5.33    2.50
               0.917    3.70 | 2.417    7.40 |' 3.917    6.20 |  5.42    2.50
               1.000    3.70 | 2.500    7.40 |' 4.000    6.20 |  5.50    2.50
               1.083    3.70 | 2.583   36.90 |' 4.083    4.90 |  5.58    2.50
               1.167    3.70 | 2.667   36.90 |' 4.167    4.90 |  5.67    2.50
```

```
                1.250    3.70 | 2.750   36.90 | 4.250    4.90 |  5.75    2.50
                1.333    3.70 | 2.833   95.90 | 4.333    4.90 |  5.83    2.50
                1.417    3.70 | 2.917   95.90 | 4.417    4.90 |  5.92    2.50
                1.500    3.70 | 3.000   95.90 | 4.500    4.90 |  6.00    2.50

     Unit Hyd Qpeak  (cms)=   0.247

     PEAK FLOW       (cms)=   0.060 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  13.692
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.222

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0001)  |   Area    (ha)=   2.02   Curve Number   (CN)= 61.5
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   5.30   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.454

     PEAK FLOW       (cms)=   0.107 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  14.669
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.238

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0015)  |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.598

     PEAK FLOW       (cms)=   0.159 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  16.505
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.268

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| RESERVOIR (0016) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE    |  OUTFLOW    STORAGE
-------------------      (cms)     (ha.m.)     |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                            0.0001     0.0072    |  0.1306      0.0171
                            0.0065     0.0083    |  0.1492      0.0194
                            0.0242     0.0096    |  0.1656      0.0220
                            0.0501     0.0112    |  0.1806      0.0248
                            0.0806     0.0129    |  0.1944      0.0278
                            0.1098     0.0149    |  0.0000      0.0000

                                AREA     QPEAK     TPEAK       R.V.
                                (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0015)      2.660      0.159      3.08      16.50
     OUTFLOW: ID= 1 (0016)      2.660      0.104      3.25      13.79

                    PEAK   FLOW   REDUCTION [Qout/Qin](%)= 65.62
                    TIME SHIFT OF PEAK FLOW         (min)= 10.00
                    MAXIMUM  STORAGE   USED       (ha.m.)=  0.0146

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0004)  |   Area    (ha)=   2.44   Curve Number   (CN)= 68.8
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.548

     PEAK FLOW       (cms)=   0.151 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  16.960
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.275

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0003)  |   Area    (ha)=   1.53   Curve Number   (CN)= 61.7
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   8.70   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.344

     PEAK FLOW       (cms)=   0.073 (i)
     TIME TO PEAK    (hrs)=   3.083
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```
     RUNOFF VOLUME    (mm)=  13.242
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.215

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0007)  |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.035 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  16.657
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.270

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| STANDHYD  (0008)  |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min  |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
-------------------

                            IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.34         1.14
     Dep. Storage     (mm)=       1.00         5.30
     Average Slope     (%)=       2.00         5.00
     Length            (m)=      99.33        40.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      95.90        28.35
               over (min)        5.00        15.00
     Storage Coeff.  (min)=       2.10 (ii)   10.98 (ii)
     Unit Hyd. Tpeak (min)=       5.00        15.00
     Unit Hyd. peak  (cms)=       0.31         0.09
                                                      *TOTALS*
     PEAK FLOW       (cms)=       0.09         0.05          0.136 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.08           3.00
     RUNOFF VOLUME    (mm)=      60.60        14.36          24.99
     TOTAL RAINFALL   (mm)=      61.60        61.60          61.60
     RUNOFF COEFFICIENT   =       0.98         0.23           0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
```

```
       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| ADD HYD   (0009) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0007):     0.59   0.035     3.08    16.66
        + ID2= 2 (0008):     1.48   0.136     3.00    24.99
          ==========================================
          ID = 3 (0009):     2.07   0.171     3.00    22.62

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0011)  |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.090 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  17.833
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.289

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| STANDHYD  (0010)  |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min  |   Total Imp(%)=  24.60   Dir. Conn.(%)=  24.60
-------------------

                            IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.31         0.97
     Dep. Storage     (mm)=       1.00         5.80
     Average Slope     (%)=       1.00         6.60
     Length            (m)=      92.38       100.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      95.90        26.51
```

```
               over (min)        5.00        20.00
     Storage Coeff.  (min)=       2.48 (ii)   17.02 (ii)
     Unit Hyd. Tpeak (min)=       5.00        20.00
     Unit Hyd. peak  (cms)=       0.29         0.06
                                                    *TOTALS*
     PEAK FLOW       (cms)=       0.08         0.04          0.108 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.17           3.00
     RUNOFF VOLUME    (mm)=      60.60        14.96          26.18
     TOTAL RAINFALL   (mm)=      61.60        61.60          61.60
     RUNOFF COEFFICIENT   =       0.98         0.24           0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| ADD HYD   (0012) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
--------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0010):     1.28   0.108     3.00    26.18
        + ID2= 2 (0011):     1.40   0.090     3.08    17.83
          ================================================
          ID = 3 (0012):     2.68   0.196     3.00    21.82

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0005) |   Area    (ha)=   1.73   Curve Number   (CN)= 68.6
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.389

     PEAK FLOW       (cms)=   0.107 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  16.853
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.274

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0014) |   Area    (ha)=   1.52   Curve Number   (CN)= 65.4
|ID= 1 DT= 5.0 min |   Ia      (mm)=   5.10   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.342

     PEAK FLOW       (cms)=   0.092 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  16.664
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.271

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0006) |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.598

     PEAK FLOW       (cms)=   0.159 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  16.505
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.268

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0013) |   Area    (ha)=   0.75   Curve Number   (CN)= 68.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   7.40   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.169

     PEAK FLOW       (cms)=   0.046 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  16.902
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.274

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0017) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.035 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  16.657
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.270

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| STANDHYD  (0018) |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
--------------------

                           IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.34         1.14
     Dep. Storage     (mm)=       1.00         5.30
     Average Slope     (%)=       2.00         5.00
     Length            (m)=      99.33        40.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      95.90        28.35
               over (min)        5.00        15.00
     Storage Coeff.  (min)=       2.10 (ii)   10.98 (ii)
     Unit Hyd. Tpeak (min)=       5.00        15.00
     Unit Hyd. peak  (cms)=       0.31         0.09
                                                    *TOTALS*
     PEAK FLOW       (cms)=       0.09         0.05          0.136 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.08           3.00
     RUNOFF VOLUME    (mm)=      60.60        14.36          24.99
     TOTAL RAINFALL   (mm)=      61.60        61.60          61.60
     RUNOFF COEFFICIENT   =       0.98         0.23           0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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| RESERVOIR (0019) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE   |  OUTFLOW    STORAGE
--------------------      (cms)     (ha.m.)   |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                           0.0001     0.0074   |   0.0165      0.0238
                           0.0045     0.0089   |   0.0180      0.0280
                           0.0084     0.0111   |   0.0193      0.0326
                           0.0110     0.0137   |   0.0206      0.0376
                           0.0131     0.0166   |   0.0218      0.0435
                           0.0149     0.0200   |   0.0000      0.0000

                                AREA     QPEAK     TPEAK       R.V.
                                (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0018)      1.480      0.136      3.00      24.99
     OUTFLOW: ID= 1 (0019)      1.480      0.017      3.83      19.96

                  PEAK   FLOW   REDUCTION [Qout/Qin](%)= 12.25
                  TIME SHIFT OF PEAK FLOW         (min)= 50.00
                  MAXIMUM  STORAGE   USED       (ha.m.)=  0.0241

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| ADD HYD   (0020) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
--------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0017):     0.59   0.035     3.08    16.66
        + ID2= 2 (0019):     1.48   0.017     3.83    19.96
          ================================================
          ID = 3 (0020):     2.07   0.049     3.08    19.02

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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| CALIB            |
| NASHYD    (0023) |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.090 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  17.833
     TOTAL RAINFALL   (mm)=  61.600
     RUNOFF COEFFICIENT   =   0.289
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB					
STANDHYD (0021)	Area (ha)= 1.28				
ID= 1 DT= 5.0 min	Total Imp(%)= 25.00	Dir. Conn.(%)= 25.00			

		IMPERVIOUS	PERVIOUS (i)	
Surface Area	(ha)=	0.32	0.96	
Dep. Storage	(mm)=	1.00	5.80	
Average Slope	(%)=	1.00	6.60	
Length	(m)=	92.38	100.00	
Mannings n	=	0.013	0.250	
Max.Eff.Inten.(mm/hr)=		95.90	26.51	
over (min)		5.00	20.00	
Storage Coeff. (min)=		2.48 (ii)	17.02 (ii)	
Unit Hyd. Tpeak (min)=		5.00	20.00	
Unit Hyd. peak (cms)=		0.29	0.06	
				TOTALS
PEAK FLOW	(cms)=	0.09	0.04	0.109 (iii)
TIME TO PEAK	(hrs)=	3.00	3.17	3.00
RUNOFF VOLUME	(mm)=	60.60	14.96	26.36
TOTAL RAINFALL	(mm)=	61.60	61.60	61.60
RUNOFF COEFFICIENT	=	0.98	0.24	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 62.5 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0022)				
IN= 2---> OUT= 1				
DT= 5.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
**** WARNING : FIRST OUTFLOW IS NOT ZERO.				
	0.0001	0.0052	0.0843	0.0172
	0.0056	0.0066	0.0945	0.0200
	0.0202	0.0082	0.1036	0.0231
	0.0402	0.0101	0.1121	0.0265
	0.0595	0.0122	0.1199	0.0301
	0.0728	0.0146	0.0000	0.0000

		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0021)		1.280	0.109	3.00	26.36
OUTFLOW: ID= 1 (0022)		1.280	0.059	3.08	22.27
PEAK FLOW REDUCTION [Qout/Qin](%)= 54.24					
TIME SHIFT OF PEAK FLOW			(min)= 5.00		
MAXIMUM STORAGE USED			(ha.m.)= 0.0122		

ADD HYD (0024)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0022):	1.28	0.059	3.08	22.27
+ ID2= 2 (0023):	1.40	0.090	3.08	17.83
ID = 3 (0024):	2.68	0.149	3.08	19.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB			
NASHYD (0055)	Area (ha)= 1.24	Curve Number (CN)= 67.5	
ID= 1 DT= 5.0 min	Ia (mm)= 8.20	# of Linear Res.(N)= 3.00	
	U.H. Tp(hrs)= 0.26		

Unit Hyd Qpeak (cms)= 0.182

PEAK FLOW (cms)= 0.056 (i)
TIME TO PEAK (hrs)= 3.167
RUNOFF VOLUME (mm)= 16.219
TOTAL RAINFALL (mm)= 61.600
RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
NASHYD (0056)	Area (ha)= 0.58	Curve Number (CN)= 66.1	
ID= 1 DT= 5.0 min	Ia (mm)= 4.60	# of Linear Res.(N)= 3.00	
	U.H. Tp(hrs)= 0.26		

Unit Hyd Qpeak (cms)= 0.085

PEAK FLOW (cms)= 0.028 (i)
TIME TO PEAK (hrs)= 3.167

RUNOFF VOLUME (mm)= 17.337
TOTAL RAINFALL (mm)= 61.600
RUNOFF COEFFICIENT = 0.281

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

****
** SIMULATION NUMBER: 4 **
****

READ STORM	Filename: C:\Users\gbergsma\AppData\Local\Temp\2e9de3a4-1259-4d17-ae23-36803a62f8c1\c775860c
Ptotal= 72.90 mm	Comments: 25-Year, 6 hour SCS Type II - Peterborou

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	2.90	1.75	7.30	3.25	16.00	4.75	4.40
0.50	2.90	2.00	7.30	3.50	16.00	5.00	4.40
0.75	4.40	2.25	8.80	3.75	7.30	5.25	2.90
1.00	4.40	2.50	8.80	4.00	7.30	5.50	2.90
1.25	4.40	2.75	43.70	4.25	5.80	5.75	2.90
1.50	4.40	3.00	113.70	4.50	5.80	6.00	2.90

CALIB			
NASHYD (0002)	Area (ha)= 1.42	Curve Number (CN)= 63.1	
ID= 1 DT= 5.0 min	Ia (mm)= 9.10	# of Linear Res.(N)= 3.00	
	U.H. Tp(hrs)= 0.22		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	2.90	1.583	7.30	3.083	16.00	4.58	4.40
0.167	2.90	1.667	7.30	3.167	16.00	4.67	4.40
0.250	2.90	1.750	7.30	3.250	16.00	4.75	4.40
0.333	2.90	1.833	7.30	3.333	16.00	4.83	4.40
0.417	2.90	1.917	7.30	3.417	16.00	4.92	4.40
0.500	2.90	2.000	7.30	3.500	16.00	5.00	4.40
0.583	4.40	2.083	8.80	3.583	7.30	5.08	2.90
0.667	4.40	2.167	8.80	3.667	7.30	5.17	2.90
0.750	4.40	2.250	8.80	3.750	7.30	5.25	2.90
0.833	4.40	2.333	8.80	3.833	7.30	5.33	2.90
0.917	4.40	2.417	8.80	3.917	7.30	5.42	2.90
1.000	4.40	2.500	8.80	4.000	7.30	5.50	2.90
1.083	4.40	2.583	43.70	4.083	5.80	5.58	2.90
1.167	4.40	2.667	43.70	4.167	5.80	5.67	2.90
1.250	4.40	2.750	43.70	4.250	5.80	5.75	2.90
1.333	4.40	2.833	113.70	4.333	5.80	5.83	2.90
1.417	4.40	2.917	113.70	4.417	5.80	5.92	2.90
1.500	4.40	3.000	113.70	4.500	5.80	6.00	2.90

Unit Hyd Qpeak (cms)= 0.247

PEAK FLOW (cms)= 0.086 (i)
TIME TO PEAK (hrs)= 3.083
RUNOFF VOLUME (mm)= 19.144
TOTAL RAINFALL (mm)= 72.900
RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
NASHYD (0001)	Area (ha)= 2.02	Curve Number (CN)= 61.5	
ID= 1 DT= 5.0 min	Ia (mm)= 5.30	# of Linear Res.(N)= 3.00	
	U.H. Tp(hrs)= 0.17		

Unit Hyd Qpeak (cms)= 0.454

PEAK FLOW (cms)= 0.147 (i)
TIME TO PEAK (hrs)= 3.083
RUNOFF VOLUME (mm)= 20.094
TOTAL RAINFALL (mm)= 72.900
RUNOFF COEFFICIENT = 0.276

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
NASHYD (0015)	Area (ha)= 2.66	Curve Number (CN)= 64.8	
ID= 1 DT= 5.0 min	Ia (mm)= 4.80	# of Linear Res.(N)= 3.00	
	U.H. Tp(hrs)= 0.17		

Unit Hyd Qpeak (cms)= 0.598

PEAK FLOW (cms)= 0.216 (i)
TIME TO PEAK (hrs)= 3.083
RUNOFF VOLUME (mm)= 22.424
TOTAL RAINFALL (mm)= 72.900

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     RUNOFF COEFFICIENT   =   0.308

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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| RESERVOIR (0016) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE   |  OUTFLOW    STORAGE
--------------------      (cms)     (ha.m.)   |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                           0.0001     0.0072   |   0.1306      0.0171
                           0.0065     0.0083   |   0.1492      0.0194
                           0.0242     0.0096   |   0.1656      0.0220
                           0.0501     0.0112   |   0.1806      0.0248
                           0.0806     0.0129   |   0.1944      0.0278
                           0.1098     0.0149   |   0.0000      0.0000

                                AREA     QPEAK     TPEAK       R.V.
                                (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0015)      2.660      0.216      3.08      22.42
     OUTFLOW: ID= 1 (0016)      2.660      0.140      3.25      19.71

                  PEAK   FLOW   REDUCTION [Qout/Qin](%)= 64.86
                  TIME SHIFT OF PEAK FLOW         (min)= 10.00
                  MAXIMUM  STORAGE   USED       (ha.m.)=  0.0185

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--------------------
| CALIB            |
| NASHYD    (0004) |   Area    (ha)=   2.44   Curve Number   (CN)= 68.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.548

     PEAK FLOW       (cms)=   0.210 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  23.305
     TOTAL RAINFALL   (mm)=  72.900
     RUNOFF COEFFICIENT   =   0.320

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0003) |   Area    (ha)=   1.53   Curve Number   (CN)= 61.7
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.70   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.344

     PEAK FLOW       (cms)=   0.103 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  18.510
     TOTAL RAINFALL   (mm)=  72.900
     RUNOFF COEFFICIENT   =   0.254

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0007) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.048 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  22.611
     TOTAL RAINFALL   (mm)=  72.900
     RUNOFF COEFFICIENT   =   0.310

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| STANDHYD  (0008) |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
--------------------

                           IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=      0.34         1.14
     Dep. Storage     (mm)=      1.00         5.30
     Average Slope     (%)=      2.00         5.00
     Length            (m)=     99.33        40.00
     Mannings n           =     0.013        0.250

     Max.Eff.Inten.(mm/hr)=    113.70        38.86
               over (min)        5.00        10.00
     Storage Coeff.  (min)=      1.96 (ii)    9.79 (ii)
     Unit Hyd. Tpeak (min)=      5.00        10.00
     Unit Hyd. peak  (cms)=      0.31         0.11
                                                          *TOTALS*
     PEAK FLOW       (cms)=      0.11         0.08          0.190 (iii)
     TIME TO PEAK    (hrs)=      3.00         3.08           3.00
     RUNOFF VOLUME    (mm)=     71.90        19.69          31.70

     TOTAL RAINFALL   (mm)=     72.90        72.90          72.90
     RUNOFF COEFFICIENT   =      0.99         0.27           0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| ADD HYD   (0009) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
--------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0007):     0.59   0.048     3.08    22.61
        + ID2= 2 (0008):     1.48   0.190     3.00    31.70
          ================================================
          ID = 3 (0009):     2.07   0.238     3.00    29.11

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0011) |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.123 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  24.104
     TOTAL RAINFALL   (mm)=  72.900
     RUNOFF COEFFICIENT   =   0.331

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| STANDHYD  (0010) |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min |   Total Imp(%)=  24.60   Dir. Conn.(%)=  24.60
--------------------

                           IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=      0.31         0.97
     Dep. Storage     (mm)=      1.00         5.80

     Average Slope     (%)=      1.00         6.60
     Length            (m)=     92.38       100.00
     Mannings n           =     0.013        0.250

     Max.Eff.Inten.(mm/hr)=    113.70        36.62
               over (min)        5.00        20.00
     Storage Coeff.  (min)=      2.31 (ii)   15.09 (ii)
     Unit Hyd. Tpeak (min)=      5.00        20.00
     Unit Hyd. peak  (cms)=      0.30         0.07
                                                          *TOTALS*
     PEAK FLOW       (cms)=      0.10         0.06          0.135 (iii)
     TIME TO PEAK    (hrs)=      3.00         3.17           3.00
     RUNOFF VOLUME    (mm)=     71.90        20.51          33.15
     TOTAL RAINFALL   (mm)=     72.90        72.90          72.90
     RUNOFF COEFFICIENT   =      0.99         0.28           0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| ADD HYD   (0012) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
--------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0010):     1.28   0.135     3.00    33.15
        + ID2= 2 (0011):     1.40   0.123     3.08    24.10
          ================================================
          ID = 3 (0012):     2.68   0.257     3.00    28.42

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0005) |   Area    (ha)=   1.73   Curve Number   (CN)= 68.6
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.389

     PEAK FLOW       (cms)=   0.148 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  23.167
     TOTAL RAINFALL   (mm)=  72.900
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     RUNOFF COEFFICIENT   =   0.318

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0014)  |   Area    (ha)=   1.52   Curve Number   (CN)= 65.4
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   5.10   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.342

     PEAK FLOW       (cms)=   0.125 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  22.655
     TOTAL RAINFALL   (mm)=  72.900
     RUNOFF COEFFICIENT   =   0.311

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0006)  |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.598

     PEAK FLOW       (cms)=   0.216 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  22.424
     TOTAL RAINFALL   (mm)=  72.900
     RUNOFF COEFFICIENT   =   0.308

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0013)  |   Area    (ha)=   0.75   Curve Number   (CN)= 68.1
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   7.40   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.169

     PEAK FLOW       (cms)=   0.064 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  23.172
     TOTAL RAINFALL   (mm)=  72.900
     RUNOFF COEFFICIENT   =   0.318

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0017)  |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.048 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  22.611
     TOTAL RAINFALL   (mm)=  72.900
     RUNOFF COEFFICIENT   =   0.310

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| CALIB             |
| STANDHYD  (0018)  |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min  |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
-------------------

                            IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.34         1.14
     Dep. Storage     (mm)=       1.00         5.30
     Average Slope     (%)=       2.00         5.00
     Length            (m)=      99.33        40.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=     113.70        38.86
               over (min)        5.00        10.00
     Storage Coeff.  (min)=       1.96 (ii)    9.79 (ii)
     Unit Hyd. Tpeak (min)=       5.00        10.00
     Unit Hyd. peak  (cms)=       0.31         0.11
                                                          *TOTALS*
     PEAK FLOW       (cms)=       0.11         0.08          0.190 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.08           3.00
     RUNOFF VOLUME    (mm)=      71.90        19.69          31.70
     TOTAL RAINFALL   (mm)=      72.90        72.90          72.90
     RUNOFF COEFFICIENT   =       0.99         0.27           0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-------------------------------------------------------------------------------
-------------------
| RESERVOIR (0019) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE    |  OUTFLOW    STORAGE
-------------------      (cms)     (ha.m.)    |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                         0.0001     0.0074    |   0.0165      0.0238
                         0.0045     0.0089    |   0.0180      0.0280
                         0.0084     0.0111    |   0.0193      0.0326
                         0.0110     0.0137    |   0.0206      0.0376
                         0.0131     0.0166    |   0.0218      0.0435
                         0.0149     0.0200    |   0.0000      0.0000

                              AREA     QPEAK     TPEAK       R.V.
                              (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0018)    1.480      0.190      3.00      31.70
     OUTFLOW: ID= 1 (0019)    1.480      0.019      3.92      26.67

                PEAK   FLOW   REDUCTION [Qout/Qin](%)=  9.99
                TIME SHIFT OF PEAK FLOW         (min)= 55.00
                MAXIMUM  STORAGE   USED       (ha.m.)=  0.0313

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| ADD HYD   (0020) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------          (ha)    (cms)    (hrs)     (mm)
         ID1= 1 (0017):     0.59   0.048     3.08    22.61
       + ID2= 2 (0019):     1.48   0.019     3.92    26.67
         ================================================
         ID = 3 (0020):     2.07   0.065     3.08    25.51

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-------------------
| CALIB             |
| NASHYD    (0023)  |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.123 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  24.104
     TOTAL RAINFALL   (mm)=  72.900
     RUNOFF COEFFICIENT   =   0.331

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| CALIB             |
| STANDHYD  (0021)  |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min  |   Total Imp(%)=  25.00   Dir. Conn.(%)=  25.00
-------------------

                            IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.32         0.96
     Dep. Storage     (mm)=       1.00         5.80
     Average Slope     (%)=       1.00         6.60
     Length            (m)=      92.38       100.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=     113.70        36.62
               over (min)        5.00        20.00
     Storage Coeff.  (min)=       2.31 (ii)   15.09 (ii)
     Unit Hyd. Tpeak (min)=       5.00        20.00
     Unit Hyd. peak  (cms)=       0.30         0.07
                                                          *TOTALS*
     PEAK FLOW       (cms)=       0.10         0.06          0.137 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.17           3.00
     RUNOFF VOLUME    (mm)=      71.90        20.51          33.35
     TOTAL RAINFALL   (mm)=      72.90        72.90          72.90
     RUNOFF COEFFICIENT   =       0.99         0.28           0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| RESERVOIR (0022) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE    |  OUTFLOW    STORAGE
-------------------      (cms)     (ha.m.)    |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                         0.0001     0.0052    |   0.0843      0.0172
                         0.0056     0.0066    |   0.0945      0.0200
```

	0.0202	0.0082	0.1036	0.0231
	0.0402	0.0101	0.1121	0.0265
	0.0595	0.0122	0.1199	0.0301
	0.0728	0.0146	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0021)	1.280	0.137	3.00	33.35
OUTFLOW: ID= 1 (0022)	1.280	0.071	3.17	29.27

PEAK FLOW REDUCTION [Qout/Qin](%)= 52.21
TIME SHIFT OF PEAK FLOW (min)= 10.00
MAXIMUM STORAGE USED (ha.m.)= 0.0144

ADD HYD (0024)					
1 + 2 = 3		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	ID1= 1 (0022):	1.28	0.071	3.17	29.27
+	ID2= 2 (0023):	1.40	0.123	3.08	24.10
	ID = 3 (0024):	2.68	0.194	3.08	26.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB					
NASHYD (0055)	Area (ha)=	1.24	Curve Number (CN)=	67.5	
ID= 1 DT= 5.0 min	Ia (mm)=	8.20	# of Linear Res.(N)=	3.00	
	U.H. Tp(hrs)=	0.26			

Unit Hyd Qpeak (cms)= 0.182

PEAK FLOW (cms)= 0.078 (i)
TIME TO PEAK (hrs)= 3.167
RUNOFF VOLUME (mm)= 22.370
TOTAL RAINFALL (mm)= 72.900
RUNOFF COEFFICIENT = 0.307

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB					
NASHYD (0056)	Area (ha)=	0.58	Curve Number (CN)=	66.1	
ID= 1 DT= 5.0 min	Ia (mm)=	4.60	# of Linear Res.(N)=	3.00	
	U.H. Tp(hrs)=	0.26			

0.417	3.30	1.917	8.10	3.417	17.90	4.92	4.90
0.500	3.30	2.000	8.10	3.500	17.90	5.00	4.90
0.583	4.90	2.083	9.80	3.583	8.10	5.08	3.30
0.667	4.90	2.167	9.80	3.667	8.10	5.17	3.30
0.750	4.90	2.250	9.80	3.750	8.10	5.25	3.30
0.833	4.90	2.333	9.80	3.833	8.10	5.33	3.30
0.917	4.90	2.417	9.80	3.917	8.10	5.42	3.30
1.000	4.90	2.500	9.80	4.000	8.10	5.50	3.30
1.083	4.90	2.583	48.90	4.083	6.50	5.58	3.30
1.167	4.90	2.667	48.90	4.167	6.50	5.67	3.30
1.250	4.90	2.750	48.90	4.250	6.50	5.75	3.30
1.333	4.90	2.833	127.00	4.333	6.50	5.83	3.30
1.417	4.90	2.917	127.00	4.417	6.50	5.92	3.30
1.500	4.90	3.000	127.00	4.500	6.50	6.00	3.30

Unit Hyd Qpeak (cms)= 0.247

PEAK FLOW (cms)= 0.107 (i)
TIME TO PEAK (hrs)= 3.083
RUNOFF VOLUME (mm)= 23.680
TOTAL RAINFALL (mm)= 81.475
RUNOFF COEFFICIENT = 0.291

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB					
NASHYD (0001)	Area (ha)=	2.02	Curve Number (CN)=	61.5	
ID= 1 DT= 5.0 min	Ia (mm)=	5.30	# of Linear Res.(N)=	3.00	
	U.H. Tp(hrs)=	0.17			

Unit Hyd Qpeak (cms)= 0.454

PEAK FLOW (cms)= 0.180 (i)
TIME TO PEAK (hrs)= 3.083
RUNOFF VOLUME (mm)= 24.585
TOTAL RAINFALL (mm)= 81.475
RUNOFF COEFFICIENT = 0.302

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB					
NASHYD (0015)	Area (ha)=	2.66	Curve Number (CN)=	64.8	
ID= 1 DT= 5.0 min	Ia (mm)=	4.80	# of Linear Res.(N)=	3.00	
	U.H. Tp(hrs)=	0.17			

Unit Hyd Qpeak (cms)= 0.598

Unit Hyd Qpeak (cms)= 0.085

PEAK FLOW (cms)= 0.038 (i)
TIME TO PEAK (hrs)= 3.167
RUNOFF VOLUME (mm)= 23.476
TOTAL RAINFALL (mm)= 72.900
RUNOFF COEFFICIENT = 0.322

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

****
** SIMULATION NUMBER: 5 **
****

READ STORM	Filename: C:\Users\gbergsma\AppData\Local\Temp\2e9de3a4-1259-4d17-ae23-36803a62f8c1\4694a7c7
Ptotal= 81.47 mm	Comments: 50-Year, 6 hour SCS Type II - Peterborou

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	3.30	1.75	8.10	3.25	17.90	4.75	4.90
0.50	3.30	2.00	8.10	3.50	17.90	5.00	4.90
0.75	4.90	2.25	9.80	3.75	8.10	5.25	3.30
1.00	4.90	2.50	9.80	4.00	8.10	5.50	3.30
1.25	4.90	2.75	48.90	4.25	6.50	5.75	3.30
1.50	4.90	3.00	127.00	4.50	6.50	6.00	3.30

CALIB					
NASHYD (0002)	Area (ha)=	1.42	Curve Number (CN)=	63.1	
ID= 1 DT= 5.0 min	Ia (mm)=	9.10	# of Linear Res.(N)=	3.00	
	U.H. Tp(hrs)=	0.22			

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	3.30	1.583	8.10	3.083	17.90	4.58	4.90
0.167	3.30	1.667	8.10	3.167	17.90	4.67	4.90
0.250	3.30	1.750	8.10	3.250	17.90	4.75	4.90
0.333	3.30	1.833	8.10	3.333	17.90	4.83	4.90

PEAK FLOW (cms)= 0.264 (i)
TIME TO PEAK (hrs)= 3.083
RUNOFF VOLUME (mm)= 27.291
TOTAL RAINFALL (mm)= 81.475
RUNOFF COEFFICIENT = 0.335

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0016)				
IN= 2---> OUT= 1				
DT= 5.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0001	0.0072	0.1306	0.0171
	0.0065	0.0083	0.1492	0.0194
	0.0242	0.0096	0.1656	0.0220
	0.0501	0.0112	0.1806	0.0248
	0.0806	0.0129	0.1944	0.0278
	0.1098	0.0149	0.0000	0.0000

**** WARNING : FIRST OUTFLOW IS NOT ZERO.

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0015)	2.660	0.264	3.08	27.29
OUTFLOW: ID= 1 (0016)	2.660	0.165	3.25	24.58

PEAK FLOW REDUCTION [Qout/Qin](%)= 62.53
TIME SHIFT OF PEAK FLOW (min)= 10.00
MAXIMUM STORAGE USED (ha.m.)= 0.0220

CALIB					
NASHYD (0004)	Area (ha)=	2.44	Curve Number (CN)=	68.8	
ID= 1 DT= 5.0 min	Ia (mm)=	8.00	# of Linear Res.(N)=	3.00	
	U.H. Tp(hrs)=	0.17			

Unit Hyd Qpeak (cms)= 0.548

PEAK FLOW (cms)= 0.257 (i)
TIME TO PEAK (hrs)= 3.083
RUNOFF VOLUME (mm)= 28.513
TOTAL RAINFALL (mm)= 81.475
RUNOFF COEFFICIENT = 0.350

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0003) |   Area    (ha)=   1.53   Curve Number   (CN)= 61.7
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.70   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.344

     PEAK FLOW       (cms)=   0.128 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  22.900
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.281

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0007) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.059 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  27.504
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.338

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| STANDHYD  (0008) |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
--------------------

                         IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=      0.34         1.14
     Dep. Storage     (mm)=      1.00         5.30
     Average Slope     (%)=      2.00         5.00
     Length            (m)=     99.33        40.00
     Mannings n           =     0.013        0.250

     Max.Eff.Inten.(mm/hr)=    127.00        47.44
               over (min)        5.00        10.00
     Storage Coeff.  (min)=      1.88 (ii)    9.10 (ii)
     Unit Hyd. Tpeak (min)=      5.00        10.00
     Unit Hyd. peak  (cms)=      0.32         0.12
                                                          *TOTALS*
     PEAK FLOW       (cms)=      0.12         0.11          0.225 (iii)
     TIME TO PEAK    (hrs)=      3.00         3.08          3.00
     RUNOFF VOLUME    (mm)=     80.48        24.11         37.07
     TOTAL RAINFALL   (mm)=     81.48        81.48         81.48
     RUNOFF COEFFICIENT   =      0.99         0.30          0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| ADD HYD   (0009) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
--------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0007):     0.59   0.059     3.08    27.50
        + ID2= 2 (0008):     1.48   0.225     3.00    37.07
          ================================================
          ID = 3 (0009):     2.07   0.283     3.00    34.35

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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| CALIB            |
| NASHYD    (0011) |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.149 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  29.236
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.359

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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--------------------
| CALIB            |
| STANDHYD  (0010) |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min |   Total Imp(%)=  24.60   Dir. Conn.(%)=  24.60
--------------------

                         IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=      0.31         0.97
     Dep. Storage     (mm)=      1.00         5.80
     Average Slope     (%)=      1.00         6.60
     Length            (m)=     92.38       100.00
     Mannings n           =     0.013        0.250

     Max.Eff.Inten.(mm/hr)=    127.00        44.89
               over (min)        5.00        15.00
     Storage Coeff.  (min)=      2.21 (ii)   13.99 (ii)
     Unit Hyd. Tpeak (min)=      5.00        15.00
     Unit Hyd. peak  (cms)=      0.30         0.08
                                                          *TOTALS*
     PEAK FLOW       (cms)=      0.11         0.07          0.172 (iii)
     TIME TO PEAK    (hrs)=      3.00         3.08          3.00
     RUNOFF VOLUME    (mm)=     80.48        25.11         38.72
     TOTAL RAINFALL   (mm)=     81.48        81.48         81.48
     RUNOFF COEFFICIENT   =      0.99         0.31          0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| ADD HYD   (0012) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
--------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0010):     1.28   0.172     3.00    38.72
        + ID2= 2 (0011):     1.40   0.149     3.08    29.24
          ================================================
          ID = 3 (0012):     2.68   0.320     3.00    33.77

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0005) |   Area    (ha)=   1.73   Curve Number   (CN)= 68.6
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.389

     PEAK FLOW       (cms)=   0.181 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  28.351
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.348

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0014) |   Area    (ha)=   1.52   Curve Number   (CN)= 65.4
|ID= 1 DT= 5.0 min |   Ia      (mm)=   5.10   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.342

     PEAK FLOW       (cms)=   0.153 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  27.578
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.338

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0006) |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.598

     PEAK FLOW       (cms)=   0.264 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  27.291
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.335

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0013) |   Area    (ha)=   0.75   Curve Number   (CN)= 68.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   7.40   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17
```

```
     Unit Hyd Qpeak  (cms)=   0.169

     PEAK FLOW       (cms)=   0.078 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  28.320
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.348

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0017) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.059 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  27.504
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.338

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| STANDHYD  (0018) |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
--------------------
                          IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.34         1.14
     Dep. Storage     (mm)=       1.00         5.30
     Average Slope     (%)=       2.00         5.00
     Length            (m)=      99.33        40.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=     127.00        47.44
               over (min)        5.00        10.00
     Storage Coeff.  (min)=       1.88 (ii)    9.10 (ii)
     Unit Hyd. Tpeak (min)=       5.00        10.00
     Unit Hyd. peak  (cms)=       0.32         0.12
                                                      *TOTALS*
     PEAK FLOW       (cms)=       0.12         0.11          0.225 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.08           3.00
     RUNOFF VOLUME    (mm)=      80.48        24.11          37.07
     TOTAL RAINFALL   (mm)=      81.48        81.48          81.48

     RUNOFF COEFFICIENT   =       0.99         0.30           0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| RESERVOIR (0019) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE   |  OUTFLOW    STORAGE
--------------------      (cms)     (ha.m.)   |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                           0.0001     0.0074   |   0.0165      0.0238
                           0.0045     0.0089   |   0.0180      0.0280
                           0.0084     0.0111   |   0.0193      0.0326
                           0.0110     0.0137   |   0.0206      0.0376
                           0.0131     0.0166   |   0.0218      0.0435
                           0.0149     0.0200   |   0.0000      0.0000

                                AREA     QPEAK     TPEAK        R.V.
                                (ha)     (cms)     (hrs)        (mm)
     INFLOW : ID= 2 (0018)      1.480      0.225      3.00       37.07
     OUTFLOW: ID= 1 (0019)      1.480      0.020      4.00       32.04

                    PEAK   FLOW   REDUCTION [Qout/Qin](%)=  9.12
                    TIME SHIFT OF PEAK FLOW          (min)= 60.00
                    MAXIMUM  STORAGE   USED       (ha.m.)=  0.0372

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--------------------
| ADD HYD   (0020) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
--------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0017):     0.59   0.059     3.08    27.50
        + ID2= 2 (0019):     1.48   0.020     4.00    32.04
          ================================================
          ID = 3 (0020):     2.07   0.077     3.08    30.75

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0023) |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.149 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  29.236
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.359

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| STANDHYD  (0021) |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min |   Total Imp(%)=  25.00   Dir. Conn.(%)=  25.00
--------------------
                          IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.32         0.96
     Dep. Storage     (mm)=       1.00         5.80
     Average Slope     (%)=       1.00         6.60
     Length            (m)=      92.38       100.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=     127.00        44.89
               over (min)        5.00        15.00
     Storage Coeff.  (min)=       2.21 (ii)   13.99 (ii)
     Unit Hyd. Tpeak (min)=       5.00        15.00
     Unit Hyd. peak  (cms)=       0.30         0.08
                                                      *TOTALS*
     PEAK FLOW       (cms)=       0.11         0.07          0.173 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.08           3.00
     RUNOFF VOLUME    (mm)=      80.47        25.11          38.94
     TOTAL RAINFALL   (mm)=      81.48        81.48          81.48
     RUNOFF COEFFICIENT   =       0.99         0.31           0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| RESERVOIR (0022) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE   |  OUTFLOW    STORAGE
--------------------      (cms)     (ha.m.)   |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                           0.0001     0.0052   |   0.0843      0.0172
                           0.0056     0.0066   |   0.0945      0.0200
                           0.0202     0.0082   |   0.1036      0.0231
                           0.0402     0.0101   |   0.1121      0.0265
                           0.0595     0.0122   |   0.1199      0.0301
                           0.0728     0.0146   |   0.0000      0.0000

                                AREA     QPEAK     TPEAK        R.V.
                                (ha)     (cms)     (hrs)        (mm)
     INFLOW : ID= 2 (0021)      1.280      0.173      3.00       38.94
     OUTFLOW: ID= 1 (0022)      1.280      0.085      3.17       34.86

                    PEAK   FLOW   REDUCTION [Qout/Qin](%)= 48.84
                    TIME SHIFT OF PEAK FLOW          (min)= 10.00
                    MAXIMUM  STORAGE   USED       (ha.m.)=  0.0174

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--------------------
| ADD HYD   (0024) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
--------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0022):     1.28   0.085     3.17    34.86
        + ID2= 2 (0023):     1.40   0.149     3.08    29.24
          ================================================
          ID = 3 (0024):     2.68   0.232     3.08    31.92

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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--------------------
| CALIB            |
| NASHYD    (0055) |   Area    (ha)=   1.24   Curve Number   (CN)= 67.5
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.20   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.26

     Unit Hyd Qpeak  (cms)=   0.182

     PEAK FLOW       (cms)=   0.096 (i)
     TIME TO PEAK    (hrs)=   3.167
     RUNOFF VOLUME    (mm)=  27.435
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.337

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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| CALIB            |
| NASHYD    (0056) |   Area    (ha)=   0.58   Curve Number   (CN)= 66.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.26

     Unit Hyd Qpeak  (cms)=   0.085

     PEAK FLOW       (cms)=   0.047 (i)
     TIME TO PEAK    (hrs)=   3.167
     RUNOFF VOLUME    (mm)=  28.510
     TOTAL RAINFALL   (mm)=  81.475
     RUNOFF COEFFICIENT   =   0.350

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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  ****************************
  ** SIMULATION NUMBER:   6 **
  ****************************


--------------------
|    READ STORM    |    Filename: C:\Users\gbergsma\AppD
|                  |              ata\Local\Temp\
|                  |              2e9de3a4-1259-4d17-ae23-36803a62f8c1\3ec6332a
| Ptotal= 89.93 mm |    Comments: 100-Year, 6 hour SCS Type II - Peterboro
--------------------
                TIME    RAIN |  TIME    RAIN |'  TIME    RAIN |  TIME    RAIN
                 hrs   mm/hr |   hrs   mm/hr |'   hrs   mm/hr |   hrs   mm/hr
                0.25    3.60 |  1.75    9.00 |  3.25   19.80 |  4.75    5.40
                0.50    3.60 |  2.00    9.00 |  3.50   19.80 |  5.00    5.40
                0.75    5.40 |  2.25   10.80 |  3.75    9.00 |  5.25    3.60
                1.00    5.40 |  2.50   10.80 |  4.00    9.00 |  5.50    3.60
                1.25    5.40 |  2.75   53.90 |  4.25    7.20 |  5.75    3.60
                1.50    5.40 |  3.00  140.20 |  4.50    7.20 |  6.00    3.60

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--------------------
| CALIB            |
| NASHYD    (0002) |   Area    (ha)=   1.42   Curve Number   (CN)= 63.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   9.10   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.22

        NOTE:  RAINFALL WAS TRANSFORMED TO   5.0 MIN. TIME STEP.

                          ---- TRANSFORMED HYETOGRAPH ----
                TIME    RAIN |  TIME    RAIN |'  TIME    RAIN |  TIME    RAIN
                  hrs   mm/hr |   hrs   mm/hr |'   hrs   mm/hr |   hrs   mm/hr
               0.083    3.60 | 1.583    9.00 | 3.083   19.80 |  4.58    5.40
               0.167    3.60 | 1.667    9.00 | 3.167   19.80 |  4.67    5.40
               0.250    3.60 | 1.750    9.00 | 3.250   19.80 |  4.75    5.40
               0.333    3.60 | 1.833    9.00 | 3.333   19.80 |  4.83    5.40
               0.417    3.60 | 1.917    9.00 | 3.417   19.80 |  4.92    5.40
               0.500    3.60 | 2.000    9.00 | 3.500   19.80 |  5.00    5.40
               0.583    5.40 | 2.083   10.80 | 3.583    9.00 |  5.08    3.60
               0.667    5.40 | 2.167   10.80 | 3.667    9.00 |  5.17    3.60
               0.750    5.40 | 2.250   10.80 | 3.750    9.00 |  5.25    3.60
               0.833    5.40 | 2.333   10.80 | 3.833    9.00 |  5.33    3.60
               0.917    5.40 | 2.417   10.80 | 3.917    9.00 |  5.42    3.60
               1.000    5.40 | 2.500   10.80 | 4.000    9.00 |  5.50    3.60
               1.083    5.40 | 2.583   53.90 | 4.083    7.20 |  5.58    3.60
               1.167    5.40 | 2.667   53.90 | 4.167    7.20 |  5.67    3.60
               1.250    5.40 | 2.750   53.90 | 4.250    7.20 |  5.75    3.60
               1.333    5.40 | 2.833  140.20 | 4.333    7.20 |  5.83    3.60
               1.417    5.40 | 2.917  140.20 | 4.417    7.20 |  5.92    3.60
               1.500    5.40 | 3.000  140.20 | 4.500    7.20 |  6.00    3.60

     Unit Hyd Qpeak  (cms)=   0.247

     PEAK FLOW       (cms)=   0.129 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  28.444
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.316

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0001) |   Area    (ha)=   2.02   Curve Number   (CN)= 61.5
|ID= 1 DT= 5.0 min |   Ia      (mm)=   5.30   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.454

     PEAK FLOW       (cms)=   0.215 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  29.289
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.326

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0015) |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.598

     PEAK FLOW       (cms)=   0.313 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  32.364
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.360

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| RESERVOIR (0016) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE    |  OUTFLOW    STORAGE
--------------------      (cms)     (ha.m.)    |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                         0.0001     0.0072    |   0.1306      0.0171
                         0.0065     0.0083    |   0.1492      0.0194
                         0.0242     0.0096    |   0.1656      0.0220
                         0.0501     0.0112    |   0.1806      0.0248
                         0.0806     0.0129    |   0.1944      0.0278
                         0.1098     0.0149    |   0.0000      0.0000

                               AREA     QPEAK     TPEAK       R.V.
                               (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0015)     2.660      0.313      3.08      32.36
     OUTFLOW: ID= 1 (0016)     2.660      0.186      3.25      29.65

                  PEAK   FLOW   REDUCTION [Qout/Qin](%)= 59.59
                  TIME SHIFT OF PEAK FLOW         (min)= 10.00
                  MAXIMUM  STORAGE   USED       (ha.m.)=  0.0261

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0004) |   Area    (ha)=   2.44   Curve Number   (CN)= 68.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.548

     PEAK FLOW       (cms)=   0.306 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  33.929
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.377

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0003) |   Area    (ha)=   1.53   Curve Number   (CN)= 61.7
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.70   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.344

     PEAK FLOW       (cms)=   0.154 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  27.518
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.306

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| NASHYD    (0007) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
--------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.070 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  32.601
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.363

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
--------------------
| CALIB            |
| STANDHYD  (0008) |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
--------------------

                          IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.34         1.14
     Dep. Storage     (mm)=       1.00         5.30
     Average Slope     (%)=       2.00         5.00
     Length            (m)=      99.33        40.00
     Mannings n           =      0.013        0.250
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```
     Max.Eff.Inten.(mm/hr)=     140.20        56.45
               over (min)        5.00        10.00
     Storage Coeff.  (min)=       1.81 (ii)    8.54 (ii)
     Unit Hyd. Tpeak (min)=       5.00        10.00
     Unit Hyd. peak  (cms)=       0.32         0.12
                                                    *TOTALS*
     PEAK FLOW       (cms)=       0.13         0.13          0.261 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.00           3.00
     RUNOFF VOLUME    (mm)=      88.92        28.75          42.59
     TOTAL RAINFALL   (mm)=      89.93        89.93          89.93
     RUNOFF COEFFICIENT   =       0.99         0.32           0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------

-------------------
| ADD HYD   (0009) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------         (ha)    (cms)    (hrs)     (mm)
         ID1= 1 (0007):     0.59   0.070     3.08    32.60
        + ID2= 2 (0008):    1.48   0.261     3.00    42.59
          ================================================
          ID = 3 (0009):    2.07   0.330     3.00    39.74

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------

-------------------
| CALIB            |
| NASHYD    (0011) |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.176 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  34.567
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.384

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------

-------------------
| CALIB            |
| NASHYD    (0005) |   Area    (ha)=   1.73   Curve Number   (CN)= 68.6
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.389

     PEAK FLOW       (cms)=   0.216 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  33.744
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.375

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| CALIB            |
| NASHYD    (0014) |   Area    (ha)=   1.52   Curve Number   (CN)= 65.4
|ID= 1 DT= 5.0 min |   Ia      (mm)=   5.10   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.342

     PEAK FLOW       (cms)=   0.181 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  32.707
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.364

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| CALIB            |
| NASHYD    (0006) |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.598

     PEAK FLOW       (cms)=   0.313 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  32.364
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.360

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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| CALIB            |
| STANDHYD  (0010) |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min |   Total Imp(%)=  24.60   Dir. Conn.(%)=  24.60
-------------------

                           IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.31         0.97
     Dep. Storage     (mm)=       1.00         5.80
     Average Slope     (%)=       1.00         6.60
     Length            (m)=      92.38       100.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=     140.20        53.60
               over (min)        5.00        15.00
     Storage Coeff.  (min)=       2.13 (ii)   13.10 (ii)
     Unit Hyd. Tpeak (min)=       5.00        15.00
     Unit Hyd. peak  (cms)=       0.31         0.08
                                                    *TOTALS*
     PEAK FLOW       (cms)=       0.12         0.09          0.199 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.08           3.00
     RUNOFF VOLUME    (mm)=      88.92        29.92          44.43
     TOTAL RAINFALL   (mm)=      89.93        89.93          89.93
     RUNOFF COEFFICIENT   =       0.99         0.33           0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
           CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| ADD HYD   (0012) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------         (ha)    (cms)    (hrs)     (mm)
         ID1= 1 (0010):     1.28   0.199     3.00    44.43
        + ID2= 2 (0011):    1.40   0.176     3.08    34.57
          ================================================
          ID = 3 (0012):    2.68   0.374     3.00    39.28

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-------------------
| CALIB            |
| NASHYD    (0013) |   Area    (ha)=   0.75   Curve Number   (CN)= 68.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   7.40   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.169

     PEAK FLOW       (cms)=   0.093 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  33.676
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.374

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| CALIB            |
| NASHYD    (0017) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.070 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  32.601
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.363

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| CALIB            |
| STANDHYD  (0018) |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
-------------------

                           IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.34         1.14
     Dep. Storage     (mm)=       1.00         5.30
     Average Slope     (%)=       2.00         5.00
     Length            (m)=      99.33        40.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=     140.20        56.45
               over (min)        5.00        10.00
     Storage Coeff.  (min)=       1.81 (ii)    8.54 (ii)
     Unit Hyd. Tpeak (min)=       5.00        10.00
     Unit Hyd. peak  (cms)=       0.32         0.12
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                                                        *TOTALS*
     PEAK FLOW       (cms)=       0.13         0.13          0.261 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.00           3.00
     RUNOFF VOLUME    (mm)=      88.92        28.75          42.59
     TOTAL RAINFALL   (mm)=      89.93        89.93          89.93
     RUNOFF COEFFICIENT   =       0.99         0.32           0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| RESERVOIR (0019) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE    |  OUTFLOW    STORAGE
-------------------      (cms)     (ha.m.)    |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                          0.0001     0.0074    |   0.0165      0.0238
                          0.0045     0.0089    |   0.0180      0.0280
                          0.0084     0.0111    |   0.0193      0.0326
                          0.0110     0.0137    |   0.0206      0.0376
                          0.0131     0.0166    |   0.0218      0.0435
                          0.0149     0.0200    |   0.0000      0.0000

                               AREA     QPEAK     TPEAK       R.V.
                               (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0018)     1.480      0.261      3.00      42.59
     OUTFLOW: ID= 1 (0019)     1.480      0.022      4.08      37.56

                    PEAK   FLOW   REDUCTION [Qout/Qin](%)=  8.34
                    TIME SHIFT OF PEAK FLOW         (min)= 65.00
                    MAXIMUM  STORAGE   USED       (ha.m.)=  0.0433

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-------------------
| ADD HYD   (0020) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0017):     0.59   0.070     3.08    32.60
        + ID2= 2 (0019):     1.48   0.022     4.08    37.56
          ================================================
          ID = 3 (0020):     2.07   0.089     3.08    36.14

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
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-------------------
| CALIB             |
| NASHYD    (0023)  |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.176 (i)
     TIME TO PEAK    (hrs)=   3.083
     RUNOFF VOLUME    (mm)=  34.567
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.384

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| STANDHYD  (0021)  |   Area    (ha)=   1.28
|ID= 1 DT= 5.0 min  |   Total Imp(%)=  25.00   Dir. Conn.(%)=  25.00
-------------------

                              IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.32         0.96
     Dep. Storage     (mm)=       1.00         5.80
     Average Slope     (%)=       1.00         6.60
     Length            (m)=      92.38       100.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=     140.20        53.60
                 over (min)        5.00        15.00
     Storage Coeff.  (min)=       2.13 (ii)   13.10 (ii)
     Unit Hyd. Tpeak (min)=       5.00        15.00
     Unit Hyd. peak  (cms)=       0.31         0.08
                                                        *TOTALS*
     PEAK FLOW       (cms)=       0.12         0.09          0.200 (iii)
     TIME TO PEAK    (hrs)=       3.00         3.08           3.00
     RUNOFF VOLUME    (mm)=      88.92        29.92          44.66
     TOTAL RAINFALL   (mm)=      89.93        89.93          89.93
     RUNOFF COEFFICIENT   =       0.99         0.33           0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  62.5    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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-------------------
| RESERVOIR (0022) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE    |  OUTFLOW    STORAGE
-------------------      (cms)     (ha.m.)    |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                          0.0001     0.0052    |   0.0843      0.0172
                          0.0056     0.0066    |   0.0945      0.0200
                          0.0202     0.0082    |   0.1036      0.0231
                          0.0402     0.0101    |   0.1121      0.0265
                          0.0595     0.0122    |   0.1199      0.0301
                          0.0728     0.0146    |   0.0000      0.0000

                               AREA     QPEAK     TPEAK       R.V.
                               (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0021)     1.280      0.200      3.00      44.66
     OUTFLOW: ID= 1 (0022)     1.280      0.094      3.25      40.58

                    PEAK   FLOW   REDUCTION [Qout/Qin](%)= 46.97
                    TIME SHIFT OF PEAK FLOW         (min)= 15.00
                    MAXIMUM  STORAGE   USED       (ha.m.)=  0.0200

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-------------------
| ADD HYD   (0024) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------         (ha)    (cms)    (hrs)     (mm)
          ID1= 1 (0022):     1.28   0.094     3.25    40.58
        + ID2= 2 (0023):     1.40   0.176     3.08    34.57
          ================================================
          ID = 3 (0024):     2.68   0.268     3.08    37.44

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-------------------
| CALIB             |
| NASHYD    (0055)  |   Area    (ha)=   1.24   Curve Number   (CN)= 67.5
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   8.20   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.26

     Unit Hyd Qpeak  (cms)=   0.182

     PEAK FLOW       (cms)=   0.115 (i)
     TIME TO PEAK    (hrs)=   3.167
     RUNOFF VOLUME    (mm)=  32.714
     TOTAL RAINFALL   (mm)=  89.925
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     RUNOFF COEFFICIENT   =   0.364

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| CALIB             |
| NASHYD    (0056)  |   Area    (ha)=   0.58   Curve Number   (CN)= 66.1
|ID= 1 DT= 5.0 min  |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.26

     Unit Hyd Qpeak  (cms)=   0.085

     PEAK FLOW       (cms)=   0.055 (i)
     TIME TO PEAK    (hrs)=   3.167
     RUNOFF VOLUME    (mm)=  33.746
     TOTAL RAINFALL   (mm)=  89.925
     RUNOFF COEFFICIENT   =   0.375

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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  ****************************
  ** SIMULATION NUMBER:   7 **
  ****************************

-------------------
| CHICAGO STORM    |    IDF curve parameters: A= 405.000
| Ptotal= 24.91 mm |                          B=   3.000
-------------------                          C=   0.760
                         used in:   INTENSITY =  A / (t + B)^C

                         Duration of storm  =  4.00 hrs
                         Storm time step    = 10.00 min
                         Time to peak ratio =  0.33

                TIME    RAIN |  TIME    RAIN |'  TIME    RAIN |  TIME    RAIN
                 hrs   mm/hr |   hrs   mm/hr |'   hrs   mm/hr |   hrs   mm/hr
                0.17    1.76 |  1.17   11.75 |  2.17    3.88 |  3.17    2.07
                0.33    2.00 |  1.33   57.66 |  2.33    3.35 |  3.33    1.93
                0.50    2.32 |  1.50   15.20 |  2.50    2.96 |  3.50    1.81
                0.67    2.81 |  1.67    8.31 |  2.67    2.66 |  3.67    1.71
                0.83    3.61 |  1.83    5.91 |  2.83    2.42 |  3.83    1.62
                1.00    5.28 |  2.00    4.66 |  3.00    2.23 |  4.00    1.54

--------------------------------------------------------------------------------
-------------------
| CALIB             |
| NASHYD    (0002)  |   Area    (ha)=   1.42   Curve Number   (CN)= 63.1
```

```
|ID= 1 DT= 5.0 min |   Ia      (mm)=   9.10   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.22

         NOTE:  RAINFALL WAS TRANSFORMED TO   5.0 MIN. TIME STEP.

                          ---- TRANSFORMED HYETOGRAPH ----
                TIME    RAIN |  TIME    RAIN |'  TIME    RAIN |  TIME    RAIN
                 hrs   mm/hr |   hrs   mm/hr |'   hrs   mm/hr |   hrs   mm/hr
               0.083    1.76 | 1.083   11.75 | 2.083    3.88 |  3.08    2.07
               0.167    1.76 | 1.167   11.75 | 2.167    3.88 |  3.17    2.07
               0.250    2.00 | 1.250   57.66 | 2.250    3.35 |  3.25    1.93
               0.333    2.00 | 1.333   57.66 | 2.333    3.35 |  3.33    1.93
               0.417    2.32 | 1.417   15.20 | 2.417    2.96 |  3.42    1.81
               0.500    2.32 | 1.500   15.20 | 2.500    2.96 |  3.50    1.81
               0.583    2.81 | 1.583    8.31 | 2.583    2.66 |  3.58    1.71
               0.667    2.81 | 1.667    8.31 | 2.667    2.66 |  3.67    1.71
               0.750    3.61 | 1.750    5.91 | 2.750    2.42 |  3.75    1.62
               0.833    3.61 | 1.833    5.91 | 2.833    2.42 |  3.83    1.62
               0.917    5.28 | 1.917    4.66 | 2.917    2.23 |  3.92    1.54
               1.000    5.28 | 2.000    4.66 | 3.000    2.23 |  4.00    1.54

     Unit Hyd Qpeak  (cms)=   0.247

     PEAK FLOW       (cms)=   0.004 (i)
     TIME TO PEAK    (hrs)=   1.667
     RUNOFF VOLUME    (mm)=   1.518
     TOTAL RAINFALL   (mm)=  24.906
     RUNOFF COEFFICIENT   =   0.061

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB           |
| NASHYD    (0001) |   Area    (ha)=   2.02   Curve Number   (CN)= 61.5
|ID= 1 DT= 5.0 min |   Ia      (mm)=   5.30   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.454

     PEAK FLOW       (cms)=   0.012 (i)
     TIME TO PEAK    (hrs)=   1.500
     RUNOFF VOLUME    (mm)=   2.144
     TOTAL RAINFALL   (mm)=  24.906
     RUNOFF COEFFICIENT   =   0.086

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| CALIB           |
| NASHYD    (0002) |   Area    (ha)=   2.66   Curve Number   (CN)= 64.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.80   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.598

     PEAK FLOW       (cms)=   0.019 (i)
     TIME TO PEAK    (hrs)=   1.500
     RUNOFF VOLUME    (mm)=   2.548
     TOTAL RAINFALL   (mm)=  24.906
     RUNOFF COEFFICIENT   =   0.102

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| RESERVOIR (0016) |
| IN= 2---> OUT= 1 |
| DT=  5.0 min     |     OUTFLOW    STORAGE    |  OUTFLOW    STORAGE
-------------------      (cms)     (ha.m.)    |   (cms)     (ha.m.)
     **** WARNING : FIRST OUTFLOW IS NOT ZERO.
                           0.0001     0.0072    |   0.1306      0.0171
                           0.0065     0.0083    |   0.1492      0.0194
                           0.0242     0.0096    |   0.1656      0.0220
                           0.0501     0.0112    |   0.1806      0.0248
                           0.0806     0.0129    |   0.1944      0.0278
                           0.1098     0.0149    |   0.0000      0.0000

                                AREA     QPEAK     TPEAK       R.V.
                                (ha)     (cms)     (hrs)       (mm)
     INFLOW : ID= 2 (0015)      2.660      0.019      1.50       2.55
     OUTFLOW: ID= 1 (0016)      2.660      0.000      4.58       0.03

                   PEAK   FLOW   REDUCTION [Qout/Qin](%)=  0.50
                   TIME SHIFT OF PEAK FLOW         (min)=185.00
                   MAXIMUM  STORAGE   USED       (ha.m.)=  0.0067

--------------------------------------------------------------------------------
-------------------
| CALIB           |
| NASHYD    (0004) |   Area    (ha)=   2.44   Curve Number   (CN)= 68.8
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.00   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.548

     PEAK FLOW       (cms)=   0.012 (i)
     TIME TO PEAK    (hrs)=   1.583

     RUNOFF VOLUME    (mm)=   2.156
     TOTAL RAINFALL   (mm)=  24.906
     RUNOFF COEFFICIENT   =   0.087

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB           |
| NASHYD    (0003) |   Area    (ha)=   1.53   Curve Number   (CN)= 61.7
|ID= 1 DT= 5.0 min |   Ia      (mm)=   8.70   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.344

     PEAK FLOW       (cms)=   0.005 (i)
     TIME TO PEAK    (hrs)=   1.583
     RUNOFF VOLUME    (mm)=   1.505
     TOTAL RAINFALL   (mm)=  24.906
     RUNOFF COEFFICIENT   =   0.060

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| CALIB           |
| NASHYD    (0007) |   Area    (ha)=   0.59   Curve Number   (CN)= 65.0
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.70   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.133

     PEAK FLOW       (cms)=   0.004 (i)
     TIME TO PEAK    (hrs)=   1.500
     RUNOFF VOLUME    (mm)=   2.591
     TOTAL RAINFALL   (mm)=  24.906
     RUNOFF COEFFICIENT   =   0.104

     (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-------------------
| CALIB           |
| STANDHYD  (0008) |   Area    (ha)=   1.48
|ID= 1 DT= 5.0 min |   Total Imp(%)=  23.00   Dir. Conn.(%)=  23.00
-------------------
                          IMPERVIOUS    PERVIOUS (i)
     Surface Area     (ha)=       0.34         1.14
     Dep. Storage     (mm)=       1.00         5.30
     Average Slope     (%)=       2.00         5.00
     Length            (m)=      99.33        40.00
     Mannings n           =      0.013        0.250

     Max.Eff.Inten.(mm/hr)=      57.66         1.95
               over (min)        5.00        30.00
     Storage Coeff.  (min)=       2.58 (ii)   28.49 (ii)
     Unit Hyd. Tpeak (min)=       5.00        30.00
     Unit Hyd. peak  (cms)=       0.29         0.04
                                                          *TOTALS*
     PEAK FLOW       (cms)=       0.05         0.00          0.054 (iii)
     TIME TO PEAK    (hrs)=       1.33         1.92          1.33
     RUNOFF VOLUME    (mm)=      23.91        2.09           7.09
     TOTAL RAINFALL   (mm)=      24.91        24.91         24.91
     RUNOFF COEFFICIENT   =       0.96         0.08          0.28

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

       (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
            CN*  =  60.7    Ia = Dep. Storage  (Above)
      (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
           THAN THE STORAGE COEFFICIENT.
     (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--------------------------------------------------------------------------------
-------------------
| ADD HYD   (0009) |
|   1 +  2 =  3    |         AREA    QPEAK    TPEAK     R.V.
-------------------          (ha)    (cms)    (hrs)     (mm)
         ID1= 1 (0007):     0.59   0.004     1.50     2.59
       + ID2= 2 (0008):     1.48   0.054     1.33     7.09
         ================================================
         ID = 3 (0009):     2.07   0.057     1.33     5.81

     NOTE:  PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

--------------------------------------------------------------------------------
-------------------
| CALIB           |
| NASHYD    (0011) |   Area    (ha)=   1.40   Curve Number   (CN)= 67.1
|ID= 1 DT= 5.0 min |   Ia      (mm)=   4.60   # of Linear Res.(N)= 3.00
-------------------   U.H. Tp(hrs)=   0.17

     Unit Hyd Qpeak  (cms)=   0.315

     PEAK FLOW       (cms)=   0.011 (i)
     TIME TO PEAK    (hrs)=   1.500
     RUNOFF VOLUME    (mm)=   2.836
     TOTAL RAINFALL   (mm)=  24.906
     RUNOFF COEFFICIENT   =   0.114
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
STANDHYD (0010)	Area (ha)= 1.28			
ID= 1 DT= 5.0 min	Total Imp(%)= 24.60	Dir. Conn.(%)= 24.60		

		IMPERVIOUS	PERVIOUS (i)	
Surface Area	(ha)=	0.31	0.97	
Dep. Storage	(mm)=	1.00	5.80	
Average Slope	(%)=	1.00	6.60	
Length	(m)=	92.38	100.00	
Mannings n	=	0.013	0.250	
Max.Eff.Inten.(mm/hr)=		57.66	1.57	
over (min)		5.00	50.00	
Storage Coeff. (min)=		3.04 (ii)	48.01 (ii)	
Unit Hyd. Tpeak (min)=		5.00	50.00	
Unit Hyd. peak (cms)=		0.27	0.02	
				TOTALS
PEAK FLOW	(cms)=	0.05	0.00	0.049 (iii)
TIME TO PEAK	(hrs)=	1.33	2.33	1.33
RUNOFF VOLUME	(mm)=	23.91	2.13	7.46
TOTAL RAINFALL	(mm)=	24.91	24.91	24.91
RUNOFF COEFFICIENT	=	0.96	0.09	0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 62.5 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0012)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
	ID1= 1 (0010):	1.28	0.049	1.33	7.46
	+ ID2= 2 (0011):	1.40	0.011	1.50	2.84
	ID = 3 (0012):	2.68	0.056	1.33	5.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (0013)	Area (ha)= 0.75	Curve Number (CN)= 68.1		
ID= 1 DT= 5.0 min	Ia (mm)= 7.40	# of Linear Res.(N)= 3.00		
	U.H. Tp(hrs)= 0.17			

Unit Hyd Qpeak (cms)= 0.169

PEAK FLOW (cms)= 0.004 (i)
TIME TO PEAK (hrs)= 1.500
RUNOFF VOLUME (mm)= 2.237
TOTAL RAINFALL (mm)= 24.906
RUNOFF COEFFICIENT = 0.090

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHYD (0017)	Area (ha)= 0.59	Curve Number (CN)= 65.0		
ID= 1 DT= 5.0 min	Ia (mm)= 4.70	# of Linear Res.(N)= 3.00		
	U.H. Tp(hrs)= 0.17			

Unit Hyd Qpeak (cms)= 0.133

PEAK FLOW (cms)= 0.004 (i)
TIME TO PEAK (hrs)= 1.500
RUNOFF VOLUME (mm)= 2.591
TOTAL RAINFALL (mm)= 24.906
RUNOFF COEFFICIENT = 0.104

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
STANDHYD (0018)	Area (ha)= 1.48			
ID= 1 DT= 5.0 min	Total Imp(%)= 23.00	Dir. Conn.(%)= 23.00		

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.34	1.14
Dep. Storage	(mm)=	1.00	5.30
Average Slope	(%)=	2.00	5.00
Length	(m)=	99.33	40.00
Mannings n	=	0.013	0.250
Max.Eff.Inten.(mm/hr)=		57.66	1.95
over (min)		5.00	30.00
Storage Coeff. (min)=		2.58 (ii)	28.49 (ii)

CALIB				
NASHYD (0005)	Area (ha)= 1.73	Curve Number (CN)= 68.6		
ID= 1 DT= 5.0 min	Ia (mm)= 8.00	# of Linear Res.(N)= 3.00		
	U.H. Tp(hrs)= 0.17			

Unit Hyd Qpeak (cms)= 0.389

PEAK FLOW (cms)= 0.008 (i)
TIME TO PEAK (hrs)= 1.583
RUNOFF VOLUME (mm)= 2.138
TOTAL RAINFALL (mm)= 24.906
RUNOFF COEFFICIENT = 0.086

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHYD (0014)	Area (ha)= 1.52	Curve Number (CN)= 65.4		
ID= 1 DT= 5.0 min	Ia (mm)= 5.10	# of Linear Res.(N)= 3.00		
	U.H. Tp(hrs)= 0.17			

Unit Hyd Qpeak (cms)= 0.342

PEAK FLOW (cms)= 0.010 (i)
TIME TO PEAK (hrs)= 1.500
RUNOFF VOLUME (mm)= 2.535
TOTAL RAINFALL (mm)= 24.906
RUNOFF COEFFICIENT = 0.102

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHYD (0006)	Area (ha)= 2.66	Curve Number (CN)= 64.8		
ID= 1 DT= 5.0 min	Ia (mm)= 4.80	# of Linear Res.(N)= 3.00		
	U.H. Tp(hrs)= 0.17			

Unit Hyd Qpeak (cms)= 0.598

PEAK FLOW (cms)= 0.019 (i)
TIME TO PEAK (hrs)= 1.500
RUNOFF VOLUME (mm)= 2.548
TOTAL RAINFALL (mm)= 24.906
RUNOFF COEFFICIENT = 0.102

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
STANDHYD (0004)	Area (ha)= 1.28			
ID= 1 DT= 5.0 min	Total Imp(%)= 24.60	Dir. Conn.(%)= 24.60		

		IMPERVIOUS	PERVIOUS (i)	
Surface Area	(ha)=	0.31	0.97	
Dep. Storage	(mm)=	1.00	5.80	
Average Slope	(%)=	1.00	6.60	
Length	(m)=	92.38	100.00	
Mannings n	=	0.013	0.250	
Max.Eff.Inten.(mm/hr)=		57.66	1.57	
over (min)		5.00	50.00	
Storage Coeff. (min)=		3.04 (ii)	48.01 (ii)	
Unit Hyd. Tpeak (min)=		5.00	30.00	
Unit Hyd. peak (cms)=		0.29	0.04	
				TOTALS
PEAK FLOW	(cms)=	0.05	0.00	0.054 (iii)
TIME TO PEAK	(hrs)=	1.33	1.92	1.33
RUNOFF VOLUME	(mm)=	23.91	2.09	7.09
TOTAL RAINFALL	(mm)=	24.91	24.91	24.91
RUNOFF COEFFICIENT	=	0.96	0.08	0.28

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 60.7 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0019)				
IN= 2---> OUT= 1				
DT= 5.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
**** WARNING : FIRST OUTFLOW IS NOT ZERO.				
	0.0001	0.0074	0.0165	0.0238
	0.0045	0.0089	0.0180	0.0280
	0.0084	0.0111	0.0193	0.0326
	0.0110	0.0137	0.0206	0.0376
	0.0131	0.0166	0.0218	0.0435
	0.0149	0.0200	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0018)	1.480	0.054	1.33	7.09
OUTFLOW: ID= 1 (0019)	1.480	0.003	3.50	2.06

PEAK FLOW REDUCTION [Qout/Qin](%)= 5.96
TIME SHIFT OF PEAK FLOW (min)=130.00
MAXIMUM STORAGE USED (ha.m.)= 0.0085

ADD HYD (0020)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
	ID1= 1 (0017):	0.59	0.004	1.50	2.59
	+ ID2= 2 (0019):	1.48	0.003	3.50	2.06
	ID = 3 (0020):	2.07	0.004	1.50	2.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB					
NASHYD (0023)	Area	(ha)=	1.40	Curve Number	(CN)= 67.1
ID= 1 DT= 5.0 min	Ia	(mm)=	4.60	# of Linear Res.(N)= 3.00	
	U.H. Tp(hrs)=		0.17		

Unit Hyd Qpeak (cms)= 0.315

PEAK FLOW (cms)= 0.011 (i)
TIME TO PEAK (hrs)= 1.500
RUNOFF VOLUME (mm)= 2.836
TOTAL RAINFALL (mm)= 24.906
RUNOFF COEFFICIENT = 0.114

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB					
STANDHYD (0021)	Area	(ha)=	1.28		
ID= 1 DT= 5.0 min	Total Imp(%)=		25.00	Dir. Conn.(%)=	25.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.32	0.96
Dep. Storage (mm)=	1.00	5.80
Average Slope (%)=	1.00	6.60
Length (m)=	92.38	100.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	57.66	1.57
over (min)	5.00	50.00
Storage Coeff. (min)=	3.04 (ii)	48.01 (ii)
Unit Hyd. Tpeak (min)=	5.00	50.00
Unit Hyd. peak (cms)=	0.27	0.02

		TOTALS
PEAK FLOW (cms)=	0.05	0.00	0.050 (iii)
TIME TO PEAK (hrs)=	1.33	2.33	1.33
RUNOFF VOLUME (mm)=	23.91	2.13	7.55
TOTAL RAINFALL (mm)=	24.91	24.91	24.91
RUNOFF COEFFICIENT =	0.96	0.09	0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 62.5 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0022)				
IN= 2---> OUT= 1				
DT= 5.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)

**** WARNING : FIRST OUTFLOW IS NOT ZERO.

0.0001	0.0052	0.0843	0.0172
0.0056	0.0066	0.0945	0.0200
0.0202	0.0082	0.1036	0.0231
0.0402	0.0101	0.1121	0.0265
0.0595	0.0122	0.1199	0.0301
0.0728	0.0146	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0021)	1.280	0.050	1.33	7.55
OUTFLOW: ID= 1 (0022)	1.280	0.004	2.75	3.46

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.63
TIME SHIFT OF PEAK FLOW (min)= 85.00
MAXIMUM STORAGE USED (ha.m.)= 0.0063

ADD HYD (0024)					
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (0022):	1.28	0.004	2.75	3.46	
+ ID2= 2 (0023):	1.40	0.011	1.50	2.84	
ID = 3 (0024):	2.68	0.011	1.50	3.14	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB					
NASHYD (0055)	Area	(ha)=	1.24	Curve Number	(CN)= 67.5
ID= 1 DT= 5.0 min	Ia	(mm)=	8.20	# of Linear Res.(N)= 3.00	
	U.H. Tp(hrs)=		0.26		

Unit Hyd Qpeak (cms)= 0.182

PEAK FLOW (cms)= 0.005 (i)
TIME TO PEAK (hrs)= 1.667

RUNOFF VOLUME (mm)= 2.006
TOTAL RAINFALL (mm)= 24.906
RUNOFF COEFFICIENT = 0.081

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB					
NASHYD (0056)	Area	(ha)=	0.58	Curve Number	(CN)= 66.1
ID= 1 DT= 5.0 min	Ia	(mm)=	4.60	# of Linear Res.(N)= 3.00	
	U.H. Tp(hrs)=		0.26		

Unit Hyd Qpeak (cms)= 0.085

PEAK FLOW (cms)= 0.003 (i)
TIME TO PEAK (hrs)= 1.667
RUNOFF VOLUME (mm)= 2.735
TOTAL RAINFALL (mm)= 24.906
RUNOFF COEFFICIENT = 0.110

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

FINISH

Appendix C

Stormwater Facility Design

3.3.2 Water Quality Sizing Criteria

The volumetric water quality criteria are presented in Table 3.2. The values are based on a 24 hour drawdown time and a design which conforms to the guidance provided in this manual. Requirements differ with SWMP type to reflect differences in removal efficiencies. Of the specified storage volume for wet facilities, 40 m³/ha is extended detention, while the remainder represents the permanent pool.

Table 3.2 Water Quality Storage Requirements based on Receiving Waters[1, 2]

Protection Level	SWMP Type	Storage Volume (m³/ha) for Impervious Level			
		35%	55%	70%	85%
Enhanced 80% long-term S.S. removal	Infiltration	25	30	35	40
	Wetlands	80	105	120	140
	Hybrid Wet Pond/Wetland	110	150	175	195
	Wet Pond	140	190	225	250
Normal 70% long-term S.S. removal	Infiltration	20	20	25	30
	Wetlands	60	70	80	90
	Hybrid Wet Pond/Wetland	75	90	105	120
	Wet Pond	90	110	130	150
Basic 60% long-term S.S. removal	Infiltration	20	20	20	20
	Wetlands	60	60	60	60
	Hybrid Wet Pond/Wetland	60	70	75	80
	Wet Pond	60	75	85	95
	Dry Pond (Continuous Flow)	90	150	200	240

[1]Table 3.2 does not include every available SWMP type. Any SWMP type that can be demonstrated to the approval agencies to meet the required long-term suspended solids removal for the selected protection levels under the conditions of the site is acceptable for water quality objectives. The sizing for these SWMP types is to be determined based on performance results that have been peer-reviewed. The designer and those who review the design should be fully aware of the assumptions and sampling methodologies used in formulating performance predictions and their implications for the design.

[2]Hybrid Wet Pond/Wetland systems have 50-60% of their permanent pool volume in deeper portions of the facility (e.g., forebay, wet pond).

Stage-Storage-Discharge: PR-100 Roadside Ditch

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: MW / CPB
Date: February 23, 2022

Storage Summary		
Top of Dead Storage:	1.00	m
Dead Storage Volume:	71.6	m^3
Active Storage Volume:	206.5	m^3

Outlet Capacity Summary				
Type	Diameter	Slope	Peak Flow	% Full

Discharge Summary			
Stage	Type	Invert Elev (m)	Diameter / Width (mm) (m)
1	Orifice Tube: Vertical	1.00	325

Stage-Storage-Discharge Summary Table

Elevation	Stage	Stage 1 Orifice Tube						Total Storage	Total Discharge	Notes
m	m	m^3/s						ha*m	m^3/s	
1.00	0.00	0.000						**0.0072**	**0.000**	
1.01	0.01	**0.000**						**0.0073**	**0.000**	
1.02	0.02	**0.001**						**0.0075**	**0.001**	
1.03	0.03	**0.002**						**0.0077**	**0.002**	
1.04	0.04	**0.003**						**0.0079**	**0.003**	
1.05	0.05	**0.005**						**0.0081**	**0.005**	
1.06	0.06	**0.006**						**0.0083**	**0.006**	
1.07	0.07	**0.009**						**0.0085**	**0.009**	
1.08	0.08	**0.011**						**0.0087**	**0.011**	
1.09	0.09	**0.014**						**0.0089**	**0.014**	
1.10	0.10	**0.017**						**0.0091**	**0.017**	
1.11	0.11	**0.021**						**0.0094**	**0.021**	<= 2 Yr: 93 m³ (1.11m)
1.12	0.12	**0.024**						**0.0096**	**0.024**	
1.13	0.13	**0.028**						**0.0099**	**0.028**	
1.14	0.14	**0.032**						**0.0101**	**0.032**	
1.15	0.15	**0.036**						**0.0104**	**0.036**	
1.16	0.16	**0.041**						**0.0106**	**0.041**	
1.17	0.17	**0.045**						**0.0109**	**0.045**	
1.18	0.18	**0.050**						**0.0112**	**0.050**	
1.19	0.19	**0.055**						**0.0115**	**0.055**	
1.20	0.20	**0.060**						**0.0117**	**0.060**	
1.21	0.21	**0.065**						**0.0120**	**0.065**	
1.22	0.22	**0.070**						**0.0123**	**0.070**	<= 5 Yr: 123 m³ (1.22m)
1.23	0.23	**0.075**						**0.0126**	**0.075**	
1.24	0.24	**0.081**						**0.0129**	**0.081**	
1.25	0.25	**0.086**						**0.0133**	**0.086**	
1.26	0.26	**0.091**						**0.0136**	**0.091**	
1.27	0.27	**0.096**						**0.0139**	**0.096**	
1.28	0.28	**0.101**						**0.0142**	**0.101**	
1.29	0.29	**0.105**						**0.0146**	**0.105**	
1.30	0.30	**0.110**						**0.0149**	**0.110**	<= 10 Yr: 146 m³ (1.3m)
1.31	0.31	**0.114**						**0.0153**	**0.114**	
1.32	0.32	**0.117**						**0.0156**	**0.117**	
1.33	0.33	**0.120**						**0.0160**	**0.120**	
1.34	0.34	**0.124**						**0.0163**	**0.124**	
1.35	0.35	**0.127**						**0.0167**	**0.127**	
1.36	0.36	**0.131**						**0.0171**	**0.131**	
1.37	0.37	**0.134**						**0.0175**	**0.134**	
1.38	0.38	**0.137**						**0.0178**	**0.137**	
1.39	0.39	**0.140**						**0.0182**	**0.140**	
1.40	0.40	**0.143**						**0.0186**	**0.143**	<= 25 Yr: 185 m³ (1.4m)
1.41	0.41	**0.146**						**0.0190**	**0.146**	
1.42	0.42	**0.149**						**0.0194**	**0.149**	
1.43	0.43	**0.152**						**0.0199**	**0.152**	
1.44	0.44	**0.155**						**0.0203**	**0.155**	
1.45	0.45	**0.158**						**0.0207**	**0.158**	
1.46	0.46	**0.160**						**0.0211**	**0.160**	
1.47	0.47	**0.163**						**0.0216**	**0.163**	
1.48	0.48	**0.166**						**0.0220**	**0.166**	<= 50 Yr: 220 m³ (1.48m)

Stage-Storage-Discharge Summary Table										
Elevation	**Stage**	**Stage 1 Orifice Tube**						**Total Storage**	**Total Discharge**	**Notes**
m	**m**	**m^3/s**						**ha*m**	**m^3/s**	
1.49	0.49	**0.168**						**0.0225**	**0.168**	
1.50	0.50	**0.171**						**0.0229**	**0.171**	
1.51	0.51	**0.173**						**0.0234**	**0.173**	
1.52	0.52	**0.176**						**0.0239**	**0.176**	
1.53	0.53	**0.178**						**0.0243**	**0.178**	
1.54	0.54	**0.181**						**0.0248**	**0.181**	
1.55	0.55	**0.183**						**0.0253**	**0.183**	
1.56	0.56	**0.185**						**0.0258**	**0.185**	
1.57	0.57	**0.188**						**0.0263**	**0.188**	<= 100 Yr: 261 m³ (1.57m)
1.58	0.58	**0.190**						**0.0268**	**0.190**	
1.59	0.59	**0.192**						**0.0273**	**0.192**	
1.60	0.60	**0.194**						**0.0278**	**0.194**	

Stage-Storage-Discharge: PR-102 Roadside Ditch

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: MW / CPB
Date: February 23, 2022

Storage Summary		
Top of Dead Storage:	1.00	m
Dead Storage Volume:	78.3	m^3
Active Storage Volume:	356.7	m^3

Outlet Capacity Summary				
Type	Diameter	Slope	Peak Flow	% Full

Discharge Summary				
Stage	Type	Invert Elev (m)	Diameter / Width (mm)	(m)
1	Orifice Tube: Vertical	1.00	95	

Stage-Storage-Discharge Summary Table

Elevation	Stage	Stage 1 Orifice Tube						Total Storage	Total Discharge	Notes
m	m	m^3/s						ha*m	m^3/s	
1.00	0.00	0.000						**0.0074**	**0.000**	
1.01	0.01	**0.000**						**0.0073**	**0.000**	
1.02	0.02	**0.000**						**0.0075**	**0.000**	
1.03	0.03	**0.001**						**0.0077**	**0.001**	
1.04	0.04	**0.001**						**0.0080**	**0.001**	
1.05	0.05	**0.002**						**0.0082**	**0.002**	
1.06	0.06	**0.003**						**0.0084**	**0.003**	
1.07	0.07	**0.004**						**0.0086**	**0.004**	<= 25 mm: 85 m^3 (1.07m)
1.08	0.08	**0.005**						**0.0089**	**0.005**	
1.09	0.09	**0.005**						**0.0091**	**0.005**	
1.10	0.10	**0.006**						**0.0094**	**0.006**	
1.11	0.11	**0.006**						**0.0097**	**0.006**	
1.12	0.12	**0.007**						**0.0099**	**0.007**	
1.13	0.13	**0.007**						**0.0102**	**0.007**	
1.14	0.14	**0.008**						**0.0105**	**0.008**	
1.15	0.15	**0.008**						**0.0108**	**0.008**	
1.16	0.16	**0.008**						**0.0111**	**0.008**	
1.17	0.17	**0.009**						**0.0114**	**0.009**	
1.18	0.18	**0.009**						**0.0117**	**0.009**	
1.19	0.19	**0.009**						**0.0120**	**0.009**	
1.20	0.20	**0.010**						**0.0123**	**0.010**	
1.21	0.21	**0.010**						**0.0126**	**0.010**	<= 2 Yr: 125 m^3 (1.21m)
1.22	0.22	**0.010**						**0.0130**	**0.010**	
1.23	0.23	**0.011**						**0.0133**	**0.011**	
1.24	0.24	**0.011**						**0.0137**	**0.011**	
1.25	0.25	**0.011**						**0.0140**	**0.011**	
1.26	0.26	**0.012**						**0.0144**	**0.012**	
1.27	0.27	**0.012**						**0.0147**	**0.012**	
1.28	0.28	**0.012**						**0.0151**	**0.012**	
1.29	0.29	**0.012**						**0.0155**	**0.012**	
1.30	0.30	**0.013**						**0.0158**	**0.013**	
1.31	0.31	**0.013**						**0.0162**	**0.013**	
1.32	0.32	**0.013**						**0.0166**	**0.013**	
1.33	0.33	**0.013**						**0.0170**	**0.013**	
1.34	0.34	**0.014**						**0.0174**	**0.014**	
1.35	0.35	**0.014**						**0.0178**	**0.014**	
1.36	0.36	**0.014**						**0.0183**	**0.014**	
1.37	0.37	**0.014**						**0.0187**	**0.014**	
1.38	0.38	**0.014**						**0.0191**	**0.014**	<= 5 Yr: 190 m^3 (1.38m)
1.39	0.39	**0.015**						**0.0196**	**0.015**	
1.40	0.40	**0.015**						**0.0200**	**0.015**	
1.41	0.41	**0.015**						**0.0204**	**0.015**	
1.42	0.42	**0.015**						**0.0209**	**0.015**	
1.43	0.43	**0.016**						**0.0214**	**0.016**	
1.44	0.44	**0.016**						**0.0218**	**0.016**	
1.45	0.45	**0.016**						**0.0223**	**0.016**	
1.46	0.46	**0.016**						**0.0228**	**0.016**	
1.47	0.47	**0.016**						**0.0233**	**0.016**	
1.48	0.48	**0.017**						**0.0238**	**0.017**	

Stage-Storage-Discharge Summary Table										
Elevation	**Stage**	**Stage 1 Orifice Tube**						**Total Storage**	**Total Discharge**	**Notes**
m	**m**	**m^3/s**						**ha*m**	**m^3/s**	
1.49	0.49	**0.017**						**0.0243**	**0.017**	<= 10 Yr: 241 m³ (1.49m)
1.50	0.50	**0.017**						**0.0248**	**0.017**	
1.51	0.51	**0.017**						**0.0253**	**0.017**	
1.52	0.52	**0.017**						**0.0258**	**0.017**	
1.53	0.53	**0.017**						**0.0263**	**0.017**	
1.54	0.54	**0.018**						**0.0269**	**0.018**	
1.55	0.55	**0.018**						**0.0274**	**0.018**	
1.56	0.56	**0.018**						**0.0280**	**0.018**	
1.57	0.57	**0.018**						**0.0285**	**0.018**	
1.58	0.58	**0.018**						**0.0291**	**0.018**	
1.59	0.59	**0.019**						**0.0296**	**0.019**	
1.60	0.60	**0.019**						**0.0302**	**0.019**	
1.61	0.61	**0.019**						**0.0308**	**0.019**	
1.62	0.62	**0.019**						**0.0314**	**0.019**	<= 25 Yr: 313 m³ (1.62m)
1.63	0.63	**0.019**						**0.0320**	**0.019**	
1.64	0.64	**0.019**						**0.0326**	**0.019**	
1.65	0.65	**0.019**						**0.0332**	**0.019**	
1.66	0.66	**0.020**						**0.0338**	**0.020**	
1.67	0.67	**0.020**						**0.0344**	**0.020**	
1.68	0.68	**0.020**						**0.0350**	**0.020**	
1.69	0.69	**0.020**						**0.0356**	**0.020**	
1.70	0.70	**0.020**						**0.0363**	**0.020**	
1.71	0.71	**0.020**						**0.0369**	**0.020**	
1.72	0.72	**0.021**						**0.0376**	**0.021**	<= 50 Yr: 372 m³ (1.72m)
1.73	0.73	**0.021**						**0.0382**	**0.021**	
1.74	0.74	**0.021**						**0.0389**	**0.021**	
1.75	0.75	**0.021**						**0.0396**	**0.021**	
1.76	0.76	**0.021**						**0.0402**	**0.021**	
1.77	0.77	**0.021**						**0.0409**	**0.021**	
1.78	0.78	**0.021**						**0.0416**	**0.021**	
1.79	0.79	**0.022**						**0.0425**	**0.022**	
1.80	0.80	**0.022**						**0.0435**	**0.022**	<= 100 Yr: 433 m³ (1.8m)

Stage-Storage-Discharge: PR-201 Roadside Ditch

Project No: 19-10874
Project Name: Life at the Woodland
Designed/Checked By: MW / CPB
Date: February 23, 2022

Storage Summary		
Top of Dead Storage:	1.00	m
Dead Storage Volume:	51.9	m^3
Active Storage Volume:	248.7	m^3

Outlet Capacity Summary				
Type	Diameter	Slope	Peak Flow	% Full

Discharge Summary				
Stage	Type	Invert Elev (m)	Diameter / Width (mm)	(m)
1	Orifice Tube: Vertical	1.00	250	

Stage-Storage-Discharge Summary Table

Elevation	Stage	Stage 1 Orifice Tube						Total Storage	Total Discharge	Notes
m	m	m^3/s						ha*m	m^3/s	
1.00	0.00	0.000						**0.0052**	**0.000**	
1.01	0.01	**0.000**						**0.0054**	**0.000**	
1.02	0.02	**0.001**						**0.0056**	**0.001**	
1.03	0.03	**0.001**						**0.0058**	**0.001**	
1.04	0.04	**0.003**						**0.0061**	**0.003**	
1.05	0.05	**0.004**						**0.0063**	**0.004**	<= 25 mm: 63 m³ (1.05m)
1.06	0.06	**0.006**						**0.0066**	**0.006**	
1.07	0.07	**0.007**						**0.0068**	**0.007**	
1.08	0.08	**0.010**						**0.0071**	**0.010**	
1.09	0.09	**0.012**						**0.0074**	**0.012**	
1.10	0.10	**0.015**						**0.0076**	**0.015**	
1.11	0.11	**0.017**						**0.0079**	**0.017**	
1.12	0.12	**0.020**						**0.0082**	**0.020**	<= 2 Yr: 81 m³ (1.12m)
1.13	0.13	**0.023**						**0.0085**	**0.023**	
1.14	0.14	**0.027**						**0.0088**	**0.027**	
1.15	0.15	**0.030**						**0.0091**	**0.030**	
1.16	0.16	**0.033**						**0.0094**	**0.033**	
1.17	0.17	**0.037**						**0.0097**	**0.037**	
1.18	0.18	**0.040**						**0.0101**	**0.040**	
1.19	0.19	**0.044**						**0.0104**	**0.044**	
1.20	0.20	**0.047**						**0.0108**	**0.047**	<= 5 Yr: 105 m³ (1.2m)
1.21	0.21	**0.051**						**0.0111**	**0.051**	
1.22	0.22	**0.054**						**0.0115**	**0.054**	
1.23	0.23	**0.057**						**0.0118**	**0.057**	
1.24	0.24	**0.059**						**0.0122**	**0.059**	
1.25	0.25	**0.061**						**0.0126**	**0.061**	<= 10 Yr: 122 m³ (1.25m)
1.26	0.26	**0.064**						**0.0130**	**0.064**	
1.27	0.27	**0.066**						**0.0133**	**0.066**	
1.28	0.28	**0.068**						**0.0137**	**0.068**	
1.29	0.29	**0.071**						**0.0141**	**0.071**	
1.30	0.30	**0.073**						**0.0146**	**0.073**	<= 25 Yr: 144 m³ (1.3m)
1.31	0.31	**0.075**						**0.0150**	**0.075**	
1.32	0.32	**0.077**						**0.0154**	**0.077**	
1.33	0.33	**0.079**						**0.0158**	**0.079**	
1.34	0.34	**0.081**						**0.0163**	**0.081**	
1.35	0.35	**0.083**						**0.0167**	**0.083**	
1.36	0.36	**0.084**						**0.0172**	**0.084**	
1.37	0.37	**0.086**						**0.0176**	**0.086**	<= 50 Yr: 174 m³ (1.37m)
1.38	0.38	**0.088**						**0.0181**	**0.088**	
1.39	0.39	**0.090**						**0.0186**	**0.090**	
1.40	0.40	**0.091**						**0.0190**	**0.091**	
1.41	0.41	**0.093**						**0.0195**	**0.093**	
1.42	0.42	**0.094**						**0.0200**	**0.094**	<= 100 Yr: 200 m³ (1.42m)
1.43	0.43	**0.096**						**0.0205**	**0.096**	
1.44	0.44	**0.098**						**0.0210**	**0.098**	
1.45	0.45	**0.099**						**0.0215**	**0.099**	
1.46	0.46	**0.101**						**0.0221**	**0.101**	
1.47	0.47	**0.102**						**0.0226**	**0.102**	
1.48	0.48	**0.104**						**0.0231**	**0.104**	

Stage-Storage-Discharge Summary Table										
Elevation	Stage	Stage 1 Orifice Tube						Total Storage	Total Discharge	Notes
m	m	m^3/s						ha*m	m^3/s	
1.49	0.49	**0.105**						**0.0237**	**0.105**	
1.50	0.50	**0.107**						**0.0242**	**0.107**	
1.51	0.51	**0.108**						**0.0248**	**0.108**	
1.52	0.52	**0.109**						**0.0253**	**0.109**	
1.53	0.53	**0.111**						**0.0259**	**0.111**	
1.54	0.54	**0.112**						**0.0265**	**0.112**	
1.55	0.55	**0.113**						**0.0270**	**0.113**	
1.56	0.56	**0.115**						**0.0276**	**0.115**	
1.57	0.57	**0.116**						**0.0282**	**0.116**	
1.58	0.58	**0.117**						**0.0288**	**0.117**	
1.59	0.59	**0.119**						**0.0294**	**0.119**	
1.60	0.60	**0.120**						**0.0301**	**0.120**	

Appendix D

Drawings

LIFE AT THE WOODLAND SUBDIVISION

OTONABEE-SOUTH MONAGHAN

TOWNSHIP OF OTONABEE-SOUTH MONAGHAN, ONTARIO

WILLS PROJECT No. 19-10874

DRAWING INDEX

SHEET No.	DRAWING	REVISION No.	REVISION DATE
100	DRAFT PLAN	2	03/14/2023
101	PRELIMINARY DETAILS	2	03/14/2023
400	PRELIMINARY PLAN PROFILE STA: 0+000 – 0+060	2	03/14/2023
401	PRELIMINARY PLAN PROFILE STA: 0+060 – 0+140	2	03/14/2023
402	PRELIMINARY PLAN PROFILE STA: 0+140 – 0+300	2	03/14/2023
403	PRELIMINARY PLAN PROFILE STA: 0+300 – 0+460	2	03/14/2023
404	PRELIMINARY PLAN PROFILE STA: 0+460 – 0+560	2	03/14/2023
405	PRELIMINARY PLAN PROFILE STA: 0+560 – 0+680	2	03/14/2023
406	PRELIMINARY PLAN PROFILE STA: 0+680 – 0+760	2	03/14/2023
407	PRELIMINARY PLAN PROFILE STA: 0+760 – 0+920	2	03/14/2023
408	PRELIMINARY PLAN PROFILE STA: 0+920 – 1+040	2	03/14/2023
409	PRELIMINARY PLAN PROFILE STA: 1+040 – 1+171	2	03/14/2023

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Peterborough, Ontario
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E. wills@dmwills.com

FSR SUBMISSION – FEBRUARY 24, 2022
ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION – MARCH 14, 2023

LIFE AT THE WOODLAND SUBDIVISION
TOWNSHIP OF OTONABEE-SOUTH MONAGHAN, ONTARIO
WILLS PROJECT No. 19-10874
FSR SUBMISSION - FEBRUARY 24, 2022

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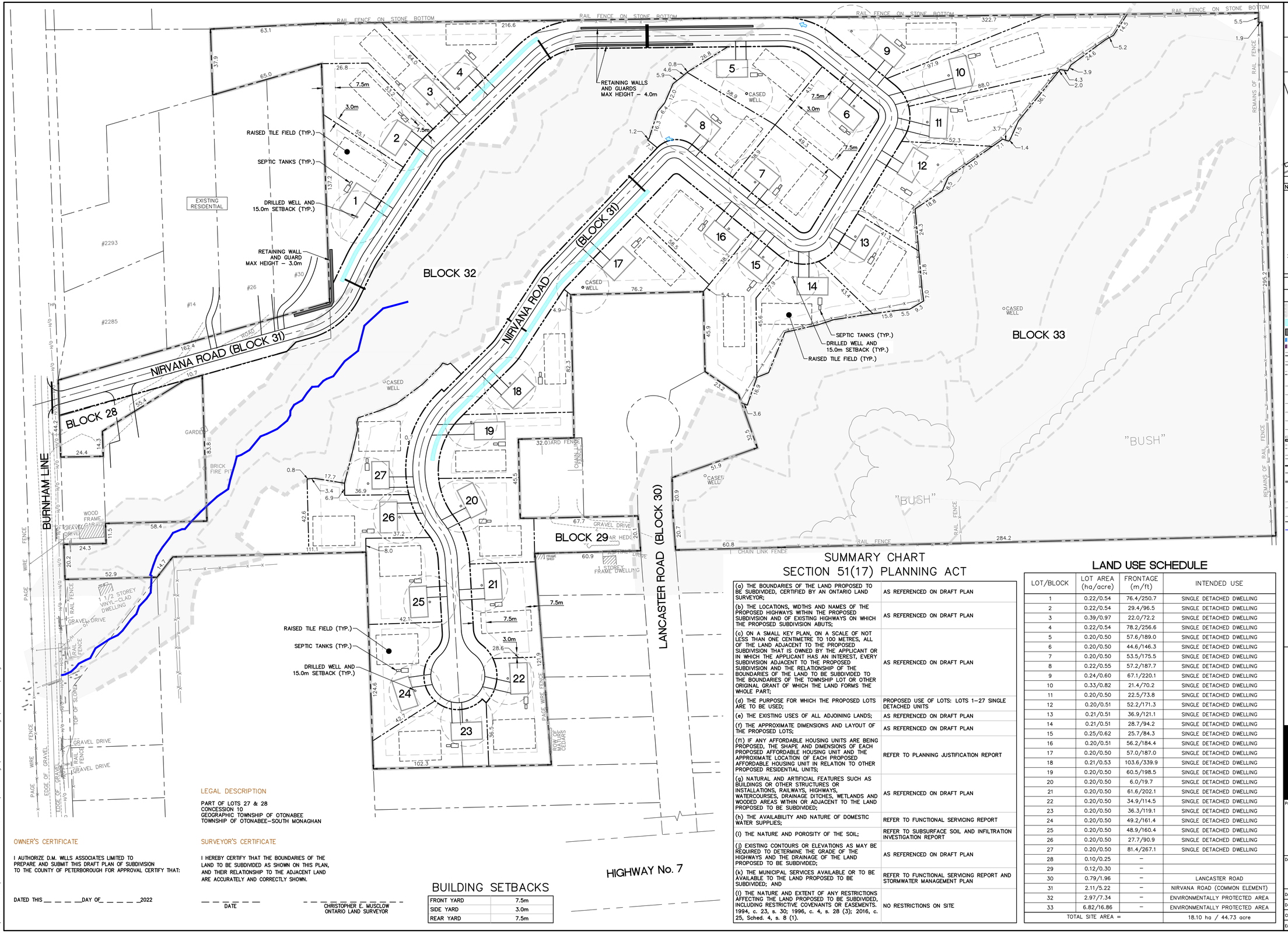

LEGAL DESCRIPTION

PART OF LOTS 27 & 28
CONCESSION 10
GEOGRAPHIC TOWNSHIP OF OTONABEE
TOWNSHIP OF OTONABEE–SOUTH MONAGHAN

OWNER'S CERTIFICATE

I AUTHORIZE D.M. WILLS ASSOCIATES LIMITED TO PREPARE AND SUBMIT THIS DRAFT PLAN OF SUBDIVISION TO THE COUNTY OF PETERBOROUGH FOR APPROVAL CERTIFY THAT:

DATED THIS_____DAY OF_____2022

SURVEYOR'S CERTIFICATE

I HEREBY CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED AS SHOWN ON THIS PLAN, AND THEIR RELATIONSHIP TO THE ADJACENT LAND ARE ACCURATELY AND CORRECTLY SHOWN.

_____ DATE

_____ CHRISTOPHER E. MUSCLOW ONTARIO LAND SURVEYOR

BUILDING SETBACKS

FRONT YARD	7.5m
SIDE YARD	3.0m
REAR YARD	7.5m

SUMMARY CHART
SECTION 51(17) PLANNING ACT

(a) THE BOUNDARIES OF THE LAND PROPOSED TO BE SUBDIVIDED, CERTIFIED BY AN ONTARIO LAND SURVEYOR;	AS REFERENCED ON DRAFT PLAN
(b) THE LOCATIONS, WIDTHS AND NAMES OF THE PROPOSED HIGHWAYS WITHIN THE PROPOSED SUBDIVISION AND OF EXISTING HIGHWAYS ON WHICH THE PROPOSED SUBDIVISION ABUTS;	AS REFERENCED ON DRAFT PLAN
(c) ON A SMALL KEY PLAN, ON A SCALE OF NOT LESS THAN ONE CENTIMETRE TO 100 METRES, ALL OF THE LAND ADJACENT TO THE PROPOSED SUBDIVISION THAT IS OWNED BY THE APPLICANT OR IN WHICH THE APPLICANT HAS AN INTEREST, EVERY SUBDIVISION ADJACENT TO THE PROPOSED SUBDIVISION AND THE RELATIONSHIP OF THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED TO THE BOUNDARIES OF THE TOWNSHIP LOT OR OTHER ORIGINAL GRANT OF WHICH THE LAND FORMS THE WHOLE PART;	AS REFERENCED ON DRAFT PLAN
(d) THE PURPOSE FOR WHICH THE PROPOSED LOTS ARE TO BE USED;	PROPOSED USE OF LOTS: LOTS 1–27 SINGLE DETACHED UNITS
(e) THE EXISTING USES OF ALL ADJOINING LANDS;	AS REFERENCED ON DRAFT PLAN
(f) THE APPROXIMATE DIMENSIONS AND LAYOUT OF THE PROPOSED LOTS;	AS REFERENCED ON DRAFT PLAN
(f1) IF ANY AFFORDABLE HOUSING UNITS ARE BEING PROPOSED, THE SHAPE AND DIMENSIONS OF EACH PROPOSED AFFORDABLE HOUSING UNIT AND THE APPROXIMATE LOCATION OF EACH PROPOSED AFFORDABLE HOUSING UNIT IN RELATION TO OTHER PROPOSED RESIDENTIAL UNITS;	REFER TO PLANNING JUSTIFICATION REPORT
(g) NATURAL AND ARTIFICIAL FEATURES SUCH AS BUILDINGS OR OTHER STRUCTURES OR INSTALLATIONS, RAILWAYS, HIGHWAYS, WATERCOURSES, DRAINAGE DITCHES, WETLANDS AND WOODED AREAS WITHIN OR ADJACENT TO THE LAND PROPOSED TO BE SUBDIVIDED;	AS REFERENCED ON DRAFT PLAN
(h) THE AVAILABILITY AND NATURE OF DOMESTIC WATER SUPPLIES;	REFER TO FUNCTIONAL SERVICING REPORT
(i) THE NATURE AND POROSITY OF THE SOIL;	REFER TO SUBSURFACE SOIL AND INFILTRATION INVESTIGATION REPORT
(j) EXISTING CONTOURS OR ELEVATIONS AS MAY BE REQUIRED TO DETERMINE THE GRADE OF THE HIGHWAYS AND THE DRAINAGE OF THE LAND PROPOSED TO BE SUBDIVIDED;	AS REFERENCED ON DRAFT PLAN
(k) THE MUNICIPAL SERVICES AVAILABLE OR TO BE AVAILABLE TO THE LAND PROPOSED TO BE SUBDIVIDED; AND	REFER TO FUNCTIONAL SERVICING REPORT AND STORMWATER MANAGEMENT PLAN
(l) THE NATURE AND EXTENT OF ANY RESTRICTIONS AFFECTING THE LAND PROPOSED TO BE SUBDIVIDED, INCLUDING RESTRICTIVE COVENANTS OR EASEMENTS. 1994, c. 23, s. 30; 1996, c. 4, s. 28 (3); 2016, c. 25, Sched. 4, s. 8 (1).	NO RESTRICTIONS ON SITE

LAND USE SCHEDULE

LOT/BLOCK	LOT AREA (ha/acre)	FRONTAGE (m/ft)	INTENDED USE
1	0.22/0.54	76.4/250.7	SINGLE DETACHED DWELLING
2	0.22/0.54	29.4/96.5	SINGLE DETACHED DWELLING
3	0.39/0.97	22.0/72.2	SINGLE DETACHED DWELLING
4	0.22/0.54	78.2/256.6	SINGLE DETACHED DWELLING
5	0.20/0.50	57.6/189.0	SINGLE DETACHED DWELLING
6	0.20/0.50	44.6/146.3	SINGLE DETACHED DWELLING
7	0.20/0.50	53.5/175.5	SINGLE DETACHED DWELLING
8	0.22/0.55	57.2/187.7	SINGLE DETACHED DWELLING
9	0.24/0.60	67.1/220.1	SINGLE DETACHED DWELLING
10	0.33/0.82	21.4/70.2	SINGLE DETACHED DWELLING
11	0.20/0.50	22.5/73.8	SINGLE DETACHED DWELLING
12	0.20/0.51	52.2/171.3	SINGLE DETACHED DWELLING
13	0.21/0.51	36.9/121.1	SINGLE DETACHED DWELLING
14	0.21/0.51	28.7/94.2	SINGLE DETACHED DWELLING
15	0.25/0.62	25.7/84.3	SINGLE DETACHED DWELLING
16	0.20/0.51	56.2/184.4	SINGLE DETACHED DWELLING
17	0.20/0.50	57.0/187.0	SINGLE DETACHED DWELLING
18	0.21/0.53	103.6/339.9	SINGLE DETACHED DWELLING
19	0.20/0.50	60.5/198.5	SINGLE DETACHED DWELLING
20	0.20/0.50	6.0/19.7	SINGLE DETACHED DWELLING
21	0.20/0.50	61.6/202.1	SINGLE DETACHED DWELLING
22	0.20/0.50	34.9/114.5	SINGLE DETACHED DWELLING
23	0.20/0.50	36.3/119.1	SINGLE DETACHED DWELLING
24	0.20/0.50	49.2/161.4	SINGLE DETACHED DWELLING
25	0.20/0.50	48.9/160.4	SINGLE DETACHED DWELLING
26	0.20/0.50	27.7/90.9	SINGLE DETACHED DWELLING
27	0.20/0.50	81.4/267.1	SINGLE DETACHED DWELLING
28	0.10/0.25	–	
29	0.12/0.30	–	
30	0.79/1.96	–	LANCASTER ROAD
31	2.11/5.22	–	NIRVANA ROAD (COMMON ELEMENT)
32	2.97/7.34	–	ENVIRONMENTALLY PROTECTED AREA
33	6.82/16.86	–	ENVIRONMENTALLY PROTECTED AREA
TOTAL SITE AREA =			18.10 ha / 44.73 acre

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC	Dimensions are in METRES and/or MILLIMETRES unless otherwise shown
LEGEND	TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

EXISTING ENVIRONMENTAL PROTECTION AREA
INFILTRATION FACILITY
RETAINING WALL
EX. FLOOD LINE
PR. FLOOD LINE
EX./PR. EDGE OF PAVEMENT
EX./PR. ROAD CENTERLINE
EX./PR. EDGE OF SHOULDER/GRAVEL
EXISTING GASMAIN
EX. OVERHEAD BELL
EX. UNDERGROUND BELL
EX. OVERHEAD HYDRO
EX. UNDERGROUND HYDRO
EX. UNDERGROUND CABLE
EX./PR. CULVERT
EX./PR. DITCH
EX./PR. FENCE
PROPERTY LINE
LOT LINE
DEVELOPMENT BOUNDARY
EXISTING VEGETATION
100.0 EX./PR. CONTOUR
EX./PR. GUIDERAIL
EXISTING EDGE OF WATER/STREAM
HP EXISTING HYDRO POLE
EX./PR. SIGN

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Project Name/Location

LIFE AT THE WOODLAND

OTONABEE–SOUTH MONAGHAN

Drawing Title

DRAFT PLAN

Drawn By: K.W.P.	SCALE: Horz. 1:1000	Vert.	
Designed By: K.W.P.	Plot Date: 03/16/2023		
Checked By: J.D.F.	Project No.: 19–10874	Sht. No.:	
Engineer: D.A.K.	Dwg File No.: 10874 – MP	400	

Printed By: srobinson Printed On: March 16, 2023
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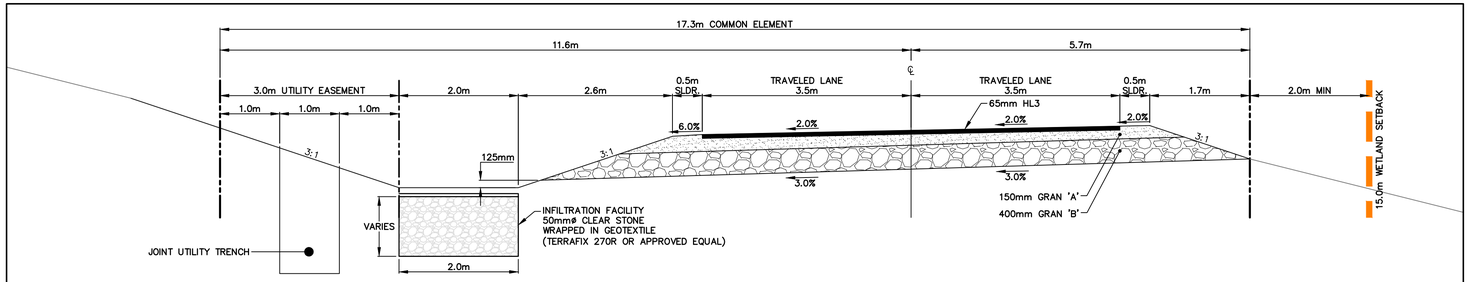

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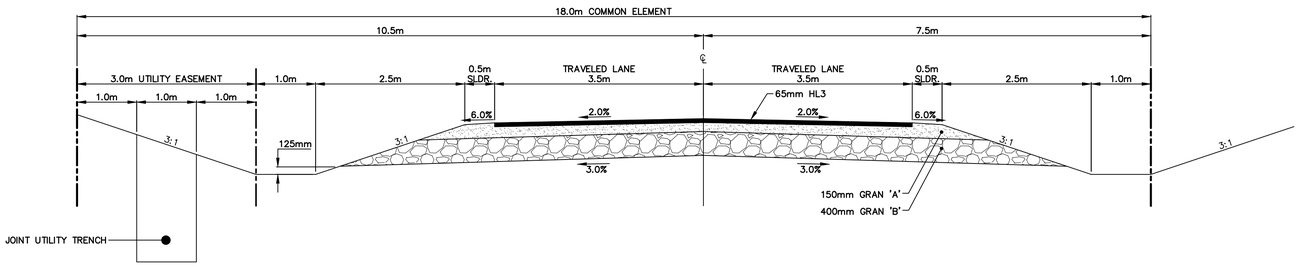
0+140 – 0+352, 0+757 – 0+950
SCALE = 1:50



901 TYPICAL SECTION
0+006 – 0+140, 0+352 – 0+757, 0+950 – 1+093
SCALE = 1:50

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC	Dimensions are in METRES and/or MILLIMETRES unless otherwise shown
LEGEND	TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

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Project Name/Location

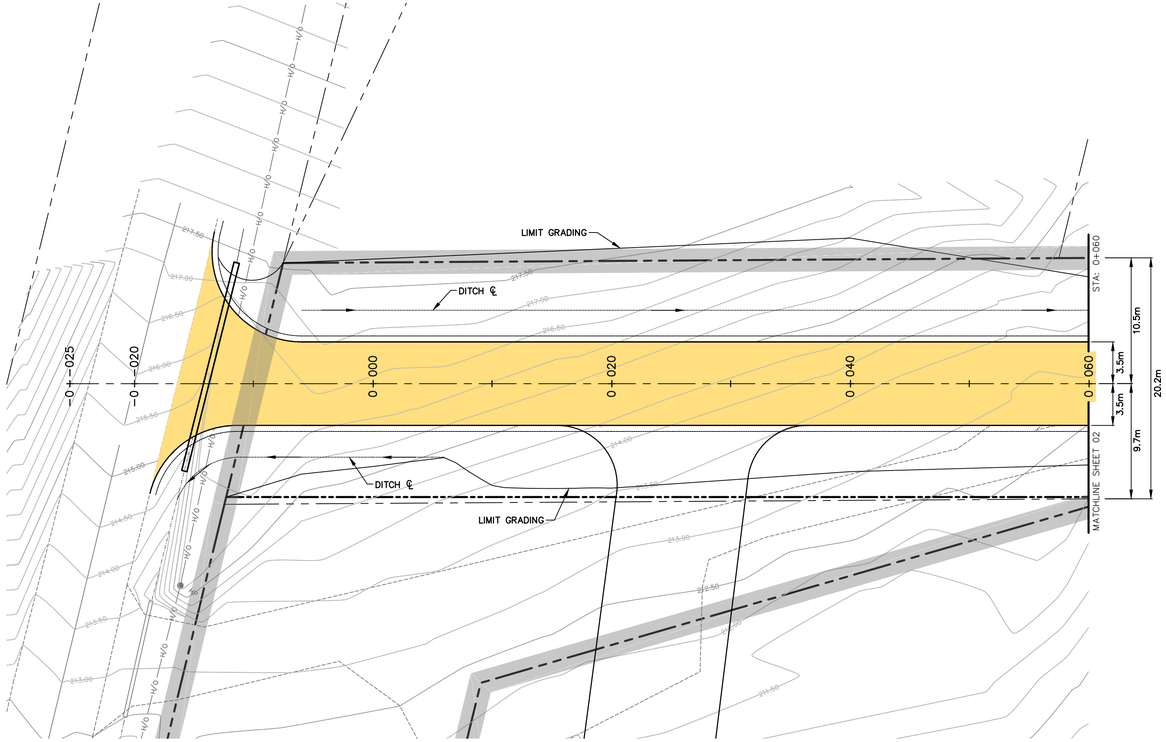
LIFE AT THE WOODLAND

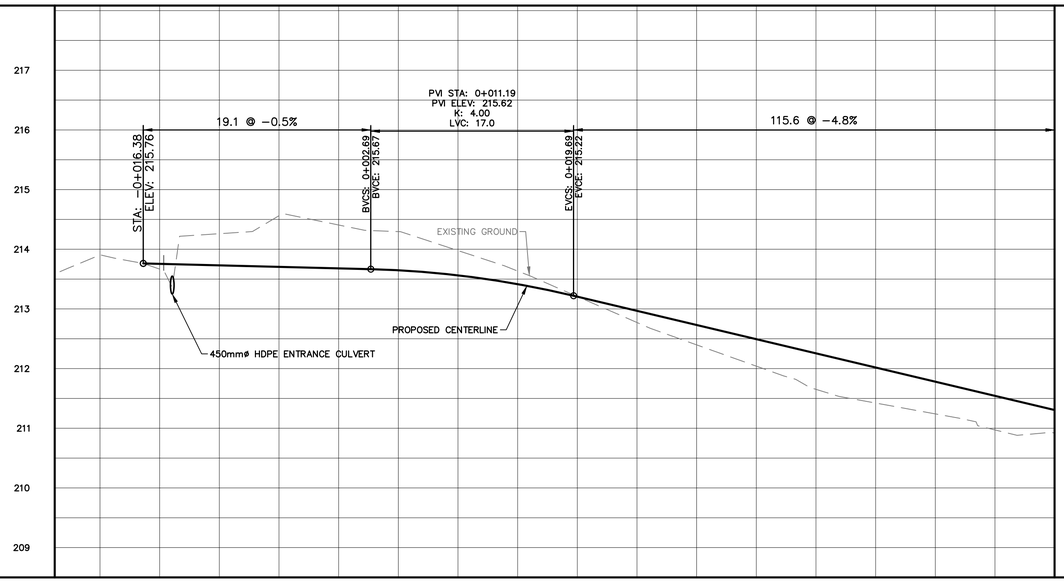
OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY DETAILS

Drawn By: K.W.P.	SCALE: Horz. Vert.	
Designed By: K.W.P.	Plot Date: 02/24/2022	
Checked By: J.D.F.	Project No.: 19-10874	Sht. No.:
Engineer: D.A.K.	Dwg File No.: 10874 – GNDT	100





℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS	−0+020	0+000	0+020	0+040	0+060
	215.90	215.68 216.41	215.21 215.21	214.26 213.65	213.31 212.92

℄ – DITCH (R)

℄ – DITCH (L)

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC Dimensions are in METRES and/or MILLIMETRES unless otherwise shown

LEGEND TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

- PROPOSED ASPHALT
- EXISTING WETLAND AREA
- EXISTING 15.0m SETBACK AND BUFFER AREA
- EXISTING 30.0m SETBACK AND BUFFER AREA
- WETLAND ENCROACHMENT
- INFILTRATION FACILITY
- RETAINING WALL
- EX. FLOOD LINE / PR. FLOOD LINE
- EX./PR. EDGE OF PAVEMENT
- EX./PR. ROAD CENTERLINE
- EX./PR. EDGE OF SHOULDER/GRAVEL
- EXISTING GASMAIN
- EX. OVERHEAD BELL
- EX. UNDERGROUND BELL
- EX. OVERHEAD HYDRO
- EX. UNDERGROUND HYDRO
- EX. UNDERGROUND CABLE
- EX./PR. CULVERT
- EX./PR. DITCH
- EX./PR. FENCE
- PROPERTY LINE
- LOT LINE
- DEVELOPMENT BOUNDARY
- EXISTING VEGETATION
- 100.0 EX./PR. CONTOUR
- EX./PR. GUIDERAIL
- EXISTING EDGE OF WATER/STREAM
- HP EXISTING HYDRO POLE
- EX./PR. SIGN
- x215.00 x214.00 EX./PR. ELEVATION
- 1.0% PROPOSED GRADE

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Project Name/Location

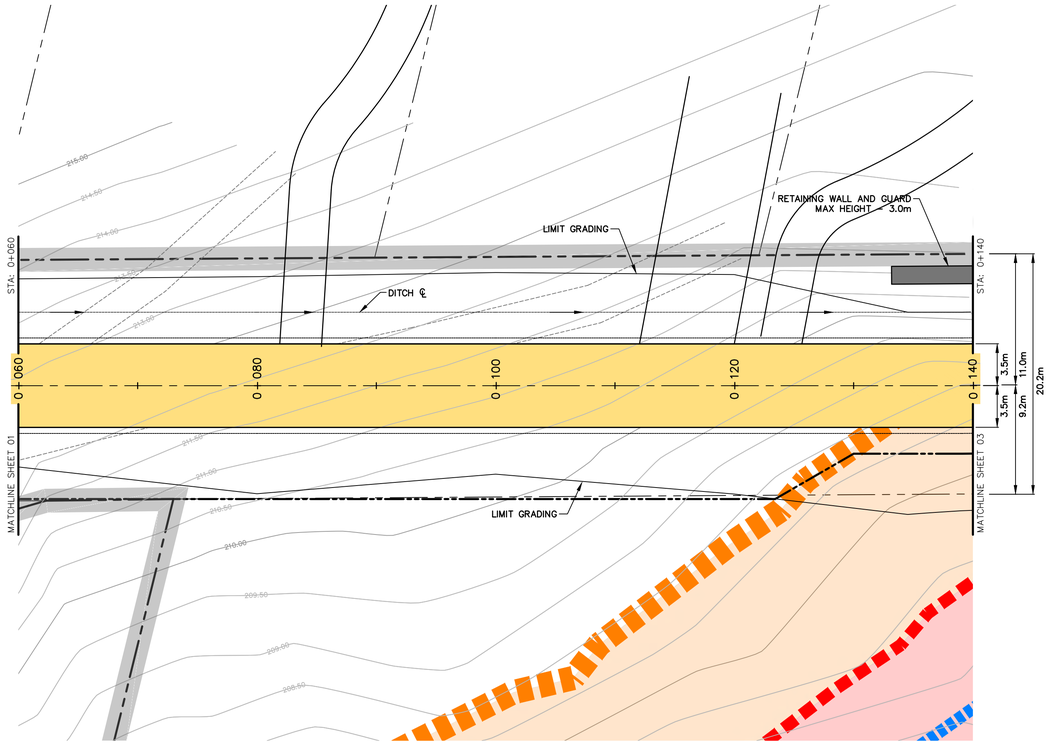
LIFE AT THE WOODLAND

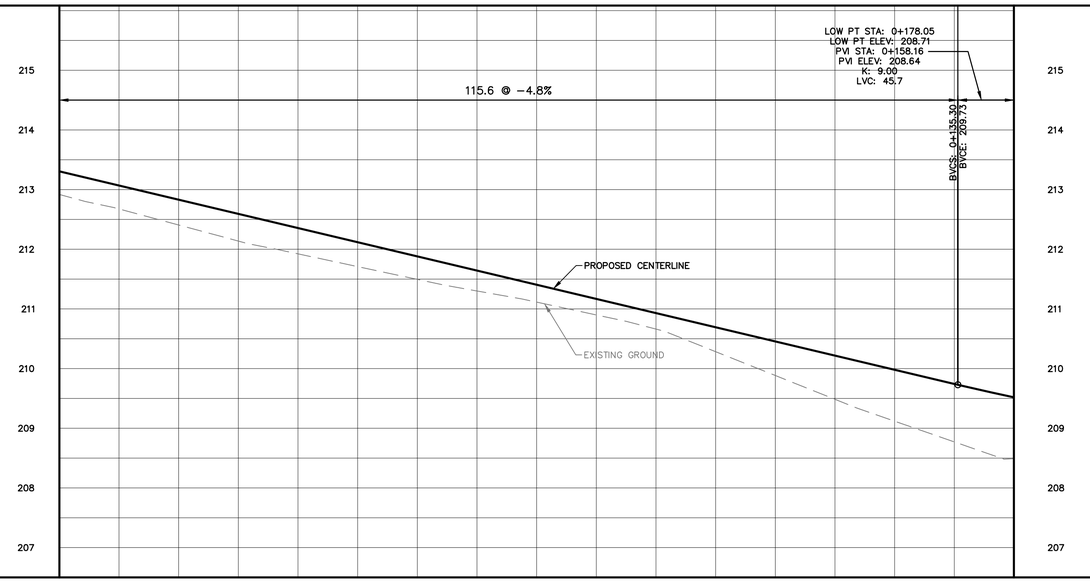
OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY PLAN AND PROFILE STA 0+000 TO 0+060

Drawn By: K.W.P.	SCALE: Horz. 1:250	Vert. 1:50	
Designed By: K.W.P.	Plot Date: 02/24/2022		
Checked By: J.D.F.	Project No.: 19−10874	Sht. No.:	
Engineer: D.A.K.	Dwg File No.: 10874 − PP	400	





℄ – DITCH (R)					℄ – DITCH (R)
℄ – DITCH (L)					℄ – DITCH (L)
CHAINAGE & ELEVATIONS	0+060 213.31	0+080 212.36	0+100 211.41	0+120 210.46	0+140 209.52	CHAINAGE & ELEVATIONS

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC Dimensions are in METRES and/or MILLIMETRES unless otherwise shown

LEGEND TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

- PROPOSED ASPHALT
- EXISTING WETLAND AREA
- EXISTING 15.0m SETBACK AND BUFFER AREA
- EXISTING 30.0m SETBACK AND BUFFER AREA
- WETLAND ENCROACHMENT
- INFILTRATION FACILITY
- RETAINING WALL
- EX. FLOOD LINE
- PR. FLOOD LINE
- EX./PR. EDGE OF PAVEMENT
- EX./PR. ROAD CENTERLINE
- EX./PR. EDGE OF SHOULDER/GRAVEL
- EXISTING GASMAIN
- EX. OVERHEAD BELL
- EX. UNDERGROUND BELL
- EX. OVERHEAD HYDRO
- EX. UNDERGROUND HYDRO
- EX. UNDERGROUND CABLE
- EX./PR. CULVERT
- EX./PR. DITCH
- EX./PR. FENCE
- PROPERTY LINE
- LOT LINE
- DEVELOPMENT BOUNDARY
- EXISTING VEGETATION
- 100.0 100.0 EX./PR. CONTOUR
- EX./PR. GUIDERAIL
- EXISTING EDGE OF WATER/STREAM
- HP EXISTING HYDRO POLE
- EX./PR. SIGN
- x215.00 x214.00 EX./PR. ELEVATION
- 1.0% PROPOSED GRADE

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Project Name/Location

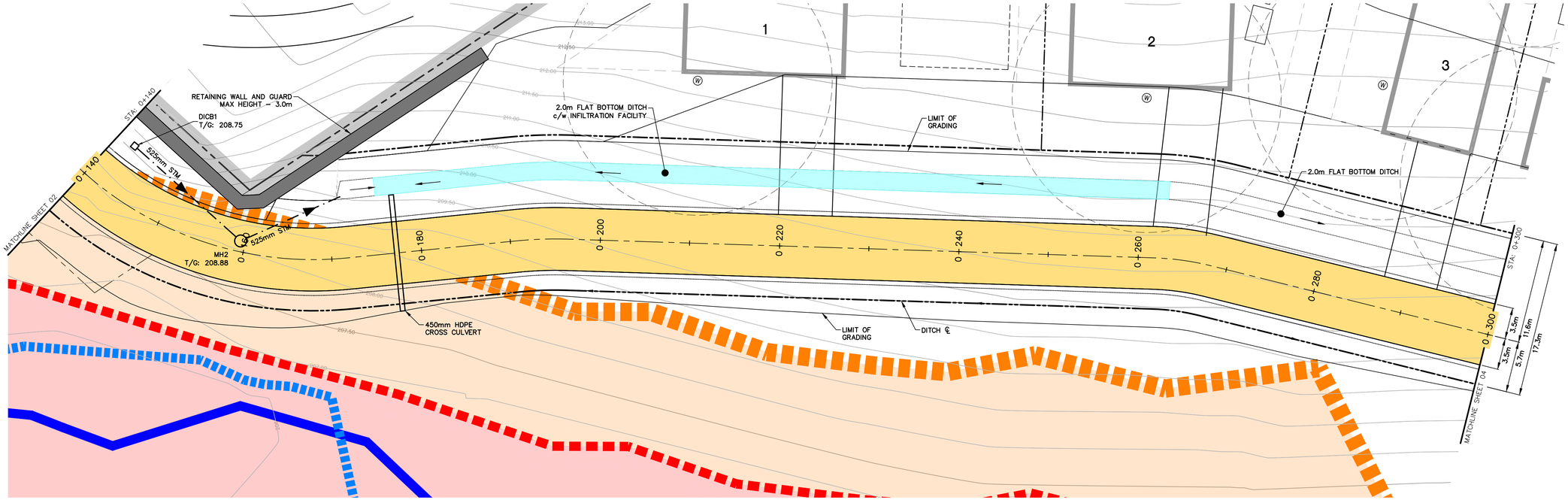
LIFE AT THE WOODLAND

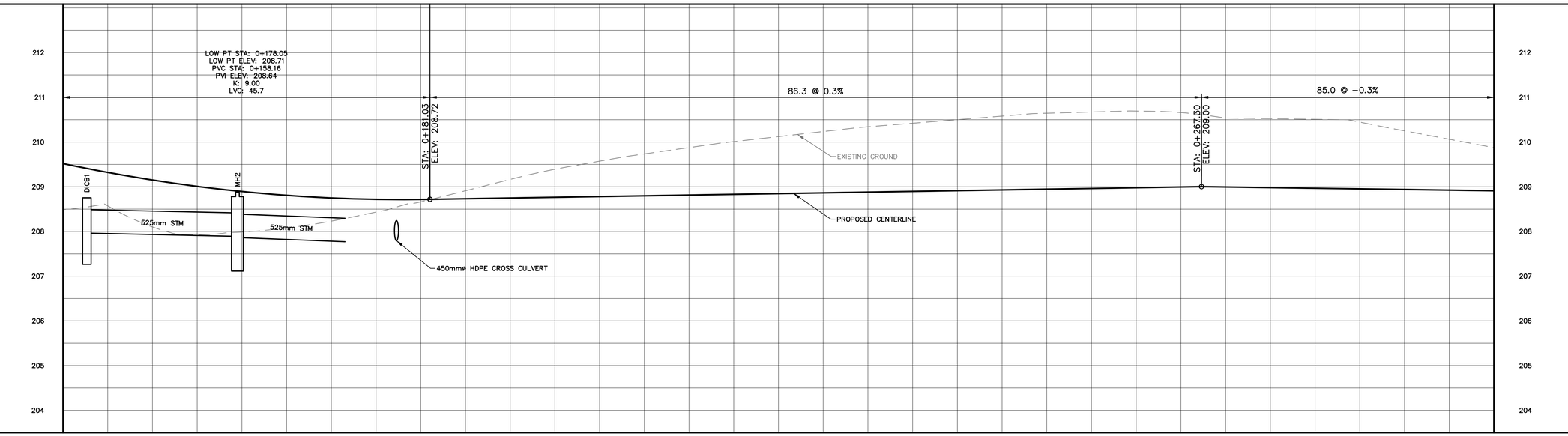
OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY PLAN AND PROFILE STA 0+000 TO 0+140

Drawn By: K.W.P.	SCALE: Horz. 1:250	Vert. 1:50	
Designed By: K.W.P.	Plot Date: 02/24/2022		
Checked By: J.D.F.	Project No.: 19-10874	Sht. No.:	
Engineer: D.A.K.	Dwg File No.: 10874 – PP	401	





℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS	0+140 209.52 208.04	0+160 208.89 208.09	0+180 208.72 208.10	0+200 208.78 208.10	0+220 208.85 208.10	0+240 208.91 208.10	0+260 208.98 208.10	0+280 208.97 208.10	0+300 208.91 208.10	CHAINAGE & ELEVATIONS

℄ – DITCH (R)

℄ – DITCH (L)

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC Dimensions are in METRES and/or MILLIMETRES unless otherwise shown

LEGEND TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

- PROPOSED ASPHALT
- EXISTING WETLAND AREA
- EXISTING 15.0m SETBACK AND BUFFER AREA
- EXISTING 30.0m SETBACK AND BUFFER AREA
- WETLAND ENCROACHMENT
- INFILTRATION FACILITY
- RETAINING WALL
- EX. FLOOD LINE / PR. FLOOD LINE
- EX./PR. EDGE OF PAVEMENT
- EX./PR. ROAD CENTERLINE
- EX./PR. EDGE OF SHOULDER/GRAVEL
- EXISTING GASMAIN
- EX. OVERHEAD BELL
- EX. UNDERGROUND BELL
- EX. OVERHEAD HYDRO
- EX. UNDERGROUND HYDRO
- EX. UNDERGROUND CABLE
- EX./PR. CULVERT
- EX./PR. DITCH
- EX./PR. FENCE
- PROPERTY LINE
- LOT LINE
- DEVELOPMENT BOUNDARY
- EXISTING VEGETATION
- EX./PR. CONTOUR (100.0 / 100.0)
- EX./PR. GUIDERAIL
- EXISTING EDGE OF WATER/STREAM
- HP EXISTING HYDRO POLE
- EX./PR. SIGN
- x215.00 x214.00 EX./PR. ELEVATION
- 1.0% PROPOSED GRADE

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Project Name/Location

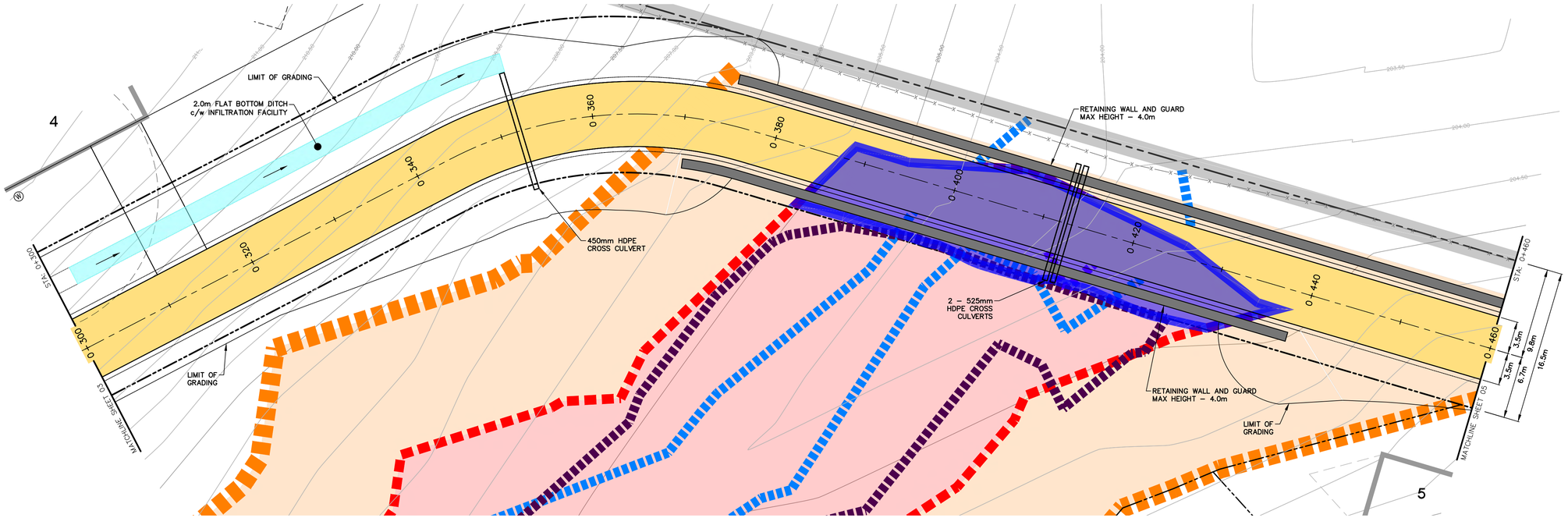
LIFE AT THE WOODLAND

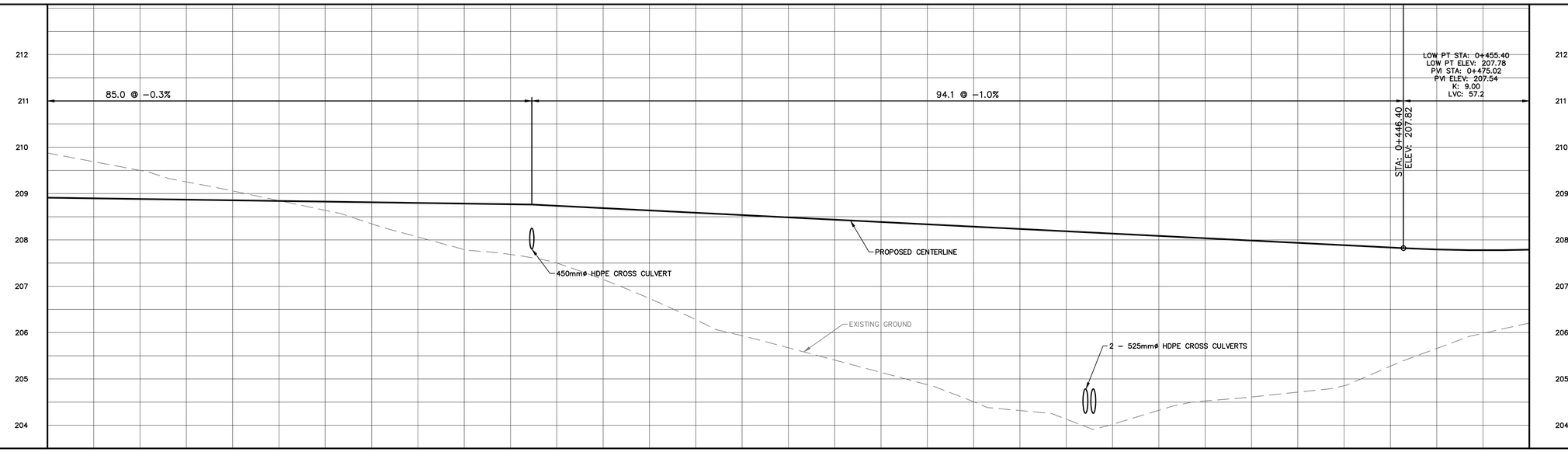
OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY PLAN AND PROFILE STA 0+140 TO 0+300

Drawn By: K.W.P.	SCALE: Horz. 1:250	Vert. 1:50	
Designed By: K.W.P.	Plot Date: 02/24/2022		
Checked By: J.D.F.	Project No.: 19–10874	Sht. No.:	
Engineer: D.A.K.	Dwg File No.: 10874 – PP	402	





℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS	0+300 208.81	0+320 208.45	0+340 208.60	0+360 208.69	0+380 208.49	0+400 208.23	0+420 208.09	0+440 207.89	0+460 207.79	CHAINAGE & ELEVATIONS

℄ – DITCH (R)

℄ – DITCH (L)

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC Dimensions are in METRES and/or MILLIMETRES unless otherwise shown

LEGEND TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

- PROPOSED ASPHALT
- EXISTING WETLAND AREA
- EXISTING 15.0m SETBACK AND BUFFER AREA
- EXISTING 30.0m SETBACK AND BUFFER AREA
- WETLAND ENCROACHMENT
- INFILTRATION FACILITY
- RETAINING WALL
- EX. FLOOD LINE
- PR. FLOOD LINE
- EX./PR. EDGE OF PAVEMENT
- EX./PR. ROAD CENTERLINE
- EX./PR. EDGE OF SHOULDER/GRAVEL
- EXISTING GASMAIN
- EX. OVERHEAD BELL
- EX. UNDERGROUND BELL
- EX. OVERHEAD HYDRO
- EX. UNDERGROUND HYDRO
- EX. UNDERGROUND CABLE
- EX./PR. CULVERT
- EX./PR. DITCH
- EX./PR. FENCE
- PROPERTY LINE
- LOT LINE
- DEVELOPMENT BOUNDARY
- EXISTING VEGETATION
- 100.0 EX./PR. CONTOUR
- EX./PR. GUIDERAIL
- EXISTING EDGE OF WATER/STREAM
- HP EXISTING HYDRO POLE
- EX./PR. SIGN
- x215.00 x214.00 EX./PR. ELEVATION
- 1.0% PROPOSED GRADE

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Project Name/Location

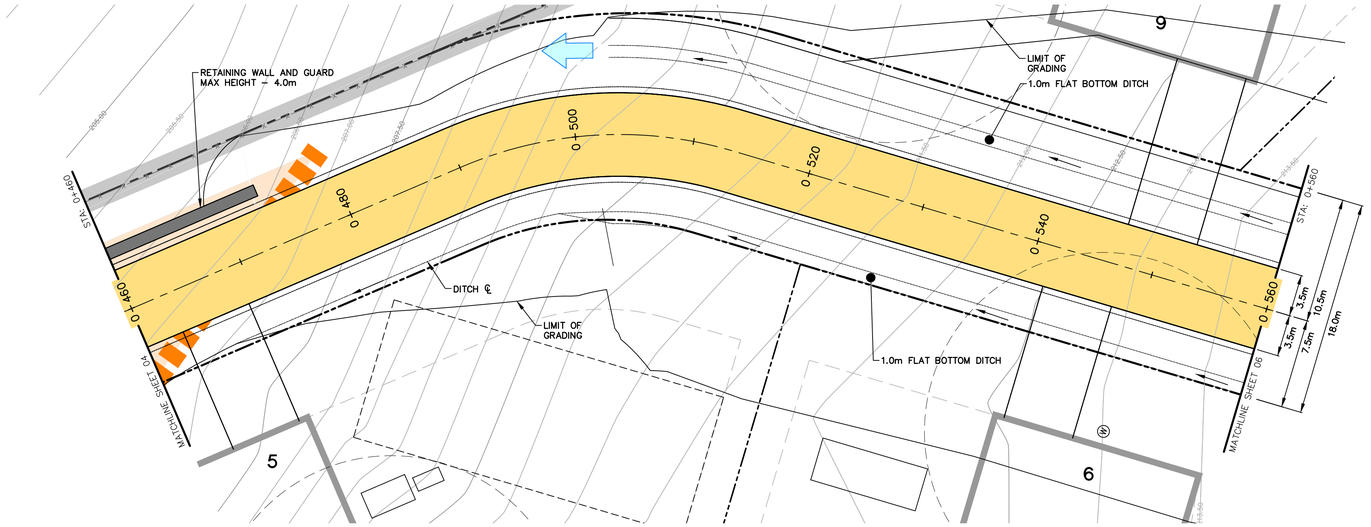
LIFE AT THE WOODLAND

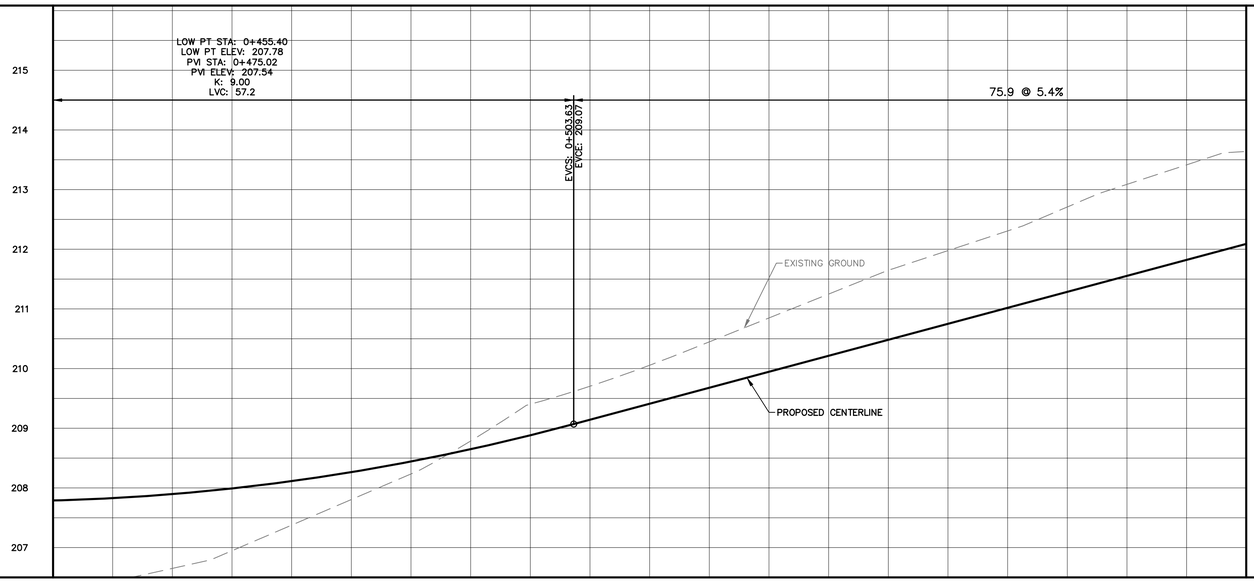
OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY PLAN AND PROFILE STA 0+300 TO 0+460

Drawn By: K.W.P.	SCALE: Horz. 1:250	Vert. 1:50	
Designed By: K.W.P.	Plot Date: 02/24/2022		
Checked By: J.D.F.	Project No.: 19–10874	Sht. No.:	
Engineer: D.A.K.	Dwg File No.: 10874 – PP	403	





℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS	0+460 207.79 206.21	0+480 208.11 207.38	0+500 208.68 209.40	0+520 209.95 210.85	0+540 211.02 212.31	0+560 212.09 213.64

℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC — Dimensions are in METRES and/or MILLIMETRES unless otherwise shown

LEGEND — TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

- PROPOSED ASPHALT
- EXISTING WETLAND AREA
- EXISTING 15.0m SETBACK AND BUFFER AREA
- EXISTING 30.0m SETBACK AND BUFFER AREA
- WETLAND ENCROACHMENT
- INFILTRATION FACILITY
- RETAINING WALL
- EX. FLOOD LINE
- PR. FLOOD LINE
- EX./PR. EDGE OF PAVEMENT
- EX./PR. ROAD CENTERLINE
- EX./PR. EDGE OF SHOULDER/GRAVEL
- EXISTING GASMAIN
- EX. OVERHEAD BELL
- EX. UNDERGROUND BELL
- EX. OVERHEAD HYDRO
- EX. UNDERGROUND HYDRO
- EX. UNDERGROUND CABLE
- EX./PR. CULVERT
- EX./PR. DITCH
- EX./PR. FENCE
- PROPERTY LINE
- LOT LINE
- DEVELOPMENT BOUNDARY
- EXISTING VEGETATION
- 100.0 / 100.0 — EX./PR. CONTOUR
- EX./PR. GUIDERAIL
- EXISTING EDGE OF WATER/STREAM
- HP — EXISTING HYDRO POLE
- EX./PR. SIGN
- x215.00 x214.00 — EX./PR. ELEVATION
- 1.0% — PROPOSED GRADE

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Project Name/Location

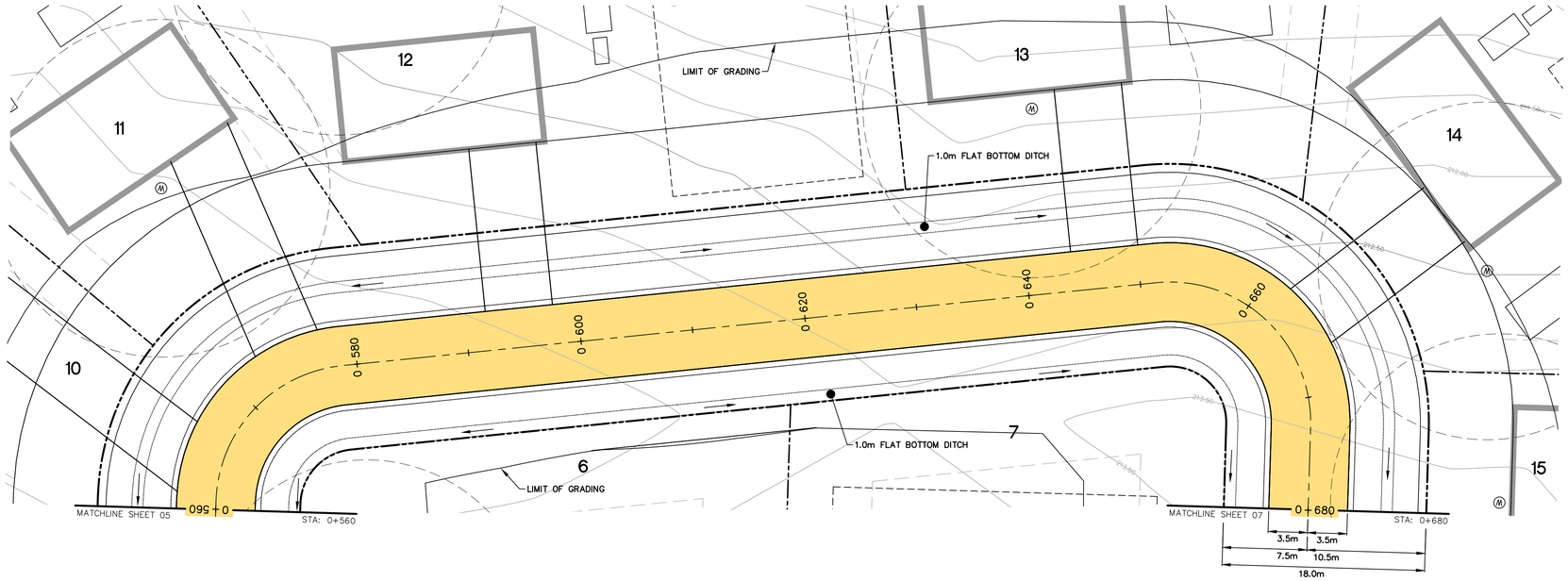
LIFE AT THE WOODLAND

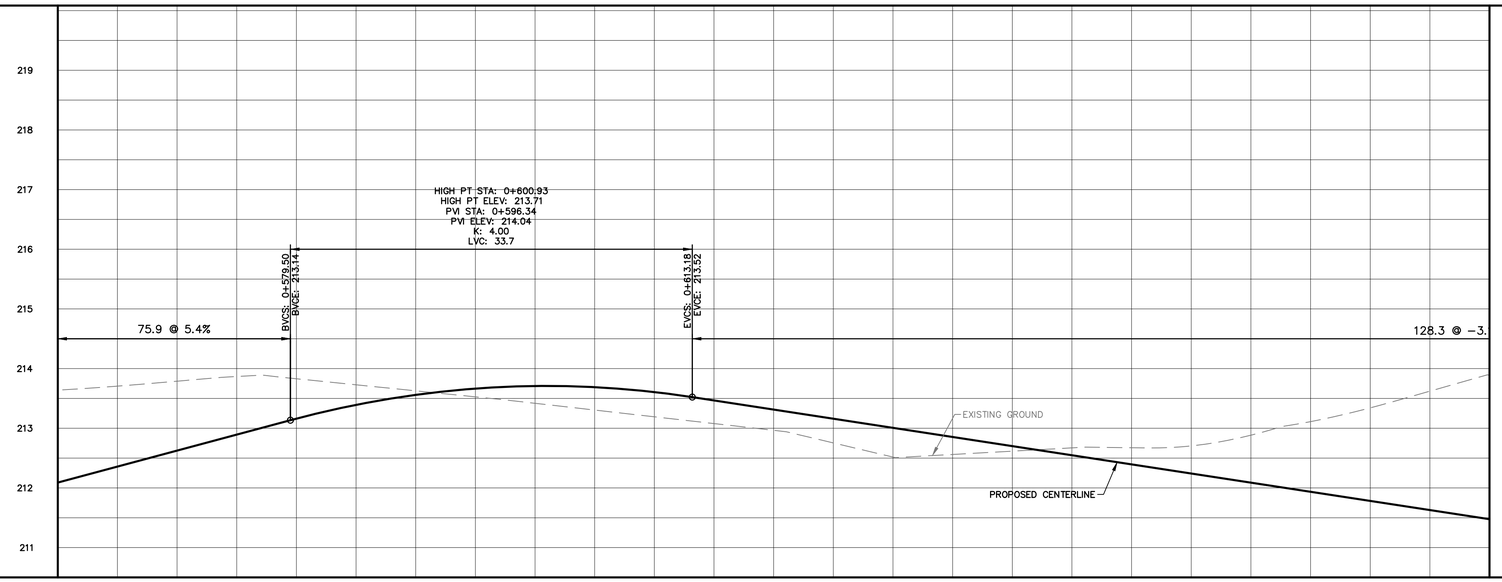
OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY PLAN AND PROFILE STA 0+460 TO 0+560

Drawn By: K.W.P.	SCALE: Horz. 1:250	Vert. 1:50	
Designed By: K.W.P.	Plot Date: 02/24/2022		
Checked By: J.D.F.	Project No.: 19-10874	Sht. No.:	
Engineer: D.A.K.	Dwg File No.: 10874 – PP	404	





℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS	0+560 212.09	0+580 213.16	0+600 213.71	0+620 213.31	0+640 212.70	0+660 212.09	0+680 211.48

℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC Dimensions are in METRES and/or MILLIMETRES unless otherwise shown

LEGEND TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

- PROPOSED ASPHALT
- EXISTING WETLAND AREA
- EXISTING 15.0m SETBACK AND BUFFER AREA
- EXISTING 30.0m SETBACK AND BUFFER AREA
- WETLAND ENCROACHMENT
- INFILTRATION FACILITY
- RETAINING WALL
- EX. FLOOD LINE / PR. FLOOD LINE
- EX./PR. EDGE OF PAVEMENT
- EX./PR. ROAD CENTERLINE
- EX./PR. EDGE OF SHOULDER/GRAVEL
- EXISTING GASMAIN
- EX. OVERHEAD BELL
- EX. UNDERGROUND BELL
- EX. OVERHEAD HYDRO
- EX. UNDERGROUND HYDRO
- EX. UNDERGROUND CABLE
- EX./PR. CULVERT
- EX./PR. DITCH
- EX./PR. FENCE
- PROPERTY LINE
- LOT LINE
- DEVELOPMENT BOUNDARY
- EXISTING VEGETATION
- 100.0 / 100.0 EX./PR. CONTOUR
- EX./PR. GUIDERAIL
- EXISTING EDGE OF WATER/STREAM
- HP EXISTING HYDRO POLE
- EX./PR. SIGN
- x215.00 x214.00 EX./PR. ELEVATION
- 1.0% PROPOSED GRADE

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E. wills@dmwills.com

Project Name/Location

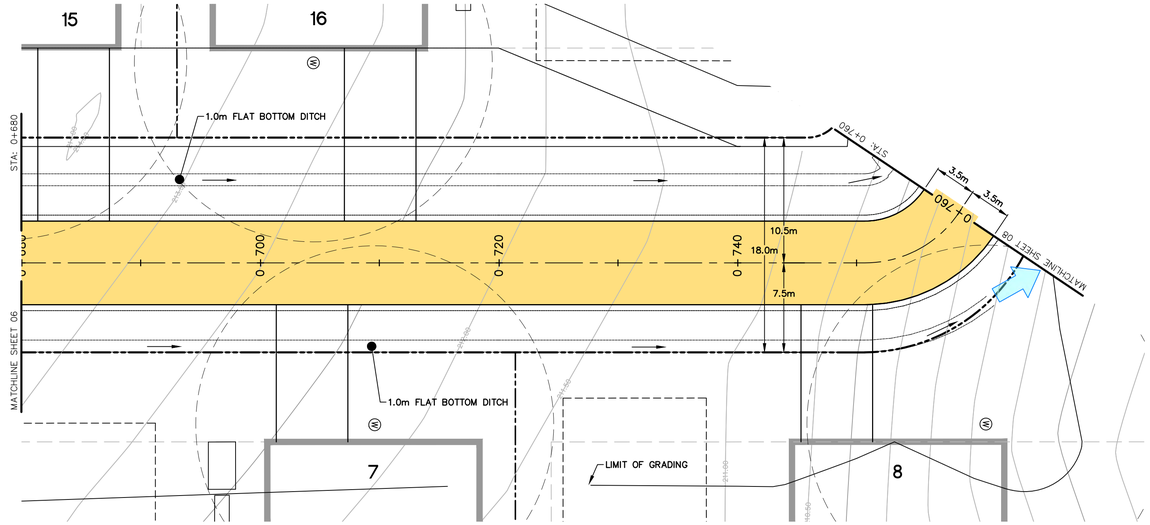
LIFE AT THE WOODLAND

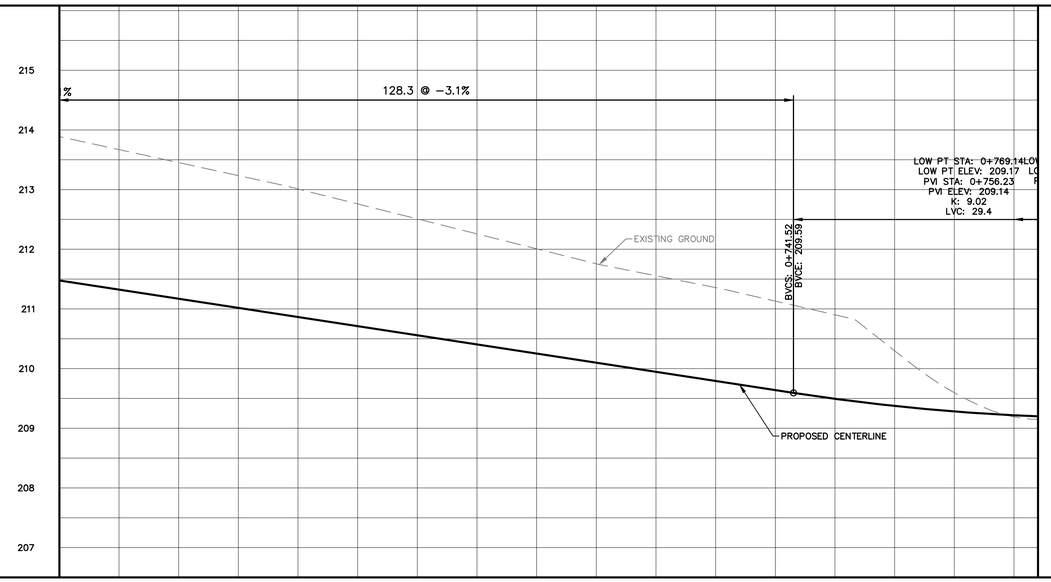
OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY PLAN AND PROFILE STA 0+560 TO 0+680

Drawn By: K.W.P.	SCALE: Horz. 1:250	Vert. 1:50	
Designed By: K.W.P.	Plot Date: 02/24/2022		
Checked By: J.D.F.	Project No.: 19-10874		Sht. No.:
Engineer: D.A.K.	Dwg File No.: 10874 – PP		405





℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS	0+680 211.48	0+700 210.86	0+720 210.25	0+740 209.64	0+760 209.22

℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC Dimensions are in METRES and/or MILLIMETRES unless otherwise shown

LEGEND TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

- PROPOSED ASPHALT
- EXISTING WETLAND AREA
- EXISTING 15.0m SETBACK AND BUFFER AREA
- EXISTING 30.0m SETBACK AND BUFFER AREA
- WETLAND ENCROACHMENT
- INFILTRATION FACILITY
- RETAINING WALL
- EX. FLOOD LINE / PR. FLOOD LINE
- EX./PR. EDGE OF PAVEMENT
- EX./PR. ROAD CENTERLINE
- EX./PR. EDGE OF SHOULDER/GRAVEL
- EXISTING GASMAIN
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- EX. OVERHEAD HYDRO
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- EX. UNDERGROUND CABLE
- EX./PR. CULVERT
- EX./PR. DITCH
- EX./PR. FENCE
- PROPERTY LINE
- LOT LINE
- DEVELOPMENT BOUNDARY
- EXISTING VEGETATION
- 100.0 / 100.0 EX./PR. CONTOUR
- EX./PR. GUIDERAIL
- EXISTING EDGE OF WATER/STREAM
- HP EXISTING HYDRO POLE
- EX./PR. SIGN
- x215.00 / x214.00 EX./PR. ELEVATION
- 1.0% PROPOSED GRADE

WILLS

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E. wills@dmwills.com

Project Name/Location

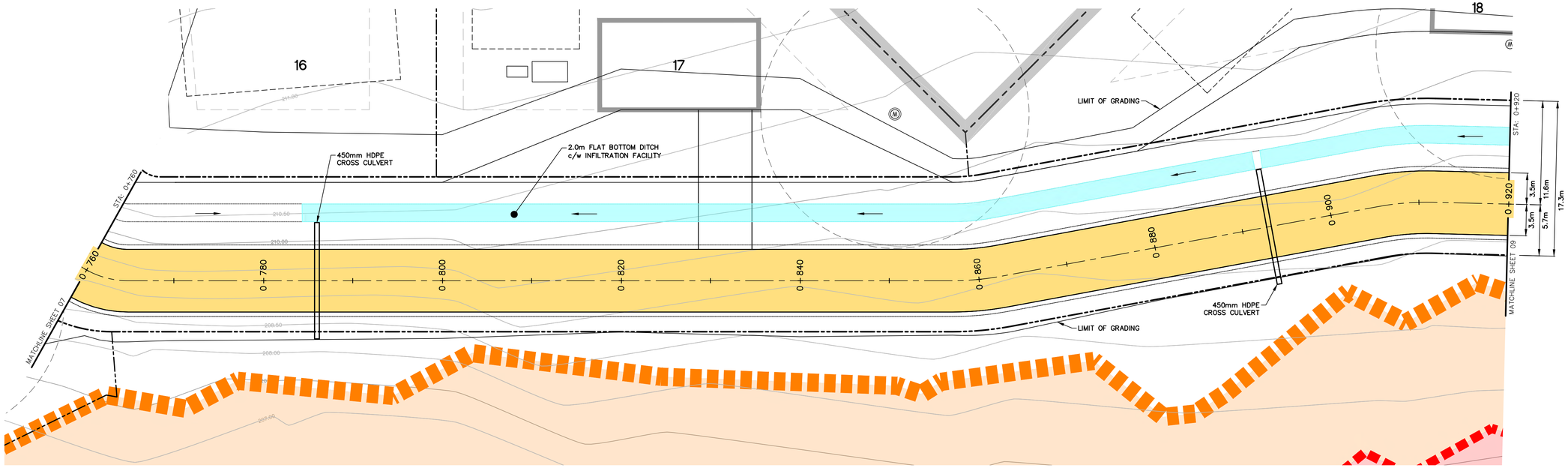
LIFE AT THE WOODLAND

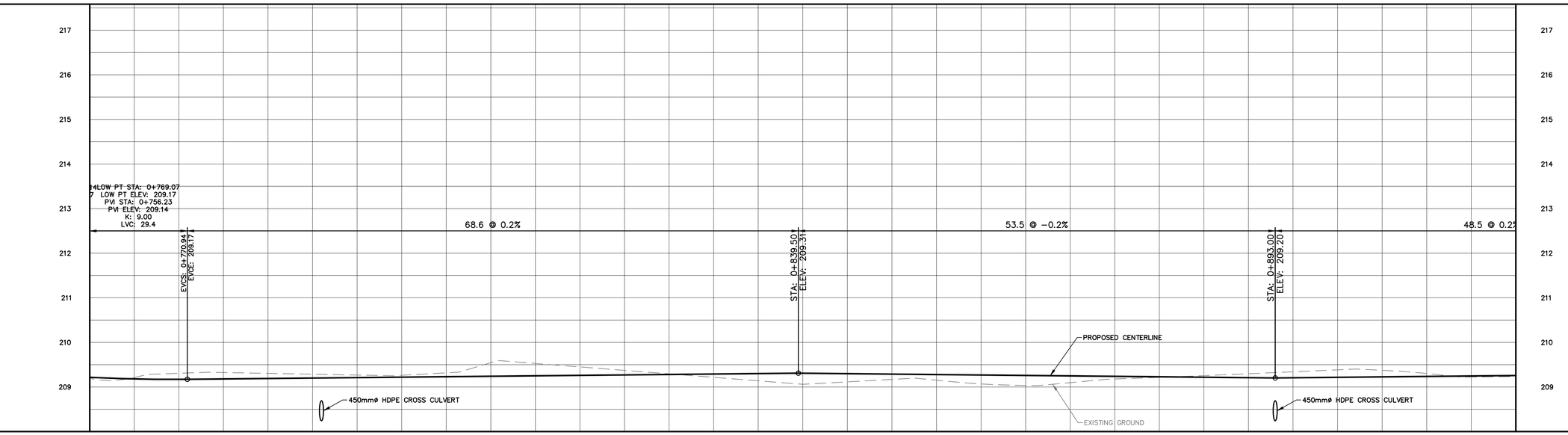
OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY PLAN AND PROFILE STA 0+680 TO 0+760

Drawn By: K.W.P.	SCALE: Horz. 1:250	Vert. 1:50	
Designed By: K.W.P.	Plot Date: 02/24/2022		
Checked By: J.D.F.	Project No.: 19-10874	Sht. No.:	
Engineer: D.A.K.	Dwg File No.: 10874 – PP	406	





℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS	0+760 209.22	0+780 209.19	0+800 209.23	0+820 209.27	0+840 209.31	0+860 209.27	0+880 209.23	0+900 209.22	0+920 209.26	CHAINAGE & ELEVATIONS

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC Dimensions are in METRES and/or MILLIMETRES unless otherwise shown

LEGEND TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

- PROPOSED ASPHALT
- EXISTING WETLAND AREA
- EXISTING 15.0m SETBACK AND BUFFER AREA
- EXISTING 30.0m SETBACK AND BUFFER AREA
- WETLAND ENCROACHMENT
- INFILTRATION FACILITY
- RETAINING WALL
- EX. FLOOD LINE / PR. FLOOD LINE
- EX./PR. EDGE OF PAVEMENT
- EX./PR. ROAD CENTERLINE
- EX./PR. EDGE OF SHOULDER/GRAVEL
- EXISTING GASMAIN
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- EX. UNDERGROUND CABLE
- EX./PR. CULVERT
- EX./PR. DITCH
- EX./PR. FENCE
- PROPERTY LINE
- LOT LINE
- DEVELOPMENT BOUNDARY
- EXISTING VEGETATION
- 100.0 EX./PR. CONTOUR
- EX./PR. GUIDERAIL
- EXISTING EDGE OF WATER/STREAM
- HP EXISTING HYDRO POLE
- EX./PR. SIGN
- x215.00 x214.00 EX./PR. ELEVATION
- 1.0% PROPOSED GRADE

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150 Jameson Drive
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P. 705.742.2297
F. 705.748.9744
E. wills@dmwills.com

Project Name/Location

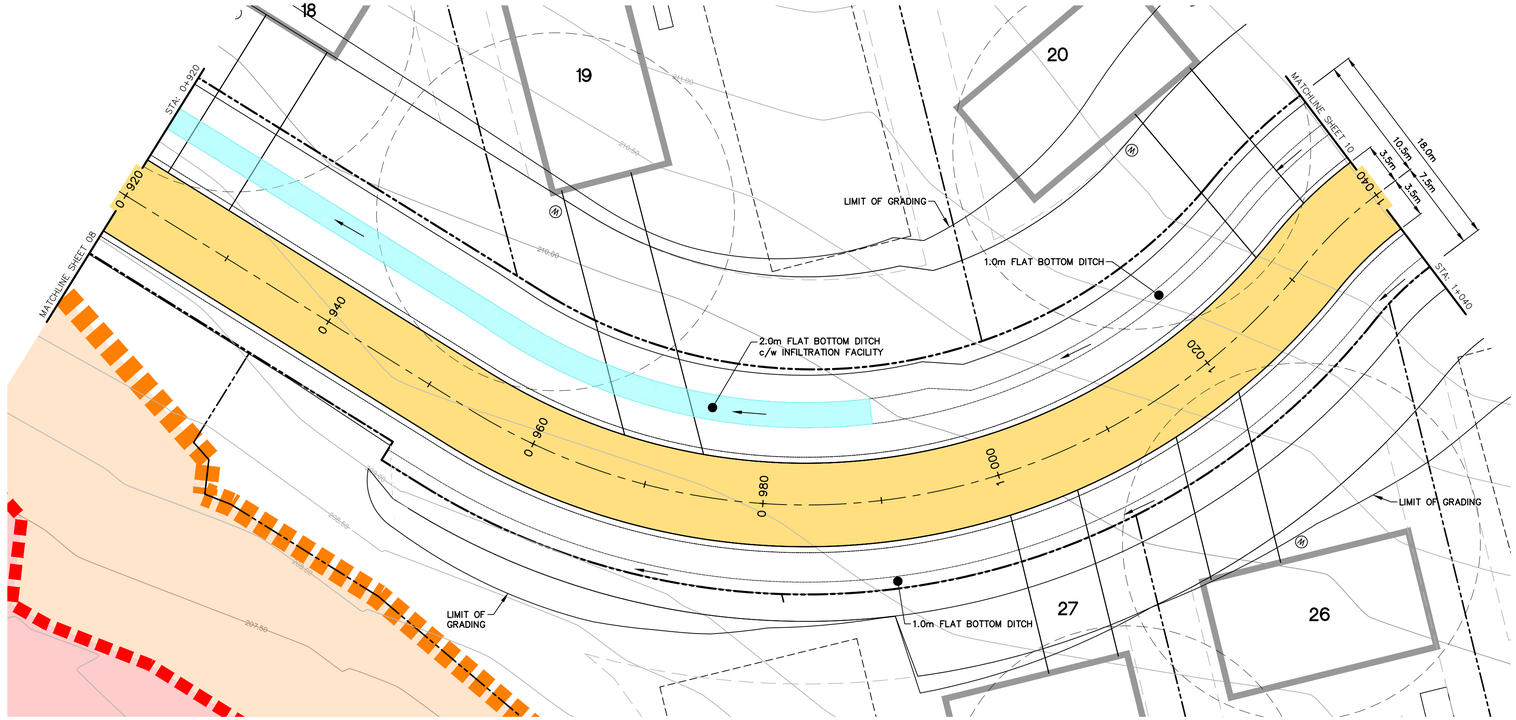
LIFE AT THE WOODLAND

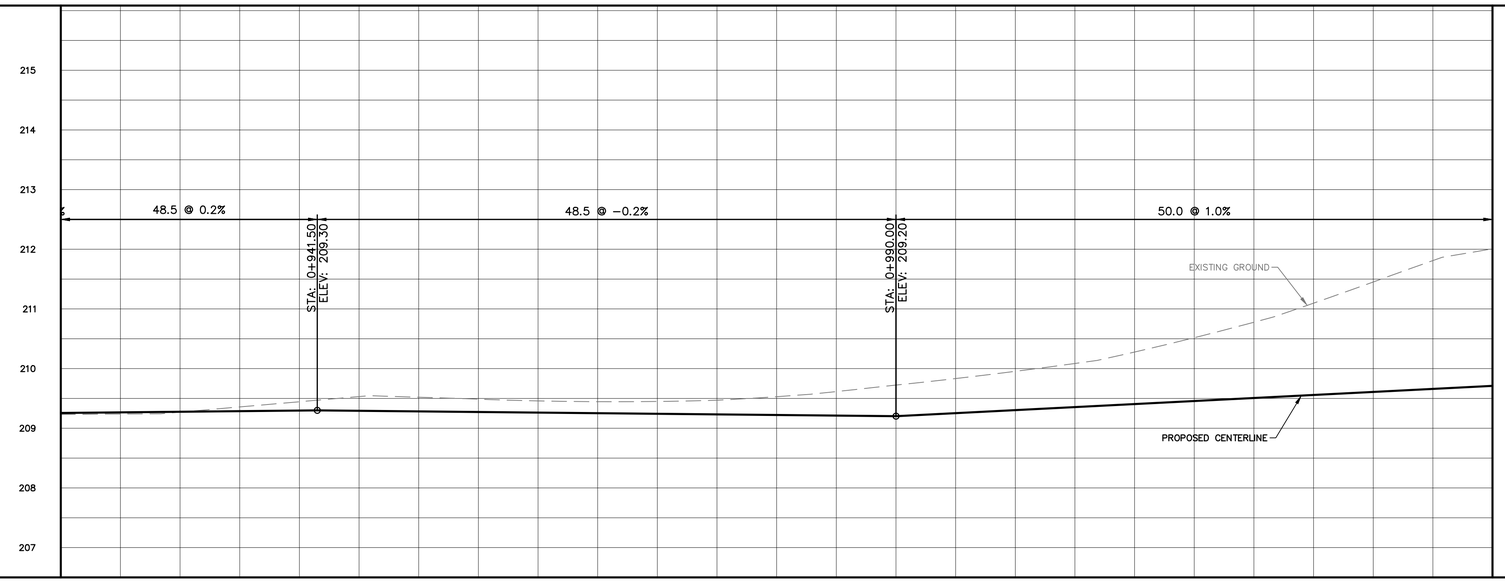
OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY PLAN AND PROFILE STA 0+760 TO 0+920

Drawn By: K.W.P.	SCALE: Horz. 1:250	Vert. 1:50	
Designed By: K.W.P.	Plot Date: 02/24/2022		
Checked By: J.D.F.	Project No.: 19-10874	Sht. No.:	
Engineer: D.A.K.	Dwg File No.: 10874 – PP	407	





℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS	0+920 209.26	0+940 209.30	0+960 209.28	0+980 209.22	1+000 209.30	1+020 209.51	1+040 209.71
	209.26	209.30	209.28	209.22	209.30	209.51	209.71

℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC — Dimensions are in METRES and/or MILLIMETRES unless otherwise shown

LEGEND — TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

- PROPOSED ASPHALT
- EXISTING WETLAND AREA
- EXISTING 15.0m SETBACK AND BUFFER AREA
- EXISTING 30.0m SETBACK AND BUFFER AREA
- WETLAND ENCROACHMENT
- INFILTRATION FACILITY
- RETAINING WALL
- EX. FLOOD LINE / PR. FLOOD LINE
- EX./PR. EDGE OF PAVEMENT
- EX./PR. ROAD CENTERLINE
- EX./PR. EDGE OF SHOULDER/GRAVEL
- EXISTING GASMAIN
- EX. OVERHEAD BELL
- EX. UNDERGROUND BELL
- EX. OVERHEAD HYDRO
- EX. UNDERGROUND HYDRO
- EX. UNDERGROUND CABLE
- EX./PR. CULVERT
- EX./PR. DITCH
- EX./PR. FENCE
- PROPERTY LINE
- LOT LINE
- DEVELOPMENT BOUNDARY
- EXISTING VEGETATION
- 100.0 / 100.0 — EX./PR. CONTOUR
- EX./PR. GUIDERAIL
- EXISTING EDGE OF WATER/STREAM
- HP — EXISTING HYDRO POLE
- EX./PR. SIGN
- x215.00 / x214.00 — EX./PR. ELEVATION
- 1.0% — PROPOSED GRADE

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Project Name/Location

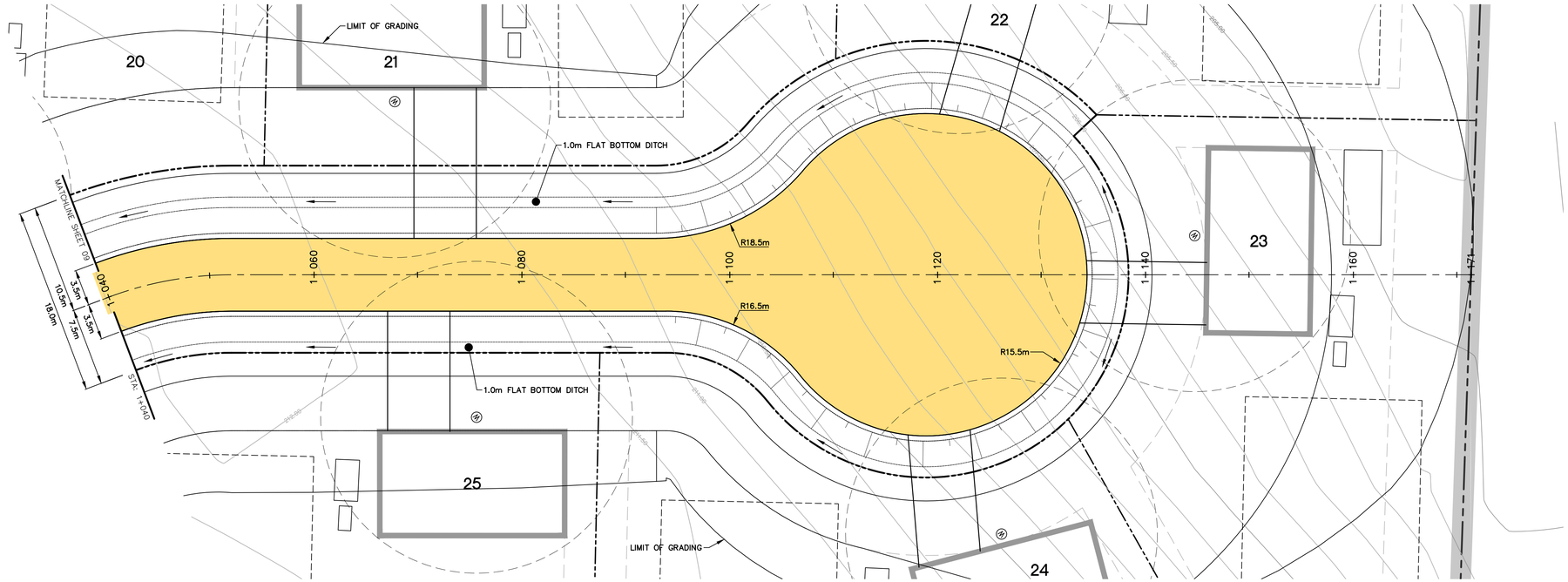
LIFE AT THE WOODLAND

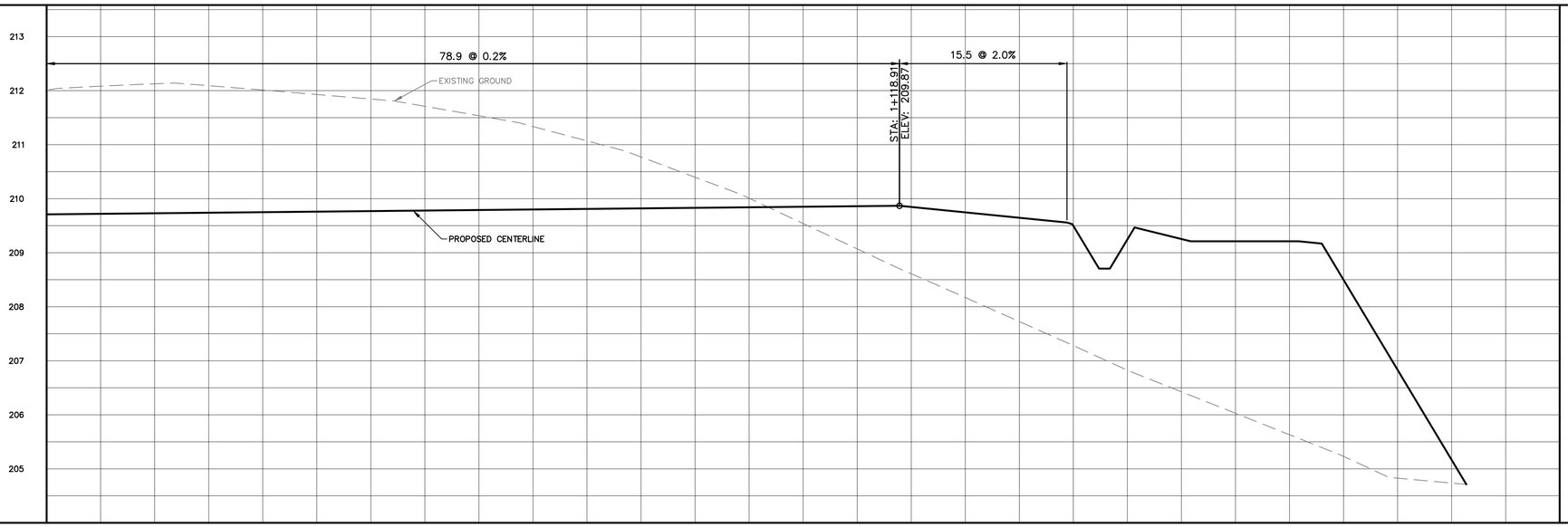
OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY PLAN AND PROFILE STA 0+920 TO 1+040

Drawn By: K.W.P.	SCALE: Horz. 1:250	Vert. 1:50	
Designed By: K.W.P.	Plot Date: 02/24/2022		
Checked By: J.D.F.	Project No.: 19–10874	Sht. No.:	
Engineer: D.A.K.	Dwg File No.: 10874 – PP	408	





℄ – DITCH (R)

℄ – DITCH (L)

CHAINAGE & ELEVATIONS	1+040 209.71 212.01	1+060 209.75 212.01	1+080 209.79 211.54	1+100 209.83 210.40	1+120 208.61	1+140 206.82	1+160 205.23	1+180	CHAINAGE & ELEVATIONS

TRUE NORTH

KEY PLAN

REVISIONS

No.	Description	Date
2	ISSUED FOR DRAFT PLAN APPROVAL – 2ND SUBMISSION	03/14/23
1	FSR	02/24/22

METRIC Dimensions are in METRES and/or MILLIMETRES unless otherwise shown

LEGEND TO BE READ IN CONJUNCTION WITH OPSD 100 SERIES

- PROPOSED ASPHALT
- EXISTING WETLAND AREA
- EXISTING 15.0m SETBACK AND BUFFER AREA
- EXISTING 30.0m SETBACK AND BUFFER AREA
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Project Name/Location

LIFE AT THE WOODLAND

OTONABEE-SOUTH MONAGHAN

Drawing Title

PRELIMINARY PLAN AND PROFILE STA 1+040 TO 1+171

Drawn By: K.W.P.	SCALE: Horz. 1:250	Vert. 1:50	
Designed By: K.W.P.	Plot Date: 02/24/2022		
Checked By: J.D.F.	Project No.: 19-10874		Sht. No.:
Engineer: D.A.K.	Dwg File No.: 10874 – PP		409