



May 24, 2017

2293040 Ontario Inc. Pilgrim's Rest Campground 23 Cheboutequion Drive North Kawartha, Ontario K0L 2H0

Attention: **Pieter Venema**, President

Re: Hydrogeological and Site Servicing Study

Proposed Residential Development

Pilgrim's Rest Campground

Part Lots 3 & 4, Concession 11 (Burleigh)

Township of North Kawartha, County of Peterborough

Our File No. 12-1629

## Dear Mr. Venema:

We are pleased to present our hydrogeological and site servicing report in support of a proposed residential condominium redevelopment of the property currently occupied by the Pilgrim's Rest Campground.

Despite the somewhat challenging groundwater supply conditions, the study has successfully demonstrated that private wells and sewage systems will be practical and sustainable at this site. Our report provides a series of recommendations intended to assist future purchasers in regards to establishing those services.

Should you have any questions, please contact the undersigned.

Yours truly, **Oakridge Environmental Ltd.** 

# **Original Signed By**

Brian R. King, P. Geo. Principal

# Hydrogeological and Site Servicing Study Proposed Residential Development Pilgrim's Rest Campground

Prepared For:

Mr. Pieter Venema 2293040 Ontario Inc. 23 Cheboutequion Dr. North Kawartha, Ontario K0L 2H0

Prepared By:

Oakridge Environmental Ltd. 380 Armour Road, Suite 127 Peterborough, Ontario K9H 7L7

May 24, 2017

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# Hydrogeological and Site Servicing Study Proposed Residential Development Pilgrim's Rest Campground

# 1.0 Introduction

# 1.1 Site Description and Access

The subject property is situated on Northey's Bay Road, approximately 1.5 km west of County Road 6 (Figure 1). The site is currently utilized as a seasonal camping resort (Pilgrim's Rest Campground) and is bounded by Northey's Bay Road in the north, Cheboutequion Drive in the west, Stony Lake to the south and Jack Creek to the east. The entrance to the campground property is via Cheboutequion Road, approximately 170 m south of it's intersection with Northey's Bay Road.

The campground can also be accessed by a fire route (Fire Route 24) that is utilized by neighbouring residents to access their private lakefront cottages adjacent to the campground to the west.

The subject site currently contains approximately 87 serviced trailer sites in addition to approximately 20 unserviced camping sites (Figure 2).

# 1.2 Approach

A proposed seasonal residential (condominium) development is intended to replace the existing seasonal campground. To support the development application, a hydrogeological and site servicing study is required to verify that there is a sufficient quantity of acceptable quality water available to supply the future residences and to determine the allowable density by completing an impact assessment based on effluent loading for the site.

These requirements are consistent with Section 2.6.3 of the County of Peterborough's Official Plan (OP), which refers to compliance with Ministry of Environment and Climate Change (MOECC) guidelines.

The hydrogeological study requirements for development applications are described in MOECC Procedures "D-5-4" and "D-5-5". Briefly, Procedure D-5-4 pertains to evaluating the site's capacity to handle septic effluent and environmental impact, whereas D-5-5 pertains to the various tests needed to demonstrate an adequate water supply.

In addition to the preceding, our study also has regard for Part 8 of the Ontario Building Code.

# 2.0 Scope of Work

Based on the above guidelines, the following scope of work has been completed:

- Relevant available background data associated with the site and surrounding lands have been compiled and reviewed.
- Our existing base plan has been updated to incorporate our most recent field data, augmented with our own mapping-grade differential GPS data.
- Site inspections have been conducted to assess the terrain and hydrogeological conditions.
- Ministry of Environment and Climate Change (MOECC) well record data for the site area have been obtained and reviewed using our Groundwater Information System (GWIS). Cross sections have been prepared to illustrate aquifer distribution in the area.
- A survey of neighbouring wells near the site has been completed to obtain information regarding local groundwater supply conditions.
- Shallow soil explorations have been conducted about the site by excavating test pits and advancing manual hand-auger boreholes for the purpose of characterizing soil conditions.
- An assessment of the site's potential capacity for sewage disposal has been conducted following the MOECC's Procedure D-5-4, guidance for impact assessments of individual on-site sewage systems.
- A review of local groundwater supply conditions, based on existing data, has been conducted.
- Seven (7) test wells were constructed for the purpose of conducting hydraulic tests in accordance with MOECC's Procedure D-5-5.
- Preliminary pumping tests were conducted on seven (7) wells, including the existing well that services the primary residence and campground office, to determine the approximate yields prior to longer-term testing.
- Select wells were subjected to a successful hydraulic fracturing program in an effort to improve well yields.
- Five (5) test wells were subjected to pumping tests and water quality sampling in accordance with MOECC Procedure D-5-5 guidelines.

- A conceptual servicing plan was prepared to illustrate how each proposed lot could be serviced while satisfying the constraints determined by the Environmental Impact Study (under separate cover) and this study.
- All data have been assessed and interpreted.
- This Hydrogeological and Site Servicing report has been prepared.

Our findings are presented in the following sections.

# 3.0 Physical Setting

# 3.1 Existing Property Use

Currently, the subject site contains two separate parcels consisting of an approximately 28.5 ha (70 acre) mainland and an 0.5 ha (1 acre) island immediately offshore. The site is operated as a seasonal trailer and camping resort referred to as Pilgrim's Rest Campground. The property contains approximately 87 fully-serviced trailer sites in addition to approximately 20 un-serviced camping sites. All of the serviced trailer sites are located within approximately 220 m of the shore of Stony Lake. Un-serviced camp sites are mostly situated north of the trailer sites within the forested areas of the property, however, camp sites are also located on the island.

The campground obtains water from a communal surface water intake located at the mouth of Jack Creek. A pumphouse on the bank of the creek supplies chlorinated water to the campground. It is understood that the water supply system is registered with the Ministry of Health (through the Peterborough County-City Health Unit) and is subjected to regular water quality compliance testing.

Wastewater generated by the campground is disposed of via a single large sewage system situated approximately 25 m from Stony Lake. We estimate the theoretical sewage flow rate of the campground to be approximately 45,000 L/day, based on Code and Guide for Sewage Systems (Ontario Building Code - OBC). The sewage system does not operate under an Environmental Compliance Approval (ECA) or Certificate of Approval (C of A) and likely predates any such requirements. The system includes a fully in-ground effluent disposal bed and does not incorporate any additional treatment beyond what is provided by the septic tanks.

The public washroom and laundry facilities are directly connected to the communal sewage system. However, none of the trailer sites are serviced directly by the system. Instead, each trailer must be manually pumped out through a pumping station that is connected to the works. As such, the current system requires a considerable amount of wastewater handling.

In addition to the central sewage system, a number of Class 1 sewage systems (outhouses) also occur throughout the site, representing an unknown proportion of the site's total sewage flows. Those units do not provide any treatment.

# 3.2 Proposed Development

To replace the existing campground, a less concentrated residential condominium development has been proposed at the site. Each condominium lot (unit) will be privately serviced by a well and modern subsurface sewage disposal system.

While the property encompasses approximately 29.0 ha (72 acres), the proposed development will only occupy approximately 19.0 ha (47 acres). The remaining area will be maintained in a natural state for passive recreational uses only (e.g., walking trail, etc). There are no redevelopment plans for the 0.5 ha (1 acre) island associated with the property. Furthermore, the island will be placed in a protective zone category which will preclude future development, reflecting the recommendations of a Stage 3 archaeological assessment completed by others.

The proposed development area (Figure 2) will utilize the campground's existing driveway off Cheboutequion Drive and Fire Route 24.

Replacement of the trailer park with a modern residential development, based on properly constructed private sewage systems that are well set-back from the lake with a less concentrated use of the waterfront areas, should be viewed as beneficial with respect to protection and improvement of the water quality of Stony Lake.

# 3.3 Topography and Drainage

The site is situated immediately north of the southern edge of the Canadian Shield. As such, the topography is dominated by rock knobs and trough-like valleys which often contain wetlands or pockets of poorly drained soil. Total relief across the site is approximately 20 m, as measured from Northey's Bay Road (255 masl) to the lakeshore (235 masl).

The subject property contains a series of small scarps and somewhat discrete plateau-like areas, typically with a few metres of elevation difference between them. The distribution of these features is controlled by the bedrock structure.

Most of the northern part of the site is forested and unimproved. In contrast, the existing camp occupies a cleared and comparatively level area within 250 m of the shoreline. The camp area appears to have been subjected to considerable grading and filling over time, including the creation of filled areas along several parts of the shore. In several instances, filling has apparently replaced lacustrine shoreline wetland areas.

The site's drainage pattern is somewhat complex, resulting from the Shield terrain. The major watercourse (other than Stony Lake) is Jack Creek, an important perennial feature which defines approximately half of the site's eastern boundary. To the north and east of the site, Jack Creek is a fairly wide, slower-flowing stream associated with wetland conditions. Closer to the lake, the creek occurs in a deeply incised valley wherein flows cascade over the bedrock, forming attractive rapids and falls.

A small and sluggish bifurcated stream system crosses the northern part of the site, entering the property primarily via culverts below Northey's Bay Road. Some of those flows appear to originate in the adjacent Provincial Park to the north and are mapped by the Ministry of Natural Resources and Forestry (MNRF) as permanent streams. Some seasonal flows may also contribute, following minor swales. Typically, the flows occupy several narrow, poorly defined channels with associated "pocket wetlands" occurring in low lying areas between the rock outcroppings. Although possible to cross on foot with some difficulty, this network of streams and wetlands effectively forms a barrier to mechanized crossing, separating the northernmost part of the property from the rest of the site.

Drainage on the site is somewhat complex with three local drainage divides occurring on the previously developed portion of the property. One of these local drainage divides occur near the centre of the site, essentially at the existing internal road (Figure 3). To the east, drainage is southeastward towards a small unevaluated wetland associated with Jack Creek. To the west, drainage occurs in a small stream feature associated with low-lying wet area (possible wetland) that drains through a culvert below Fire Route 24. This feature appears to collect and convey minor groundwater seepage from the base of the rock, plus any accumulated runoff. The presence of the drainage divide and existing road provides a crossing for these drainage features.

A second (minor) local drainage divide splits surficial drainage between the wetlands associated with a small tributary of Jack Creek on the northeast side of the hydroelectric corridor and the unevaluated wetland to the southeast that is associated with Jack Creek.

The third local drainage divide is located near the northern extent of the current trailer sites (Figure 3). On the south-side of the divide, surface water flows south into Stony Lake while on the north-side flows are split between the unevaluated wetland associated with Jack Creek and the small tributary that drains towards Fire Route 24.

The remainder of the property drains through the central wetland area associated with Jack Creek.

# 3.4 Regional Geology

## 3.4.1 Bedrock Geology

The bedrock geology is composed entirely of Precambrian age rocks, consisting primarily of granitic gneiss and iron rich metasediments (rusty schists), as illustrated by Figure 4. Typically, these rock types are dense and competent, although the rusty schist (also referred to as a type of "iron formation") can be friable, with reduced competency. These iron rich rocks could be a source of iron mineralization for local groundwater and surface water. Despite the dominant granite gneiss bedrock, metamorphosed limestone (i.e., marble) is also known to occur within the metasediments in the central portion of the site.

Published mapping indicates that an ancient (inactive) NE-trending fault zone occurs along the path of Jack Creek. The creek may be following the rock structure associated with that feature. It is not known whether the structural zone is of any significance with respect to the movement of groundwater.

## 3.4.2 Surficial Geology

Published mapping indicates that the site's physiography is nondescript, consisting of thin soils overlying bedrock. However, aggregate resources mapping provides some detail regarding the distribution of granular deposits in the area, including two small occurrences on (or partially on) the subject site (Figure 5). These deposits consist predominantly of outwash sand and are classified as "secondary" deposits in terms of their resource importance. Both contain 1.5 m to 5 m of sand, described as "suitable for pit-run uses" only.

Based on our site observations, the extent of the two sand deposits appear to have been grossly exaggerated by the mapping. The general locations are, however, reasonably accurate. The central sand deposit has been historically exploited on the site, possibly for shoreline reclamation, maintenance of the small beach area and to provide fill for miscellaneous purposes.

# 3.5 Site Geology

# 3.5.1 Shallow Soils Exploration

To explore the on-site geology, soil and terrain conditions, a series of twenty-five (25) test pits and six (6) hand-auger holes were excavated to expose the shallow subsurface (Figure 6).

Briefly, the proposed development area of the site is dominated by granitic bedrock subcrops covered with a thin layer of sand (SP type) with silt and occasional gravel. The

sand deposits occur in a variety of thicknesses between areas of bedrock outcrops. In depressed areas between outcrop and subcrop locations, black organic silty topsoil occurs in thicknesses ranging between 10 and 50 cm. Typically, silty sand with trace gravel occurs below the topsoil in these locations.

At the southern extent of the site (where the existing campground development is located), the subsurface soils are dominated by sand fill with gravel and occasional cobbles/boulders. Most of the sand fill on the previously developed lands seem to have originated from a small sand pit that was once located in the central portion of the site (Figure 2).

Relatively significant (e.g. >1 m thickness) sand deposits occur in two (2) locations on the property. One was formerly present in the central portion of the site, but has been exhausted of most of the useable aggregate materials. The second deposit occurs at the northwestern extent of the site where the current campground office is located. These "acidic" silicate-rich sands appear to be derived from the underlying gneissic bedrock.

Buried lake sediments and wood debris have also been encountered by camp staff in other areas of the property.

Copies of the test pit logs and grain size analysis are included in Appendix A.

# 3.5.2 Terrain Mapping

Based on the subsurface explorations and our site inspections, a terrain map has been prepared to illustrate overburden thickness across the site (Figure 6). As the on-site geology is relatively consistent (i.e., dominated by a mantle of sandy soil), our terrain mapping focuses on the overburden thickness, which have been categorized into three (3) units:

- areas with no significant overburden materials (0 to 0.2 m thick) referred to as "Terrain Type A",
- areas with minimal overburden (0.2 to 0.8 m thick) referred to as "Terrain Type B", and
- areas with significant overburden thickness (>0.8 m thick) referred to as "Terrain Type C".

#### 3.6 Shallow Groundwater

Groundwater was encountered in nine (9) of the of the exploratory test pits. Typically the depth to groundwater was measured to be approximately 0.7 m below the ground surface. It is expected that groundwater in these locations represents water trapped on top of the bedrock within bedrock depressions that occur throughout the site. These "perched" water

bearing zones would not represent a significant water source (i.e., not aquifers) and would not be suitable for future supply purposes.

Groundwater discharge was not observed on the site, however, some minor seasonal groundwater discharge is assumed to occur based on indicators (e.g., vegetation, erosional scars), associated with the banks of the wetlands and watercourses observed on the site.

# 4.0 Hydrogeology

#### 4.1 MOECC Well Record Database

As part of this study, we have compiled and reviewed MOECC well record data for recorded wells surrounding the subject site (within approximately 5 km). In total, 310 local well records have been incorporated into our Groundwater Information System (GWIS), representing conditions in the site area (Appendix B). The locations of these wells, based on the co-ordinates provided, are illustrated by Figure 7.

Of the 310 local well records, only 72 fall within 1 km of the site, restricting statistical analysis within the immediate site area. Therefore, for the purpose of statistical analysis (below), the entire data set of 310 local well records were utilized.

All but one (1) of the well records represent drilled wells. Dug wells are not likely to occur in the area given the minimal overburden cover and granitic bedrock which likely wouldn't support a shallow aquifer that could be utilized by dug wells.

The majority of the recorded wells are described as being for domestic use. The average (mean) reported test rate is approximately 8.0 gpm, with most wells achieving a fairly low yield in the range 1 gpm to 4 gpm (Figure 8). However, the reported test yields range up to a maximum of 100 gpm, somewhat skewing the average. The dataset includes four (4) instances of wells described as having been abandoned due to "insufficient supply" and two (2) instances of a well being abandoned due to unspecified water quality issues.

Overall, while these statistical results indicate that an adequate quantity of water supplies are generally available in the study area, these data also suggest that obtaining a successful well (i.e., well yield of 5 gpm or greater) may require more than one attempt. At the very least, the data indicate that groundwater supply conditions are variable, as would be expected from the geological setting.

According to the MOECC's well record data, local wells encounter groundwater within a wide elevation range of 195 masl to 248 masl (mean = 222 masl). Within that range, there appears to be a highly variable distribution of aquifer depths, however, the majority of the wells appear to intersect an aquifer from 216 masl to 230 masl (Figure 8). Below the subject site, the average aquifer elevation would correspond to a depth of roughly 20 m. The well record data does not suggest that there is any appreciable correlation between

aquifer elevation and well yield. As such, there does not appear to be any significant difference in yield potential from one aquifer to another.

Cross sections illustrating the distribution of aquifers through the site are presented in Figures 9 and 10.

## 4.2 Regional Aquifer Distribution

From the cross sections (Figures 9 and 10) we have identified three (3) principal aquifers that occur in the site area. For simplicity, these are labelled as the "Overburden/Basal Aquifer", the "Shallow Bedrock Aquifer" and the "Deep Bedrock Aquifer". Each is briefly described below:

# Overburden / Basal Aquifer

In areas of sufficient overburden thickness, dug wells would typically be utilized to tap a shallow aquifer occurring at, or just above the underlying bedrock. Typically, these aquifers are variable and occur within a few metres of the ground surface (usually referred to as overburden aquifers).

Deeper wells that utilize this aquifer occur only in areas where the overburden thickness is substantial. In those cases, the wells are completed in sand layer that occurs immediately above the bedrock (typically referred to as the basal aquifer).

As outlined previously, the subject site is not likely to contain this type of aquifer.

### Shallow Bedrock Aquifer

The majority of wells in the study area utilize an aquifer that occurs between 210 masl and 230 masl. This aquifer correlates closely with the average surface water elevation of Stony Lake (approximately 234 m asl). The driller's logs do not appear to indicate any changes in rock formation correlating with the presence of water suggesting the water is obtained within fracture zones in the bedrock.

### Deep Bedrock Aquifer

A few wells in the dataset utilize an aquifer that appears to occur between elevations 160 masl and 180 masl, in the site area. The average yield of the wells in this aquifer appears to be approximately 6 gpm.

Based on the preceding, both of the bedrock zones are considered the target aquifers for

the proposed development. The corresponding well depths will, therefore, vary from roughly 20 m to 90 m below the subject site. As these aquifers appear to be associated with fracture zones within the bedrock, their occurrence will be highly variable.

## 4.3 Well Survey

In order to better define local aquifer conditions, a door-to-door well survey was completed within 500 m of the subject site on June 5, 2013. Residents were asked to provide basic information on their well, sewage system and occupancy in order to assess local servicing conditions. Residents not home at the time of the survey were left a letter explaining the purpose of the survey and contact information if they chose to participate. A copy of the well survey questionnaire and letter have been included in Appendix C.

Unfortunately, only one respondent to the well survey provided information. It is understood that the resident has a lake intake system as their main potable water supply. This is not unusual given that most of the homes/cottages in the area are waterfront lots.

#### 4.4 On-site Test Wells

Seven (drilled) test wells are present on the subject site and have been utilized for this investigation. One of the wells (TW-1) was constructed prior to commencing the hydrogeological investigations, suppling potable water to the camp office and the associated residence.

All of the wells were constructed by (or under the direction of) White's Water Well Drilling. All of the wells intersected the gneissic bedrock within 4 m of the ground surface. Each well casing was installed into the underlying bedrock and grouted to surface. Well locations were selected based on accessibility and to provide a somewhat even distribution of wells about the property. Copies of the respective well records are presented in Appendix D. A detailed description of each well is included below.

#### TW-1

TW-1 was constructed on March 23, 2012. The well was drilled to a depth of 51.5 m (169 ft), reportedly intersecting two (2) water-bearing zones at 27.1 m (89 ft) and 50.3 m (165 ft). It is understood that the well was subject to low-pressure hydraulic fracturing in an attempt to increase its yield. That procedure appears to have been successful, as the camp office and residence have not experienced any issues related to water quantity. Water quality has been characterized by camp staff as being high in iron.

#### TW-2

TW-2 was constructed on May 12, 2013 in the central portion of the property, just north of the former aggregate pit. The well was constructed to a depth of 57.9 m (190 ft). However, upon completion was reported as "dry".

The well was subjected to low-pressure hydraulic fracturing in an attempt to encounter water. After many attempts, a minor fracture in the bedrock was observed to be supplying approximately 2 gpm. Using a down-hole video camera, the water was observed to be entering the well bore at a depth of 39.0 m (128 ft).

## TW-3

TW-3 was constructed on July 13, 2013 on the south-western portion of the subject site, having a total depth of 91.4 m (300 ft). Some water was encountered by TW-3, however, the depth of the aquifer could not be determined by the driller.

Low-pressure hydraulic fracturing was utilized in order to obtain increased well yield. Upon completion, the well was observed to be capable of supplying 6 gpm according to the driller's log. Water was observed (utilizing a video camera) to be entering the well at a depth of 37.2 m (122 ft). Although not observed, the driller suggested that another water bearing zone was also intersected below 41 m (135 ft). Given the limited capabilities of the driller's equipment, it was not possible to drawdown the water column below 41 m (135 ft). Despite the well record indicating water bearing fractures at 20.5 m (67 ft) and 21.3 m (70 ft), these were observed to be essentially dry fractures under sustained pumping.

#### TW-4

TW-4 was constructed on November 20, 2013 approximately 42 m southwest of Jack Creek and 43 m southeast of a small wetland inlet associated with Jack Creek. In addition, TW-4 was located adjacent to the mapped fault zone indicated by the bedrock geology mapping. Despite the close proximity of surface water features surrounding the well, the wellhead is elevated on top of a large bedrock outcrop that extends 5 m above the surrounding surface water features. The drilling of TW-4 was extended specifically to examine the potential for deeper aquifers based on the success of TW-3.

TW-4 was constructed to a depth of 89 m (292 ft) and although the well had approximately 83 m of water column immediately following the drilling, the well contractor indicated that the well yield was unknown, but likely around 2 gpm. As such, the well was subjected to low-pressure hydraulic fracturing to increase the yield. The hydraulic fracturing was followed by pumping to determine where the

water bearing fracture(s) occurred.

Unfortunately, the well driller was unable to drawdown the water column sufficiently to determine where the water was coming into the well.

#### TW-5

TW-5 was constructed on December 9, 2013 in the central portion of the property, 67 m southeast of a small tributary that flows southwest into Stony Lake. The well was constructed to a depth of 76.2 m (250 ft). The driller indicated that a significant (i.e., 5 gpm) water bearing fracture had been encountered at an unknown depth.

The well was subjected to low-pressure hydraulic fracturing followed immediately by pumping in an attempt to "clean-out" the well. Water was observed to be entering the well (utilizing a down-hole camera) at a depth of 43.6 m (143 ft). At the cessation of pumping, the water appeared to be running clear.

#### TW-6

TW-6 was constructed on May 7, 2014, by Wensley Water Well Ltd under the supervision of White's Water Well Drilling Ltd. The well was constructed to a depth of 99.1 m (320 ft), on the northern part of the proposed development, adjacent to the former sand pit.

During well construction, the drill intersected a bedrock contact between the upper black granite gneiss and the underlying feldspar-rich pink granite. The driller indicated the presence of a water bearing fracture at a depth of 33.3 m (110 ft) and further indicated that there could also be a minor water bearing zone at a depth of 88.4 m (290 ft). In addition, a water bearing fracture was noted at a depth of 36.5 m (120 ft) during subsequent pumping.

Upon completion of the well, the driller reported an approximate yield of 2 gpm. It is suspected that the driller's estimate was limited by the depth at which the pump was set (i.e., 46 m). The well was subjected to low-pressure hydraulic fracturing by White's Water Well Drilling Ltd. to increase the apparent well yield. Following the hydrofracking, the driller's estimated yield was 6 gpm.

#### TW-7

TW-7 was constructed on June 12, 2014, by Wensley Water Well under the supervision of White's Water Well Drilling Ltd. The well was constructed to a

depth of 48.8 m (160 ft), situated approximately 65 m north of Stony Lake in the southern portion of the site. Upon completion, the driller noted a water bearing fracture at a depth of 36.6 m (120 ft).

Two (2) water bearing fractures were noted to occur at depths of 16 m (52 ft) and 23.6 m (77 ft) during subsequent pumping. Following the same procedure as all previous test wells, the well was subjected to low-pressure hydraulic fracturing conducted by White's Water Well Drilling Ltd. The driller estimated the yield of the well to be 5 gpm following the hydrofracking.

# 4.5 Preliminary Pumping Tests

In order to determine whether the wells were suitable for aquifer testing in accordance with MOECC Procedure D-5-5, a series of short-term (i.e., 2 to 3 hour) pumping tests were conducted. A brief description of the preliminary pumping tests is included below.

#### TW-1

TW-1 was subjected to a preliminary pumping test on May 6, 2013 utilizing the well's existing plumbing. Unfortunately, the existing pump and pressure tank assembly limited the ability to reduce the flow rate to the rate recommended by the driller. As such, after pumping the well for 50 minutes at an average rate of 0.53 litres per second (L/s) or 7.0 imperial gallons per minute (igpm), the water level approached the pump and the test had to be stopped. During the test, a total of 1,582 L were extracted from the well. The well recovered to within 87% of the original (static) water level within approximately 2.5 hours.

It is understood that the resident of the home that is serviced by TW-1 did not experience any interruptions in water supply following the cessation of the preliminary pumping test.

Given the limitations of the existing plumbing at TW-1, it was determined that the well was not suitable for future testing in accordance with MOECC Procedure D-5-5, however it is quite capable of supplying water to the primary residence of the campground superintendent and the campground office.

## TW-2

TW-2 was subjected to a 192 minute pumping test on June 12, 2013 utilizing a submersible pump supplied by the well contractor. The end of the discharge pipe was outfitted with a 2 gpm flow restriction device ("restrictor") to maintain a constant flow rate throughout the test. During the test, a total of 1,363 L of water was extracted from the well. The well recovered to 45% of the original static water

level within 183 minutes.

Preliminary analysis of the drawdown curve revealed that the well would not be able to sustain constant rate pumping at 2 gpm. As such, it was determined that the well (in its current state) likely had insufficient yield for further testing.

#### TW-3

TW-3 was subjected to prolonged pumping on July 15, 2013, following the low-pressure hydraulic fracturing of the well. Based on the prolonged pumping of TW-3, it was determined that the well could likely yield 5+ gpm and would be suitable for testing in accordance with MOECC Procedure D-5-5.

## TW-4

TW-4 was subjected to a 130 minute pumping test on November 20, 2013 utilizing a submersible pump supplied by the well contractor with a 2 gpm flow restrictor. During the preliminary pumping test, a total of 984 L were extracted from the well. Water quality during the test was noted to be turbid prior to the cessation of pumping.

Analysis of the preliminary test curve revealed that the well likely wouldn't support prolonged pumping at 2 gpm. As such, TW-4 (in its current state) would not likely be suitable for future testing.

#### TW-5

TW-5 was subjected to a 120 minute pumping test on December 9, 2013 at a rate of 5 gpm utilizing a submersible pump supplied by the well contractor. A total of 2,271 L was extracted from the well during the pumping test. During the test, cascading was observed to occur at a depth of 43.6 m (143 ft) below the top of casing (btoc).

Analysis of the preliminary test curve revealed that the well would not be able to sustain a pumping rate of 5 gpm for a longer period of time. Subsequent recovery data analysis suggested that the maximum long-term yield of the well (in its current state) was likely to be approximately 1 gpm.

#### TW-6

TW-6 was subjected to a 120 minute pumping test on May 9, 2014. The well was

pumped at a rate of 4 gpm utilizing a submersible pump and flow restrictor provided by the well contractor. Approximately 1,817 L of water was extracted from the well during the preliminary pumping test.

Prior to the cessation of pumping, the water became turbid and cascading was observed to occur 33.5 m (110 ft) btoc. Preliminary analysis of the test curve revealed that the well (in its current state) likely wouldn't be able to sustain a pumping rate of 4 gpm for a long period of time. Subsequent recovery data analysis suggested that the maximum long-term yield of the well was likely to be approximately 2 gpm.

#### TW-7

TW-7 was subjected to a 140 minute pumping test on June 12, 2014. The well was pumped at a rate of 5 gpm utilizing a submersible pump and flow restrictor provided by the well contractor.

A total of 2,650 L was extracted from the well during the preliminary pumping test. During the test, the water appeared to be approaching stabilization, suggesting the long-term yield of the well was 5+ gpm.

# 4.6 High-Pressure Hydraulic Fracturing

Initially, all of the wells on the site were subjected to low-pressure hydraulic fracturing ("fracking") by White's Water Well Drilling Ltd., whereby a single packer was utilized to inject water into the well bore in an attempt to open additional fractures and/or "clean out" existing fractures to improve the well yield (Figure 11).

During the initial fracking program, the maximum pressure observed was approximately 150 psi. The method utilized a single packer set at a depth of 30 m (100 ft) in most instances. As such, the injection pressure would be distributed over a great length of the bore (i.e., >30 m). As such, the effectiveness of the hydraulic fracturing was limited. Only wells that exhibited turbid conditions initially had an increase in apparent yield from the low-pressure hydrofracturing.

Based on the preliminary pumping tests (discussed above), a number of wells were selected to be subjected to additional hydrofracturing utilizing a dual-packer arrangement (Figure 11). This approach could apply the injection pressure over a relatively short bore length (i.e., 6.1 m). Test wells TW-2, TW-4, TW-5 and TW-6 were subjected to high-pressure (often observed to be over 200 psi) hydraulic fracturing by Holmes Water Well Hydrofracturing. Each well was fracked across a number of different target depths in an attempt to widen existing fractures and open up (and/or clean out) any other fractures that may occur in the bedrock. With the exception of TW-2, sudden pressure drops during

the hydrofracturing were observed suggesting that the hydrofracturing had been successful.

Following the hydraulic fracturing, each well was subjected to aggressive pumping (albeit less than 50,000 L/day) to evacuate all of the injected water. Additional water bearing fractures and apparent well yields were noted during the pumping of each well and are briefly discussed in the following section.

# 4.7 Pumping Tests

### 4.7.1 General

Following the aggressive hydraulic fracturing program, five (5) on-site wells were selected for test pumping in accordance with MOECC Procedure D-5-5. The remaining on-site wells were utilized as observation wells. Prior to the pumping tests, an attempt was made to contact adjacent land owners through campground staff in order to obtain permission to monitor off-site (domestic) wells. Although an adjacent land owner did indicate the existence of a well on their property, it is understood that the wellhead was inaccessible and the property owner had indicated that the well was only used for non-potable supply (i.e., lawn watering).

Prior to each test, the target flow rate was determined based on the guidance provided by Section 4.3 of MOECC Procedure D-5-5. To summarize, D-5-5 states that each pumping test should be conducted such that the flow rate is not less than 18.75 litres per minute (L/min) or 4.1 imperial gallons (igal), unless specific occupancy parameters are contemplated. Therefore over a 6 hour period, the minimum volume needed to satisfy D-5-5 is 6,750 L. For those instances where our preliminary testing suggested that a well would not be capable of supporting the flow rate, D-5-5 makes an allowance for extending the pumping time to achieve the minimum total volume. In addition, D-5-5 states that the well must recover to within 95% of the original static water level within 24 hours of the beginning of the pumping test (i.e., so that the same pumping procedure could be repeated daily).

Based on the preceding, the duration and flow rate utilized for each pumping test was based on extracting the minimum volume of 6,750 L within a single 24 hour period.

Prior to the pumping tests, the pumped well and the observation wells were outfitted with programmable data logging pressure transducers ("loggers") to help facilitate frequent (i.e., every 1 minute) water level readings. To supplement the logger measurements, manual measurements were collected periodically throughout the pumping tests.

A detailed description of the formal well testing program has been included below. Copies of the pumping test curves are presented in Appendix E.

#### 4.7.2 TW-3

Based on the results of the preliminary pumping test, TW-3 was subjected to a pumping test at a rate of 0.32 L/s (4.2 igal) for 369 minutes on July 17, 2014.

The total volume of water extracted during the test was approximately 7,094 Litres. During the test, the well exhibited a maximum drawdown of 18 m, followed by stabilization of the pumping level. At the time of the test, TW-3 had a total available drawdown of about 90 m based on the difference between the static level and the driller's recommended pump setting.

The well recovered to within 95% of the initial static water level within 296 minutes of the cessation of pumping.

Based on the drawdown measured in the pumped well, the transmissivity is estimated at approximately 0.61 m²/day, utilizing a Cooper-Jacob analysis. Analysis of the recovery data for TW-3 suggests a lower transmissivity of approximately 0.36 m²/day, based on a Theis analysis. Unfortunately, the minimal drawdown observed at TW-7 (observation well) was insufficient for analysis.

Based on the 6-hour specific capacity (i.e., 0.02~L/s/m) and the total available drawdown (i.e., 90~m), the theoretical yield of TW-3 is on the order of 1.6 L/s (21 igpm). In reality, the achievable yield would likely be lower, as a result of well losses (inefficiency) and any boundary effects. Regardless, it is clear that the aquifer tapped by TW-3 has sufficient yield for domestic supply purposes and meets the D-5-5 criteria, without the need for supplementary water storage.

#### 4.7.3 TW-4

Based on the results of the high-pressure hydrofracturing, TW-4 was subjected to a pumping test at a rate of 0.19 L/s (2.5 igpm) for 605 minutes on December 2, 2014.

The total volume of water extracted during the test was approximately 6,897 litres. During the test, the well exhibited a maximum drawdown of 34 m. At the time of the test, TW-4 had a total available drawdown of about 67 m based on the difference between the static level and the driller's pump setting.

The well recovered to within 95% of the initial static water level within 513 minutes of the cessation of pumping.

The test curve is consistent with a semi-confined (leaky) aquifer condition. The observation well data indicate no discernable interference effects with any of the observation wells.

Based on the drawdown measured in the pumped well, the transmissivity is estimated at approximately  $8.6 \times 10^{-2} \text{ m}^2/\text{day}$ , based on a Cooper-Jacob analysis. Analysis of the recovery data for TW-4 suggests a slightly lower transmissivity of approximately  $7.5 \times 10^{-2} \text{ m}^2/\text{day}$ , based on a Theis analysis.

Based on the 10-hour specific capacity (i.e.,  $5.5 \times 10^{-3} \text{ L/s/m}$ ) and the total available drawdown (i.e., 67 m), the theoretical yield of TW-4 is on the order of 0.37 L/s (4.9 igpm). In reality, the achievable yield would likely be lower, as a result of well losses (inefficiency) and any boundary effects. Regardless, it is clear that the aquifer tapped by TW-4 has sufficient yield for domestic supply purposes, which meets the D-5-5 criteria. Supplementary water storage may be desired to satisfy short-term water demand needs (discussed in a following section).

#### 4.7.4 TW-5

Based on the results of the high-pressure hydrofracturing, TW-5 was subjected to a pumping test at a rate of 0.09 L/s (1.5 gpm) for 1451 minutes on February 19, 2015.

The total volume of water extracted during the test was approximately 7,835 Litres. During the test, the well exhibited a maximum drawdown of 24 m, followed by stabilization of the pumping level. At the time of the test, TW-5 had a total available drawdown of about 72 m based on the difference between the static level and the driller's recommended pump setting.

The test curve is consistent with a semi-confined (leaky) aquifer condition. The observation well data indicate no discernable interference effects with any of the observation wells.

The well recovered to within 95% of the initial static water level within 440 minutes of the cessation of pumping.

Based on the drawdown measured in the pumped well, the transmissivity is estimated at approximately  $0.1~\text{m}^2/\text{day}$ , based on a Cooper-Jacob analysis. Analysis of the recovery data for TW-5 suggests a lower transmissivity of approximately  $6.9~\text{x}~10^{-2}~\text{m}^2/\text{day}$ , based on a Theis analysis.

Based on the 24-hour specific capacity (i.e.,  $3.8 \times 10^{-3}$  L/s/m) and the total available drawdown (i.e., 72 m), the theoretical yield of TW-5 is on the order of 0.27 L/s (3.6 igpm). In reality, the achievable yield would likely be lower, as a result of well losses (inefficiency) and any boundary effects. Regardless, it is clear that the aquifer tapped by TW-5 has sufficient yield for domestic supply purposes, which meets the D-5-5 criteria. Supplementary water storage may be desired to satisfy short-term water demand needs (discussed in a following section).

#### 4.7.5 TW-6

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Following the hydrofracturing of TW-6, turbidity was persistent in the well. As such, a 24 hour pumping test was conducted at a relatively low rate 0.13 L/s (1.7 igpm) to determine if a lower pumping rate would resolve the water quality issue. Although it was determined that TW-6 was quite capable of supporting the pumping rate for 24 hours, prior to the cessation of pumping, small amounts of sediment were observed to be persistent in the groundwater. As such, it was recommended that the well be "cleaned-out" prior to retesting.

During the clean-out, a pump was lowered to the bottom of the well and the well was vigorously pumped at a high rate to evacuate all the water and sediment. It is likely that the sediment was introduced during the hydrofracturing process and settled into the bottom of the well. The well was allowed to recover and the process was repeated until sediment-free conditions persisted.

Based on the results of the attempted 24 hour pumping test, TW-6 was subjected to a pumping test at a rate of 0.19 L/s (2.5 igpm) for 606 minutes on September 3<sup>rd</sup>, 2014.

The total volume of water extracted during the test was approximately 6,908 Litres. During the test, the well exhibited a maximum drawdown of 55 m. At the time of the test, TW-6 had a total available drawdown of about 93 m based on the difference between the static level and the driller's recommended pump setting.

The initial part of the test curve is consistent with a semi-confined (leaky) aquifer condition. At a depth of approximately 33 m, the water level in the well dropped below a significant water-bearing fracture, causing an increased drawdown in the water level. A stable pumping level was not achieved prior to the cessation of pumping.

The well recovered to within 95% of the initial static water level within 585 minutes following the cessation of pumping.

Based on the drawdown measured in the pumped well, the transmissivity is estimated at approximately  $0.1~\text{m}^2/\text{day}$ , based on a Cooper-Jacob analysis. Analysis of the recovery data from the well suggests a slightly lower transmissivity of approximately  $0.07~\text{m}^2/\text{day}$ , based on a Theis analysis.

Based on the 10-hour specific capacity (i.e.,  $3.5 \times 10^{-3}$  L/s/m) and the total available drawdown (i.e., 95 m), the theoretical yield of TW-6 is on the order of 0.32 L/s (4.2 igpm). In reality, the achievable yield would likely be lower, as a result of well losses (inefficiency) and any boundary effects. Regardless, it is clear that the aquifer tapped by TW-6 has sufficient yield for domestic supply purposes, which meets the D-5-5 criteria, without the need for supplementary water storage.

#### 4.7.6 TW-7

Based on the results of the preliminary pumping test, TW-7 was subjected to a pumping test at a rate of 0.32 L/s (4.2 igpm) for 371 minutes on July 16, 2014.

The total volume of water extracted during the test was approximately 7,133 Litres. During the test, the well exhibited a maximum drawdown of 34 m, followed by stabilization of the pumping level. At the time of the test, TW-7 had a total available drawdown of about 44 m based on the difference between the static level and the driller's recommended pump setting.

The well recovered to within 95% of the original static water level within 98 minutes following the cessation of pumping.

The test curve is consistent with a semi-confined (leaky) aquifer condition. The observation well data indicate a barely discernable interference effect at TW-3 (separation = 137 m). No significant responses were observed at any of the other observation wells.

Based on the drawdown measured in the pumped well, the transmissivity is estimated at approximately  $0.34~\mathrm{m^2/day}$ , utilizing a Cooper-Jacob analysis. Analysis of the observation well data for TW-3 suggests a considerably higher transmissivity of approximately  $7.47~\mathrm{m^2/day}$  with an associated storativity of  $4.6~\mathrm{X}~10^{-5}$ , also based on a Cooper-Jacob analysis. However, given the distance between the pumped and observation wells, "scale effects" could be exaggerating the calculated transmissivity somewhat.

Based on the 6-hour specific capacity (i.e.,  $9.4 \times 10^{-3} \text{ L/s/m}$ ) and the total available drawdown (i.e., 44 m), the theoretical yield of TW-7 is on the order of 0.41 L/s (5.4 igpm). In reality, the achievable yield would likely be lower, as a result of well losses (inefficiency) and any boundary effects. Regardless, it is clear that the aquifer tapped by TW-7 has sufficient yield for domestic supply purposes, which meets the D-5-5 criteria, without the need for supplementary water storage.

## 4.7.7 Discussion

Domestic water usage is typically split into two main daily usage periods: i.e., one demand period in the morning and one in the evening. MOECC Procedure D-5-5 states that the average per-person water demand is 450 L per day. This is equivalent to a peak demand rate of 3.75 litres/minute for each person. The occupancy is generally considered to be *the number of bedrooms" plus one"*.

Taking into consideration the above, a four (4) bedroom home would theoretically have an occupancy of five (5) persons, resulting in a daily average water demand of 5 X 450 L/day = 2,250 L/day. If the day is split according to a morning and evening peak usage period, each period would require approximately 1,125 L. These higher usage periods would most

often occur within a relatively short time frame of 1 to 2 hours.

To comply with this minimum requirement of Procedure D-5-5, the test wells should be capable of meeting the above criteria. For wells capable of meeting the average daily demand but not able to meet the peak, short-term demand, lower pumping rates can be acceptable, provided supplementary water storage is available in the system.

Despite the challenging water supply conditions at the site, all of the tested wells satisfied the requirements of MOECC Procedure D-5-5.

Based on the results of the pumping test data, it appears that a combined strategy of drilling and high-pressure or targeted hydraulic fracturing has been quite successful at this site. As such, it is expected that all future wells in the development should follow a similar drilling and hydraulic fracturing program when initial well yields are estimated to be below 0.09 L/s (1.2 igpm).

Most of the water bearing fractures occur at a depth of approximately 46 m (150 ft), however, deeper wells have revealed the possibility of deeper (> 60 m) supplementary water bearing fractures. These deeper fractures do not appear to impair water quality. It is expected that future wells on the site will require similar construction to maximize yield and storage in addition to hydraulic fracturing.

Supplementary water storage will likely be necessary for the majority of the proposed lots on the site, however, the need for supplementary storage should be determined on an individual basis, based on the well conditions and homeowners' water supply requirements.

#### 4.8 Interference Assessment

Monitoring data obtained during the pumping tests indicate that water takings within the proposed subdivision will likely create barely discernable drawdown interference effects in nearby wells. During the pumping tests, only one observed interference effect of approximately 0.5 m occurred in a nearby well. Given that the majority of the residences neighbouring the site are known to be on lake intake systems, the development would be considered "remote" from other groundwater users.

For future supply wells, the available drawdown typically expected will be substantial, based on the test well data (i.e., range 44 m to 93 m). Cumulative interference effects should not exceed a few metres within the development, given the very low density. Interference on that scale should be quite manageable and should not be expected to represent a potential impact.

# 4.9 Water Quality

#### 4.9.1 General

Water quality samples were collected from each of the test wells at preselected intervals throughout the formal pumping tests. Samples were forwarded to Caduceon Environmental Laboratories in Ottawa/Kingston for chemical and bacteria analysis. In addition, field water quality measurements for pH, conductivity, temperature, total dissolved solids (TDS) and turbidity were taken periodically throughout the pumping tests. A summary of the water quality results and the laboratory certificates are presented in Appendix F.

Overall, water quality in all of the test wells is reasonably good and within the expected range of values for groundwater in the study area. All of the test wells meet the *health* related quality criteria of D-5-5 and the Ontario Drinking Water Quality Standards (ODWQS).

A brief summary of the water quality results for each well is presented below.

#### 4.9.2 TW-3

Groundwater samples were collected from TW-3 during the 6 hour pumping test. One sample was collected 240 minutes into the test and the last sample was collected just prior to the cessation of pumping.

The laboratory data indicate that groundwater from TW-3 is generally characterised by a slightly high hardness concentration of 194 mg/L, occurring above the Ontario Drinking Water Standards (ODWQS) ideal range of 80 mg/L to 100 mg/L. Hardness is an aesthetic parameter and does not pose any threat to human health, however, at elevated concentrations could cause staining of fixtures. It is expected that the hardness could be readily treated through a water softener. Treatment to reduce hardness is not mandatory.

TW-3 also exhibits high iron and manganese concentrations, occurring at 3.77 mg/L and 0.522 mg/L respectively. Both exceed their respective ODWQS objectives of 0.3 mg/L and 0.05 mg/L respectively. At these concentrations, treatment to reduce iron and manganese will likely be desirable. Both are considered treatable through a combination of aeration and filtration.

Iron and manganese are aesthetic objectives and should not have any impact on human health. However, at these concentrations, iron and manganese will likely form precipitates that can cause staining of fixtures and a slight bitter taste in the water. These precipitates undoubtably contributed to the laboratory's high turbidity value of 25.2 NTU. Turbidity measured in the field, however, was

observed to be 2.41 NTU at the wellhead when stabilised utilizing a dilute acid solution. Future use of TW-3 will likely improve well development and will further reduce any remaining well-related turbidity.

Bacteriological samples were collected approximately at the mid-point and just prior to the cessation of the pumping test. Prior to sampling, an in-field test was conducted to verify the absence of residual chlorine. The laboratory results indicate counts of 0 cfu/100 ml for Total Coliform and 0 cfu/100 ml for E. Coli, indicating acceptable quality.

### 4.9.3 TW-4

Groundwater samples were collected from TW-4 during the ten (10) hour pumping test and forwarded to Caduceon Environmental Laboratories. One sample was collected 300 minutes into the test and the last sample was collected just prior to the cessation of pumping.

The groundwater is generally characterised by a slightly elevated hardness concentration of 159 mg/L occurring marginally above the Ontario Drinking Water Standards' (ODWQS) ideal range of 80 mg/L to 100 mg/L. Hardness is an aesthetic parameter and does not pose any threat to human health, however, at elevated concentrations could cause staining of fixtures. It is expected that the hardness could be readily treated through a water softener. Treatment to reduce hardness is not mandatory.

TW-4 exhibits high iron and manganese concentrations, occurring at 7.79 mg/L and 0.596 mg/L respectively. Both occur above their respective ODWQS objectives of 0.3 mg/L and 0.05 mg/L respectively. At these concentrations, treatment to reduce iron and manganese will likely be desirable, similar to TW-3. Both are considered treatable through a combination of aeration and filtration.

Iron and manganese are aesthetic objectives and should not have any impact on human health. However, at these concentrations, iron and manganese will likely form precipitates that can cause staining of fixtures and a slight bitter taste to the water. These precipitates undoubtably contributed to the laboratory's high turbidity value of 75.2 NTU. Turbidity measured in the field, however, was observed to be 3.85 NTU at the wellhead. Future use of TW-4 will likely improve well development and should reduce any remaining well-related turbidity.

Bacteriological samples were approximately half-way through and just prior to the cessation of the ten (10) hour pumping test. Prior to sampling, an in-field test was conducted to verify the absence of residual chlorine. The laboratory results indicate counts of 0 cfu/100 mL for Total Coliform and 0 cfu/100 mL for E. Coli, at the 5 hour mark of the test. The final sample collected prior to the cessation of pumping had

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12 cfu/100 mL for Total Coliform.

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Near the end of the pumping test, the water was observed to have black sediment and other organic material in the discharge. As such, it is believed that sediment remaining in the bottom of the well bore from the hydrofracturing process was liberated once the water level was sufficiently drawn down. As such, it is believed that the water sample collected at the end of the pumping test was not representative of the water typically produced by this well.

Similar to TW-6 (discussed above), TW-4 was subjected to the same pumping procedure to effectively clean out the well. During the clean out procedure, TW-4 was observed to have a large amount of sediment in the bottom of the well. Therefore, TW-4 was subjected to repeated pumping over several days. During that time, sediment and organic debris was observed in the discharge water that resembled lake sediments. On May 7<sup>th</sup>, 2015, a water sample was collected from TW-4 and forwarded to Caduceon Environmental Laboratories for re-analysis of major ion and bacteriological parameters.

The results of the water quality analysis indicate counts of 0 cfu/100 mL for Total Coliform and 0 cfu/100 mL for E. Coli. In addition, the DOC was reported to be 4.9 mg/L, just below the ODWQS limit of 5.0 mg/L. Similar to the iron and manganese concentrations, the DOC concentration is expected to improve with use. Based on these results, it appears that TW-4 has acceptable water quality.

#### 4.9.4 TW-5

Groundwater from TW-5 exhibits a moderate total dissolved solids (TDS) content with a somewhat reduced hardness in comparison to other on-site wells (i.e., occurring below the ideal range of 80 mg/L to 100 mg/L). It is apparent that some natural softening (i.e., ion exchange) is occurring in the aquifer, as calcium has a lower concentration while sodium has a higher concentration, when compared to most of the other on-site wells.

Although well within the aesthetic objective of 200 mg/L, the sodium concentration in TW-5 occurs above the warning level of 20 mg/L. As indicated in the Ontario Drinking Water Quality Standards (ODWQS), it is generally recommended that the local Medical Officer of Health be notified when the sodium concentration exceeds 20 mg/L so that this information may be communicated to local physicians for their use with patients on sodium restricted diets.

TW-5 exhibits slightly high iron and manganese concentrations, occurring at 0.355 mg/L and 0.087 mg/L respectively. Both occur marginally above their respective objectives. Iron and manganese are aesthetic objectives and therefore should not have any impact on human health. However, at these concentrations,

iron and manganese could form precipitates that can cause staining of fixtures and a slight bitter taste to the water. At these concentrations, iron and manganese are considered treatable through a combination of aeration and filtration.

Bacteriological samples were collected half-way through and just prior to the cessation of the 24 hour pumping test. Prior to sampling, an in-field test was conducted to verify the absence of residual chlorine. The laboratory results indicate counts of 0 cfu/100 ml for Total Coliform and 0 cfu/100 ml for E. Coli, indicating acceptable quality.

#### 4.9.5 TW-6

Groundwater from TW-6 exhibits a moderate total dissolved solids (TDS) content with a somewhat reduced hardness in comparison to other on-site wells (i.e., occurring below the ideal range of 80 mg/L to 100 mg/L). Similar to TW-5, it is apparent that some natural softening (i.e., ion exchange) is occurring in the aquifer, as calcium has a lower concentration while sodium has a higher concentration, when compared to most of the other on-site wells.

Although well within the aesthetic objective of 200 mg/L, the sodium concentration in TW-6 occurs above the warning level of 20 mg/L. As indicated in the Ontario Drinking Water Quality Standards (ODWQS), it is generally recommended that the local Medical Officer of Health be notified when the sodium concentration exceeds 20 mg/L so that this information may be communicated to local physicians for their use with patients on sodium restricted diets.

TW-6 exhibits high iron and manganese concentrations, occurring at 5.37 mg/L and 0.469 mg/L respectively. Both occur above their respective ODWQS objectives. Iron and manganese are aesthetic parameters. At these concentrations, iron and manganese will form precipitates that can cause staining of fixtures and a slight bitter taste to the water. The iron and manganese are considered treatable through a combination of aeration and filtration.

Water from TW-6 differs from all the other site wells as pH and fluoride are elevated over their respective ODWQS limits. While the pH was observed to decline with pumping, fluoride appears to be stable at a concentration of approximately 2.0 mg/L. Similar to sodium, it is generally recommended that the local Medical Officer of Health be notified when fluoride occurs at a concentration between 1.5 mg/L and 2.4 mg/L. This information may be communicated to the public to raise awareness about excessive fluoride use. As such, any future owner should be advised not to consume any fluoride from additional sources.

It is expected that the pH will further decline with use, however, the current pH, (i.e., above 8.5) can cause mineral incrustations and a bitter taste to occur. If pH

remains above 8.5, the pH can be adjusted through treatment (i.e., by adding a neutralizer).

Bacteriological samples were collected half-way through and just prior to the cessation of the 10 hour pumping test. Prior to sampling, an in-field test was conducted to verify the absence of residual chlorine. The laboratory results indicate counts of 0 cfu/100 ml for Total Coliform and 0 cfu/100 ml for E. Coli, indicating acceptable quality.

#### 4.9.6 TW-7

Groundwater samples were collected from TW-7 during the 6 hour pumping test. The first sample was collected 180 minutes into the test and the last sample was collected just prior to the cessation of pumping. The groundwater is generally characterised by an slightly high hardness concentration of 168 mg/L occurring above the Ontario Drinking Water Standards (ODWQS) ideal range of 80 mg/L to 100 mg/L. Hardness is an aesthetic parameter and does not pose any threat to human health, however at elevated concentrations could cause staining of fixtures. It is expected that the hardness could be readily treated through a water softener.

TW-7 exhibits high iron and manganese concentrations, occurring at 5.37 mg/L and 0.469 mg/L respectively. Both iron and manganese occur above their respective aesthetic objectives. At these concentrations, iron and manganese will likely form precipitates that can cause staining of fixtures and a slight bitter taste to the water. These precipitates undoubtably contributed to the laboratory's high turbidity value of 60.9 NTU. Turbidity measured in the field, however, was observed to be 0.89 NTU at the wellhead, when measured in an acid stabilized sample. Future use of TW-7 will likely improve well development and reduce any remaining well-related turbidity. At the reported concentrations, iron and manganese are considered treatable through a combination of aeration and filtration.

Bacteriological samples were collected half-way through and just prior to the cessation of the 6 hour pumping test. Prior to sampling, an in-field test was conducted to verify the absence of residual chlorine. The laboratory results indicate counts of 0 cfu/100 ml for Total Coliform and 0 cfu/100 ml for E. Coli, indicating acceptable quality.

### 4.9.7 Discussion

Overall, the groundwater quality at the site is generally good and satisfies the requirements of MOECC Procedure D-5-5. Groundwater at the subject site is characterized by elevated levels of iron and manganese, both aesthetic parameters. It is likely that water from some wells will require rigorous treatment by a

combination of aeration and filtration.

Softening may not be desired in all wells, as naturally soft water (i.e., below 80 mg/L) will occur in some locations. It is possible that wells tapping a marble-type aquifer (i.e., carbonate source) may exhibit natural softening through ion exchange processes.

Wells TW-4 and TW-6 exhibited persistent turbidity issues due to sediment introduced during the hydrofracturing process despite long-term pumping following those activities. It is believed the sediment originated from impurities in the water used in fracking. As such, for any future wells subjected to hydraulic fracturing, it will be important to fully clean the well by evacuating the entire water column until sediment-free conditions persist (similar to the procedure used on TW-6 and TW-4). Water quality samples should be collected following the clean-out activities to verify acceptable water quality.

## 5.0 Wastewater Treatment

# 5.1 Existing Wastewater Treatment

A legacy central sewage disposal system is currently situated in the south-eastern portion of the subject site, within approximately 25 m of Stony Lake (Figure 2). The system services 87 trailer units, a common washroom with shower facilities and a central laundry facility. A trailer pump-out is located in proximity to the washroom facilities. We estimate the theoretical sewage flow rate of the campground to be approximately 45,000 L/day, based on Code and Guide for Sewage Systems (Ontario Building Code - OBC). However, the system appears to be undersized for that flow rate.

The system includes a fully in-ground effluent disposal bed. However, a test pit excavated immediately east of the bed revealed that bedrock could occur at a depth of only 0.49 m in the area. The OBC states that the bottom of an absorption trench must be at least 0.9 m "above the high ground water table, rock or soil with a percolation time greater than 50 minutes". As such, it is unclear whether the existing disposal bed would meet this requirement and/or whether the effluent is being sufficiently treated.

The public washroom and laundry facilities are directly connected to the communal sewage system. However, none of the trailer sites are serviced directly by the system. Instead, each trailer must be manually pumped out through a pumping station that is connected to the works. As such, the current system requires a considerable amount of wastewater handling.

The sewage system does not operate under an Environmental Compliance Approval (ECA) or Certificate of Approval (C of A) and likely predates any such requirements. The system does not incorporate any additional treatment beyond what is provided by the septic

tanks.

In addition to the central sewage system, a number of Class 1 sewage systems (outhouses) also occur throughout the site, representing an unknown proportion of the site's total sewage flows. Those units do not provide any treatment. The locations of Class 1 sewage systems observed on the site are included in Figure 2.

Based on its age, location, construction and the absence of nutrient reduction treatment, the existing central sewage system would not be appropriate for use to service the proposed development. As such, the proposed development will be serviced by modern, properly constructed individual sewage systems on each lot. The private sewage systems will be substantially set-back from the Stony Lake shoreline.

# 5.2 Nitrate Impact Assessment

#### 5.2.1 General

1

The principal impact of the proposed development on groundwater resources is related to the introduction of septic effluent into the shallow flow zone from the proposed individual tile bed systems.

Within the effluent, nitrate is considered the critical contaminant as elevated nitrate concentrations are linked to infant methaemoglobinaemia (nitrate poisoning). To protect groundwater resources, the Ministry of Environment's Procedure D-5-4 sets the maximum allowable nitrate concentration at the site boundary to be 10 mg/L (also the Ontario Drinking Water Standard). The nitrate impact assessment is therefore conducted to verify that this limit is not exceeded.

Naturally occurring bacteria and soil interaction mechanisms can, and usually do, result in nitrate being renovated. However, Procedure D-5-4 acknowledges *dilution* as the principal attenuation mechanism<sup>1</sup> to be used to predict future nitrate concentrations as a result of subdivision development.

For the purpose of assessing the subject site, three (3) local drainage areas have been designated in the development area. Each area drains into one of three surface water features. The drainage areas are herein referred to as the "Jack Creek Drainage Area", "Stony Lake Drainage Area" and "Tributary of Stony Lake Drainage Area". Each are identified on Figure 6.

Procedure D-5-4 also acknowledges monitoring-based assessments and other specialized assessment forms, primarily for use in areas where there is scientific precedent.

# 5.2.2 Development Area Available Dilution

In order to estimate the availability of sustained dilution from direct recharge, the following calculation has been considered:

$$D_w = A \times W_s \times I_f$$

where,

 $D_w$ = Available dilution water A = Net dilution area  $W_s$ = Water surplus  $I_f$  = Infiltration factor<sup>2</sup>

For this assessment, the average regional water surplus<sup>3</sup> is estimated to be 287.8 mm.

Only lands that are proposed to be developed were considered as part of the following calculations.

## Jack Creek Drainage Area

The Jack Creek Drainage Area comprises of 7.56 ha (18.7 acres) of land area. Soils within this area are dominated by sand of variable thickness. As such, the soil component of the infiltration factor is based on the percentage of the area that contains over 20 cm of overburden thickness (i.e., 80%) as illustrated on Figure 6:

Soil factor = 0.31 (medium sand factor  $0.39 \times 80\%$ )

Slope factor = 0.15 (rolling terrain)

Cover factor = 0.15 (mostly grass and tree covering)

Total = 0.61

Substituting the various factors into the above expression and accounting for impermeable surfaces (i.e. 5% of lot area) yields the following total dilution availabilities for the remaining (7.18 ha) area:

 $7.18 \text{ ha X } 287.8 \text{ mm/yr X } 0.61 = 12,605 \text{ m}^3/\text{yr } (34.5 \text{ m}^3/\text{day})$ 

Infiltration factor calculation method: From MOEE Hydrogeological Technical Information Requirements for Land Development Applications, April 1995

Based on average water surplus for Peterborough, Lakefield and Apsley stations, as listed in: Department of Transport "Average Annual Water Surplus in Canada" (1967).

## Stony Lake Drainage Area

The Stony Lake Drainage Area comprises of 7.35 ha (18.2 acres) of land area. Soils within this area are also comprised of sand of variable thickness. As such, the soil component of the infiltration factor is based on the percentage of the area that contains over 20 cm of overburden thickness (i.e., 90 %) as illustrated on Figure 6:

Soil factor = 0.35 (medium sand factor  $0.39 \times 90\%$ )

Slope factor = 0.15 (gently sloping terrain) Cover factor = 0.13 (mostly grass covering)

Total = 0.63

Substituting the various factors into the above expression and accounting for impermeable surfaces (i.e. 5% of lot area) yields the following total dilution availabilities for the remaining (7.0 ha) area:

 $7.00 \text{ ha X } 287.8 \text{ mm/yr X } 0.63 = 12,692 \text{ m}^3/\text{yr } (34.8 \text{ m}^3/\text{day})$ 

## Tributary of Stony Lake Drainage Area

The Tributary of Stony Lake Drainage Area comprises an area of 6.47 ha (16.0 acres). Soils within this drainage area are comprised of sand of variable thickness. As such, the soil component of the infiltration factor is based on the percentage of the area that contains over 20 cm of overburden thickness (i.e., 80 %) as illustrated on Figure 6:

Soil factor = 0.33 (medium sand  $0.39 \times 85\%$ )

Slope factor = 0.15 (rolling terrain)

Cover factor = 0.15 (mostly grass and tree covering)

Total = 0.63

Substituting the various factors into the above expression and accounting for impermeable surfaces (i.e. 5% of lot area) yields the following total dilution availabilities for the remaining (6.15 ha) area:

 $6.15 \text{ ha X } 287.8 \text{ mm/yr X } 0.63 = 11{,}151 \text{ m}^3\text{/yr } (30.6 \text{ m}^3\text{/day})$ 

The combined total estimated available dilution for the three (3) drainage areas is approximately 36,452 m<sup>3</sup>/yr (99.9 m<sup>3</sup>/day).

## 5.2.3 Development Area Impact Evaluation

Lot density is determined through a simple mass-balance calculation which considers the following factors:

- available dilution (total 99.9 m³/day, see above)
- total volume of septic effluent (1,000 L/day)
- baseline nitrate levels in the supply aquifer (assumed to be 0.01 g/day)
- nitrate input from septic systems (40 g/day)

To determine the total number of supportable lots, an evaluation of each drainage area described above) has been conducted:

Jack Creek Drainage Area

$$[Nitrate] = \underbrace{(septic\ input\ + supply\ aquifer\ input)} \bullet \ No.\ of\ Lots}_{available\ dilution\ + \ volume\ of\ septic\ effluent}$$
 
$$[Nitrate] = \underbrace{(40\ g/day\ + 0.01\ g/day)\ X\ 11)}_{34.5\ m^3/day\ + (1\ m^3/day\ X\ 11\ lots)}$$
 
$$= 9.67\ mg/L$$

Therefore, based on the above analysis, a total of 11 lots can be supported by the Jack Creek Drainage Area while remaining in compliance with MOECC Procedure D-5-4.

Stony Lake Drainage Area

$$[Nitrate] = \underbrace{(septic\ input + supply\ aquifer\ input)} \bullet \ No.\ of\ Lots}_{available\ dilution\ +\ volume\ of\ septic\ effluent}$$
 
$$[Nitrate] = \underbrace{(40\ g/day\ +\ 0.01\ g/day)\ X\ 9}_{34.8\ m^3/day\ +\ (1\ m^3/day\ X\ 9\ lots)}$$
 
$$= 8.22\ mg/L$$

Therefore, based on the above analysis, *a total of 9 lots can be supported* by the Stony Lake Drainage Area while remaining in compliance with MOECC Procedure D-5-4.

Tributary of Stony Lake Drainage Area

$$[Nitrate] = \underbrace{(septic\ input + supply\ aquifer\ input)} \bullet \ No.\ of\ Lots$$
 
$$available\ dilution + volume\ of\ septic\ effluent}$$
 
$$[Nitrate] = \underbrace{(40\ g/day + 0.01\ g/day)\ X\ 10}_{30.6\ m^3/day + (1\ m^3/day\ X\ 10\ lots)}$$
 
$$= 9.85\ mg/L$$

Therefore, based on the above analysis, *a total of 10 lots can be supported* by the Tributary of Stony Lake Drainage Area while remaining in compliance with MOECC Procedure D-5-4.

From the preceding analyses, the proposed development area should be capable of supporting the development of 30 privately serviced lots while remaining in compliance with MOECC Procedure D-5-4, based on conventional private sewage systems.

## 6.0 Servicing Considerations

#### 6.1 Private Wells

The results of this study support the construction and sustainable use of private, individual wells to supply potable water for each of the proposed subdivision lots. Seven (7) test wells have been constructed on the subject site to verify those conditions. Five (5) are suitable for future supply use and one is currently being utilized by a residence on the site.

Figure 12 illustrates the proposed/recommended locations for future private wells within the development. These locations are based on accommodating minimum separation distances and the juxtaposition of proposed building envelopes and proposed septic systems.

As a result of the generally thin, permeable soils, the site would be considered moderately sensitive with respect to hydrogeological conditions. As such, the recommended separation distances between wells and septic systems has been maximized (where practical).

#### 6.2 Sewage Disposal

The proposed development is to be serviced by conventional type sewage disposal systems (i.e., individual septic tank and tile bed at each lot). The existing legacy central sewage system will not be utilized and will be decommissioned.

In addition to obtaining data regarding the shallow aquifer, the soils information obtained during the terrain mapping is also valuable for determining the type of tile bed system most appropriate for the site (e.g., fully raised, partially raised, or in-ground). Primarily due to the presence of bedrock near surface at the site, and to a lesser extent, due to perched shallow water-table conditions in some areas, we expect that future sewage systems will need to be constructed with fully raised tile beds.

Figure 12 illustrates the recommended locations for a sewage disposal bed area.

#### 7.0 Conclusions and Recommendations

- 7.1 This Hydrogeological and Site Servicing Study has been prepared in support of a proposed seasonal residential condominium development at the current site of Pilgrim's Rest Campground in the Township of North Kawartha. The primary objectives of this report are to present a summary of the site conditions, provide an evaluation of groundwater supply potential and present an impact assessment to verify the sustainability of privately serviced lots while complying with MOECC Procedures D-5-4 and D-5-5.
- 7.2 The campground currently utilizes an intake situated at the mouth of Jack Creek to supply a communal water works. This system will not be utilized for the proposed development and will be decommissioned.
- 7.3 The campground currently utilizes a large central sewage disposal system situated within approximately 25 m of Stony Lake. The legacy system services 87 trailer units, a common washroom with shower facilities and a central laundry facility. A trailer pump-out is located in proximity to the washroom facilities. The current disposal system requires a considerable amount of wastewater handling.

The system includes a fully in-ground effluent disposal bed that may be undersized for the theoretical camp flows. The sewage system does not operate under an Environmental Compliance Approval (ECA) or Certificate of Approval (C of A) and likely predates any such requirements. Other than provided by the septic tanks, the system incorporates no additional treatment.

In addition to the central sewage system, a number of Class 1 sewage systems (outhouses) also occur throughout the site, representing an unknown proportion of the site's total sewage flows. Those units do not provide any treatment. The locations of Class 1 sewage systems observed on the site are included in Figure 2.

Based on its age, location, construction and the absence of nutrient reduction treatment, the existing central sewage system would not be appropriate for use to service the proposed development. Therefore, proposed development will result in decommissioning of all the existing sewage (Class 1 and 4) systems on-site.

7.4 It is proposed that the development will be individually serviced with a well and subsurface sewage disposal system. Replacement of the trailer park with a modern, less concentrated seasonal residential development, serviced by properly constructed private sewage systems that are well set-back from the lake, with less concentrated waterfront usage, should be viewed as beneficial with respect to protection of Stony Lake.

- 7.5 A series of twenty-five (25) test pits and six (6) hand-auger holes were excavated on the property to explore the on-site geology, soil, terrain and shallow groundwater conditions. All of the test pits and hand-auger holes intersected a continuous sandy mantle (SP-type) in areas devoid of granite outcrops.
- 7.6 According to MOECC well records, statistical results indicate that an adequate quantity of water supplies are generally available in the study area. These data also suggest that obtaining a successful well (i.e., well yield of 5 gpm or greater) may require more than one attempt. At the very least, the data indicate that groundwater supply conditions are variable, as would be expected from the geological setting.
- 7.7 Seven (7) wells have been constructed on the subject site to determine the aquifer conditions. Five (5) of these wells have been subjected to pumping tests and water quality sampling, in accordance with the requirements of MOECC Procedures D-5-5. Based on those tests, five (5) of the test wells are suitable for future supply use and one is currently being utilized by a residence on the site. Despite the challenging groundwater conditions, the test well evaluations confirm that sufficient groundwater should be available to support the proposed units (lots).

Based on our overall findings, it is our recommendation that each proposed lot should be serviced for water supply through construction of a drilled well meeting the requirements of Ontario Regulation 903, as amended. Dug or bored wells are *not* appropriate for this site. Although there is a virtually unlimited supply of water available from the lake, a drilled well is the preferred alternative. A lake-based, potable water supply system is not currently considered for any proposed lot.

- 7.8 Based on the test well data, we conclude that the proposed lots can obtain an adequate supply of acceptable quality water from a combination of two (2) water bearing zones in the Precambrian bedrock. These aquifers consist of fractured granitic gneiss and/or marble that may be formational contacts. The water bearing zones may require aggressive hydrofracturing to develop an acceptable yield. Hydrofracturing conducted during this study was very successful. The drilling contractor should be advised of this condition prior to any future well construction.
- 7.9 The test well data suggest that yields can be variable and some wells may have lower than ideal yields, not unlike most rural Ontario settings. As such, it is apparent that a small percentage of future lot owners may not obtain an acceptable yield upon initial well construction. In those cases, the use of supplementary water storage may be appropriate to compensate for a lower well yield. In other instances, it may be necessary to construct more than one well to obtain an acceptable quantity of groundwater.

7.10 As a result of the variable groundwater conditions at the subject site, we are recommending that a *Well Certification Program* be implemented at this site. The program will require that prior to issuance of a building permit, a well be constructed under the supervision of, and tested by, a Qualified Person (P. Geo. or P. Eng.) who will certify in writing that a drilled well has been constructed, meeting the minimum construction, water demand and water quality requirements as set forth herein. The well "certification report" shall be submitted to the municipality as part of the Building Permit application. The requirements of the Program are outlined in Appendix G.

As a general guide, unless the Qualified Person recommends otherwise, new drilled wells should be constructed at the locations illustrated on the accompanying Recommended Lot Servicing Plan, Figure 12. Figure 12 illustrates that each of the proposed lots has ample room to support the construction of a residence, a drilled well and a fully-raised conventional sewage disposal system based on an anticipated sewage flow of 2,000 L/day.

Notwithstanding the contents of this study, well certification reports should be prepared and provided for the future owners of TW-3, TW-4, TW-5, TW-6 and TW-7 so that purchasers of those lots will have the benefit of any additional recommendations that might be presented therein.

- 7.11 Heat pump feasibility has not been investigated as part of this study. The use of open loop heat pump systems is not recommended for this site. Any such open loop heat pump installations should only be considered if a hydrogeologist has determined that such systems can be utilized without compromising groundwater availability and quality.
- 7.12 Based on the results of our on-site geology investigation and terrain mapping, an assessment of the nitrate impact has been completed, relying entirely on natural attenuation of septic effluent by dilution to achieve compliance with MOECC Procedure D-5-4. The assessment was conducted on three (3) separate drainage areas within proposed development area. The drainage areas were referred to as "Jack Creek Drainage Area", "Tributary of Stony Lake Drainage Area" and "Stony Lake Drainage Area".

Based on the results of the nitrate impact assessment, the proposed development should be able to support thirty (30) residences and still comply with MOECC Procedure D-5-4.

7.13 The development is expected to have no significant impact on water quality in Stony Lake and should represent a net improvement by removing the current trailer park. The presence of "acidic" noncalcareous silicate derived sands mantling the site will help attenuate (through sorption) the phosphorus contained within the effluent from the

sewage systems<sup>4</sup>.

As a precaution, it is recommended that a 30 m setback be imposed which will preclude the construction of septic systems close to the lake.

The use of proprietary wastewater treatment systems is also recommended as a means of further reducing nutrient output and as a means of reducing the system footprints. This can be achieved by implementing any of the tertiary treatment systems that are preapproved under the Ontario Building Code (OBC).

\* end of report \*

Yours truly, **Oakridge Environmental Ltd.** 

## Original Signed By

Dan MacIntyre, B.Sc. Senior Hydrogeological Technician **Original Signed By** 

Brian R. King, P. Geo. Principal

Robertson, W.D. <u>Enhanced Attenuation of Septic System Phosphate in Noncalcareous Sediments</u>. Vol. 41, No. 1 - Ground Water - January - February 2003. Pg. 48-56.

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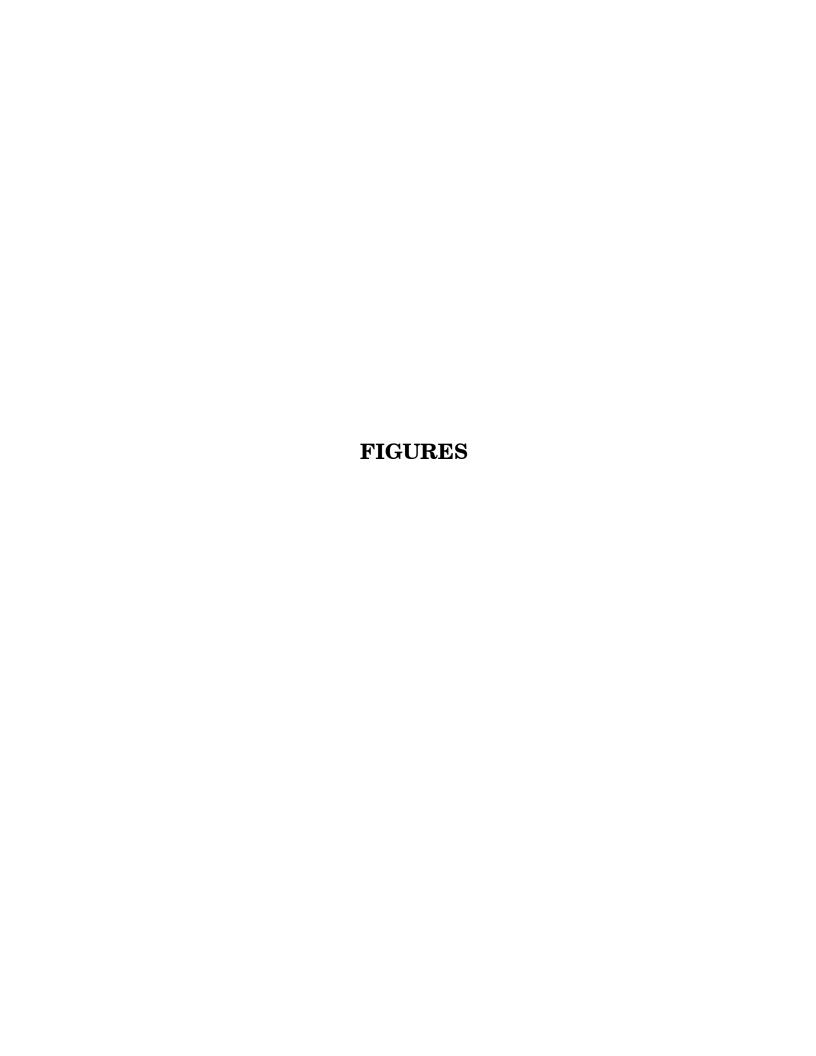
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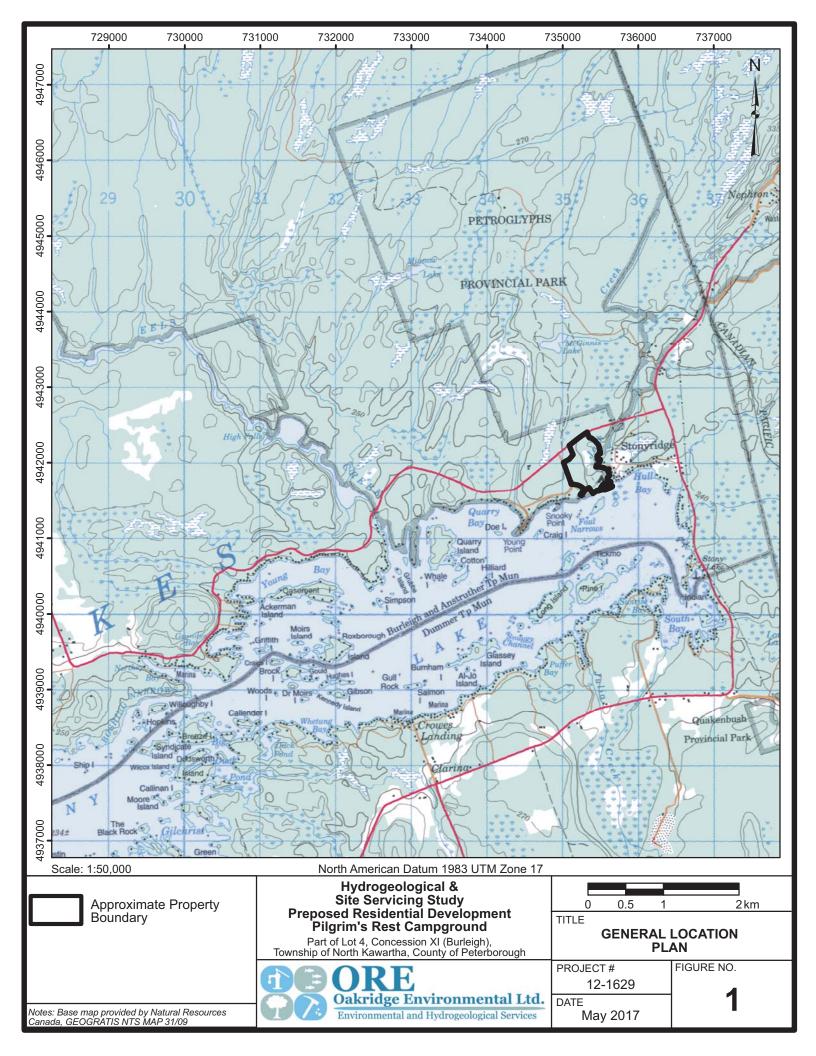
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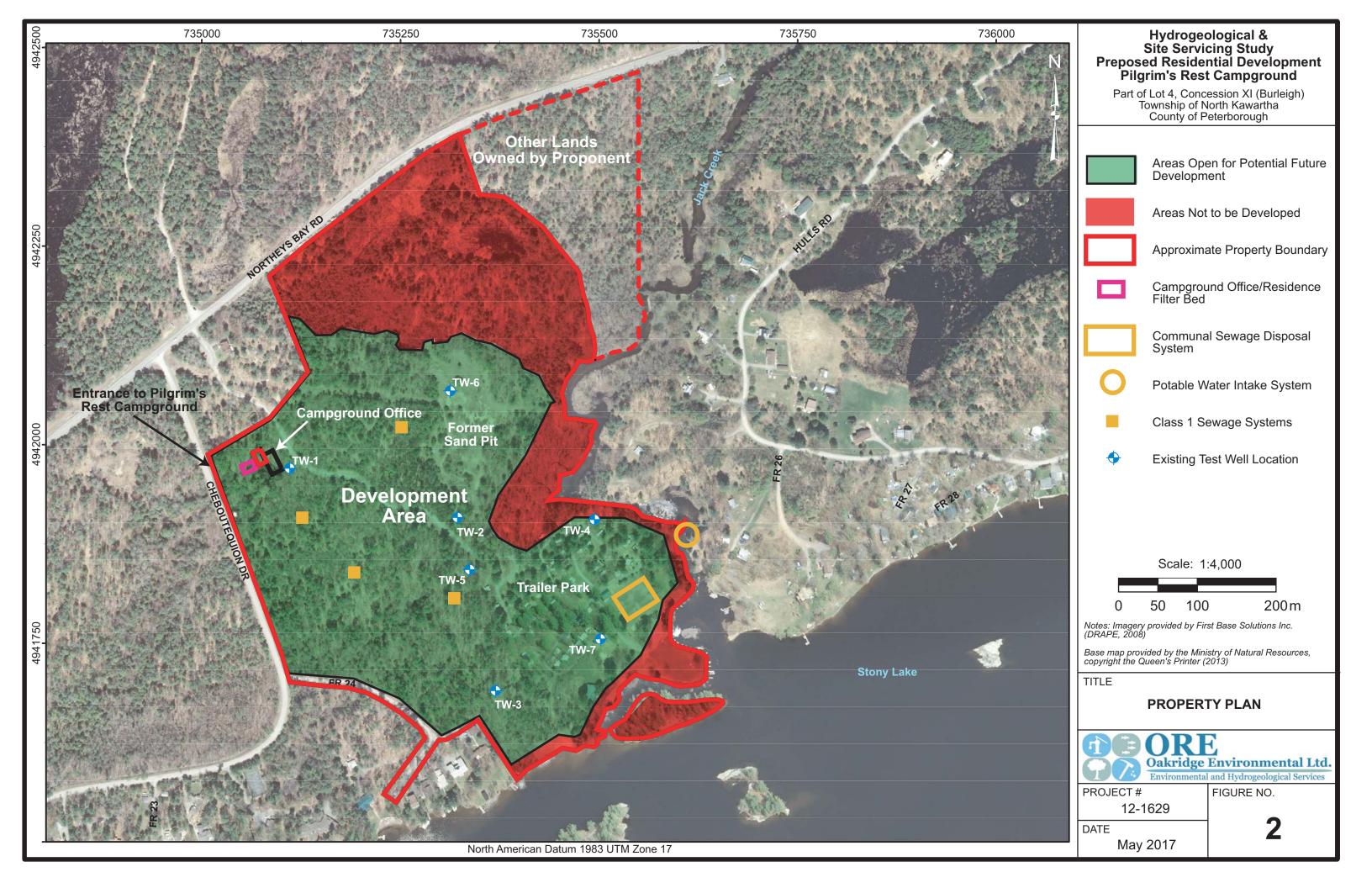
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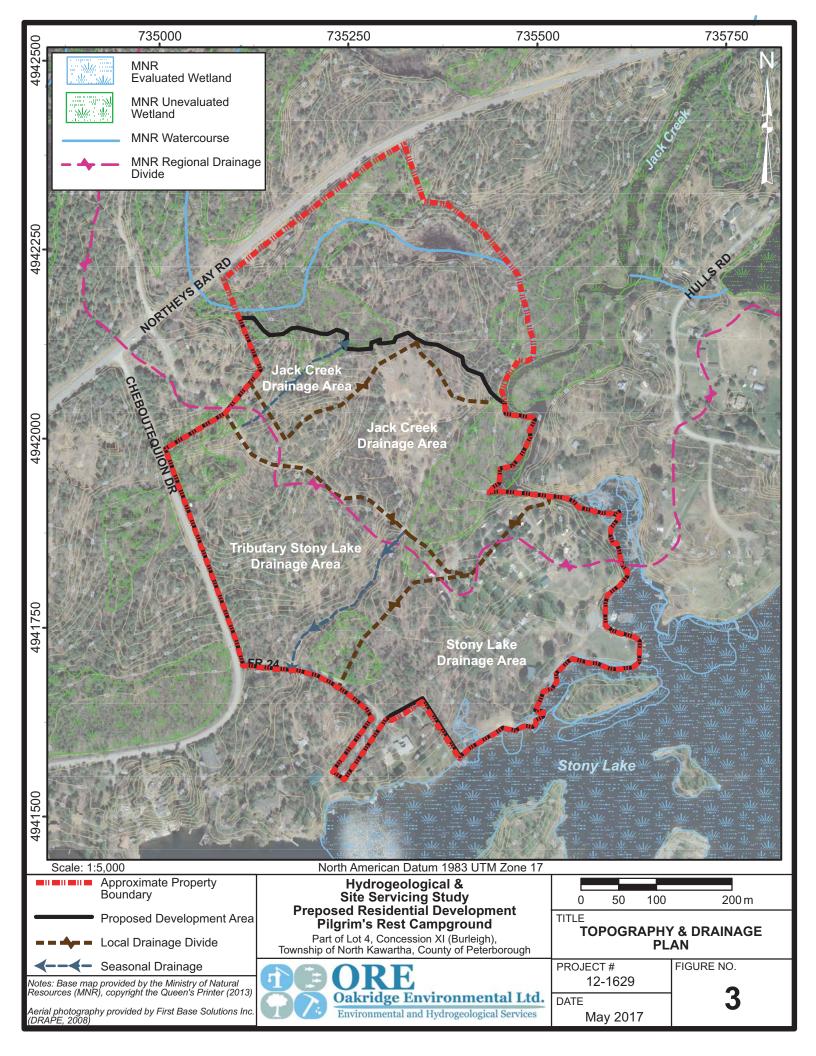
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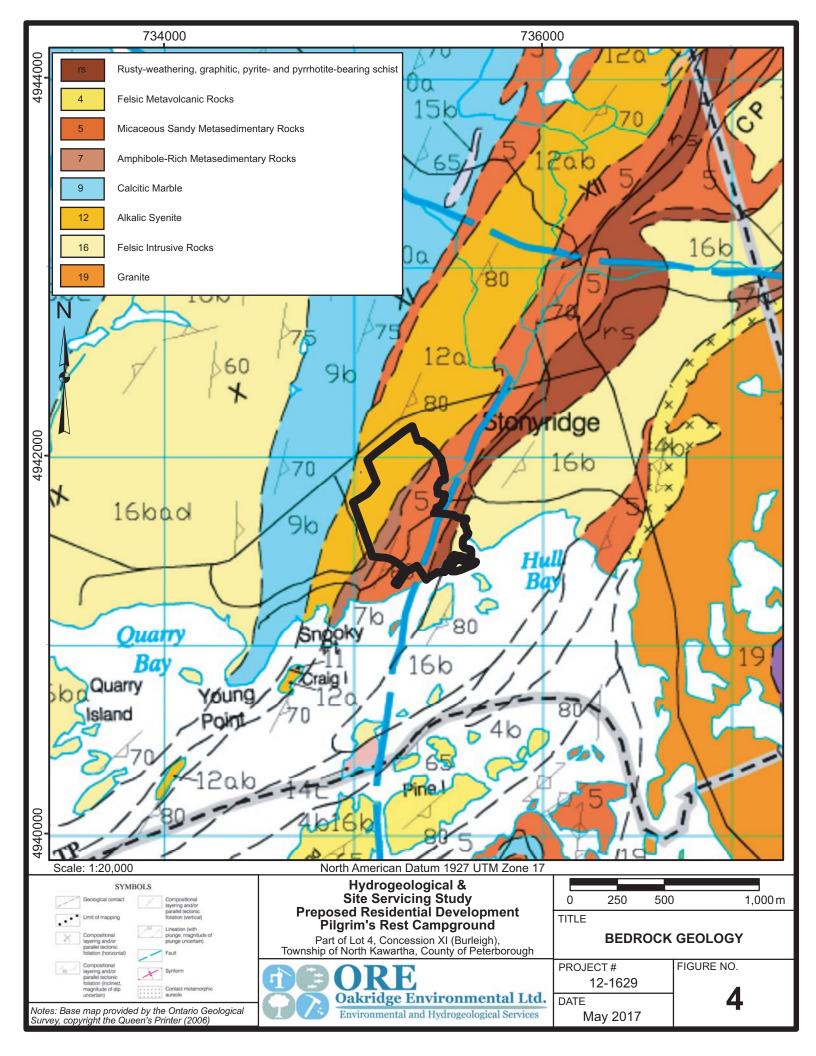
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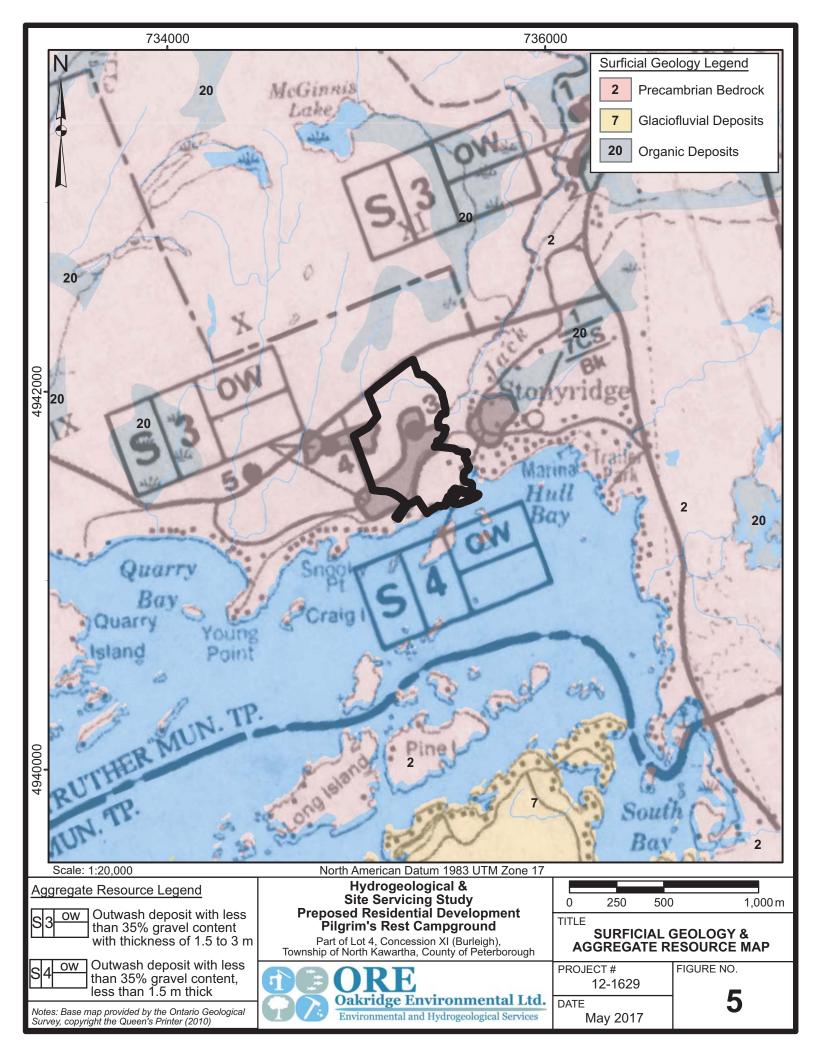


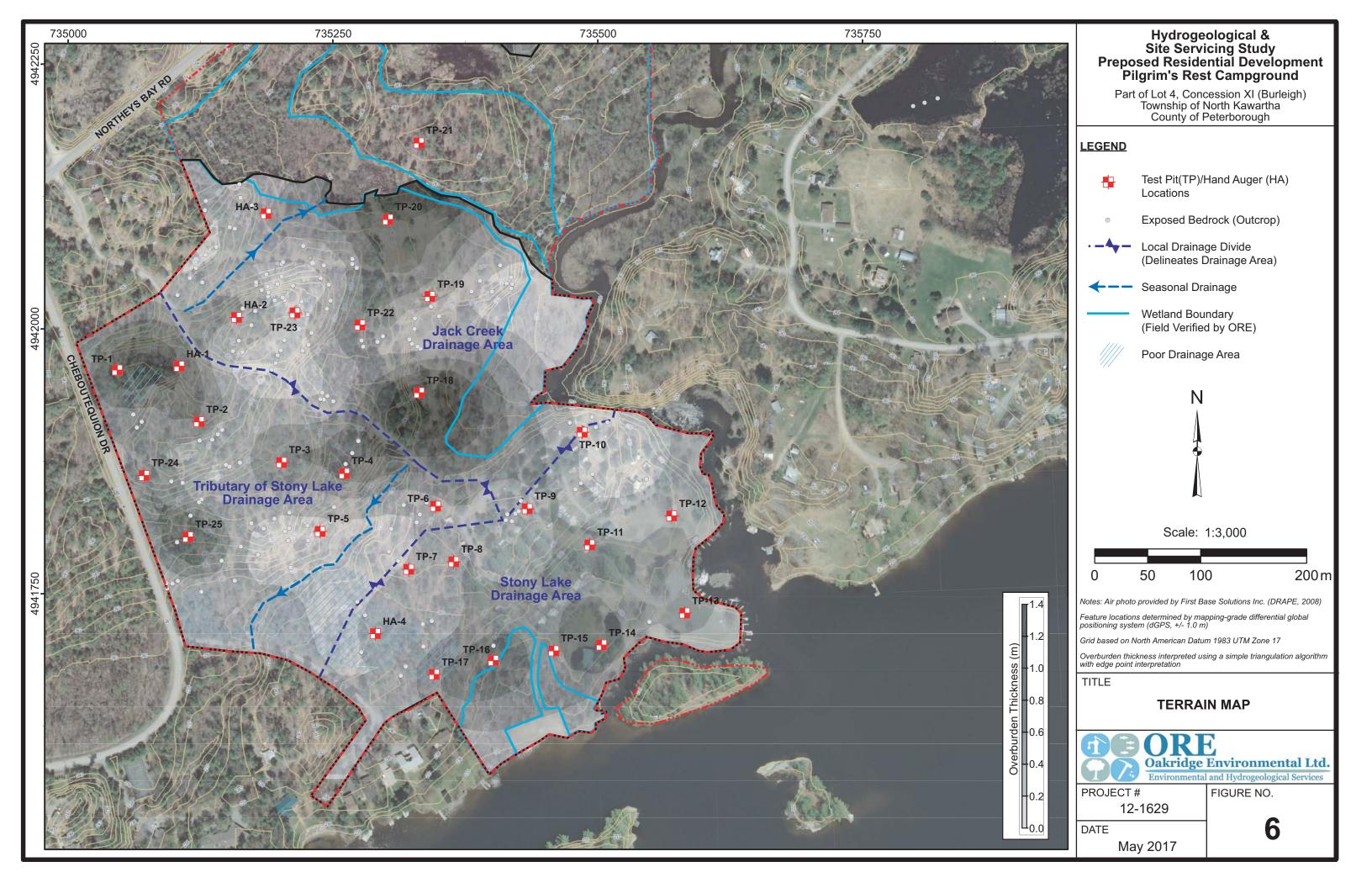


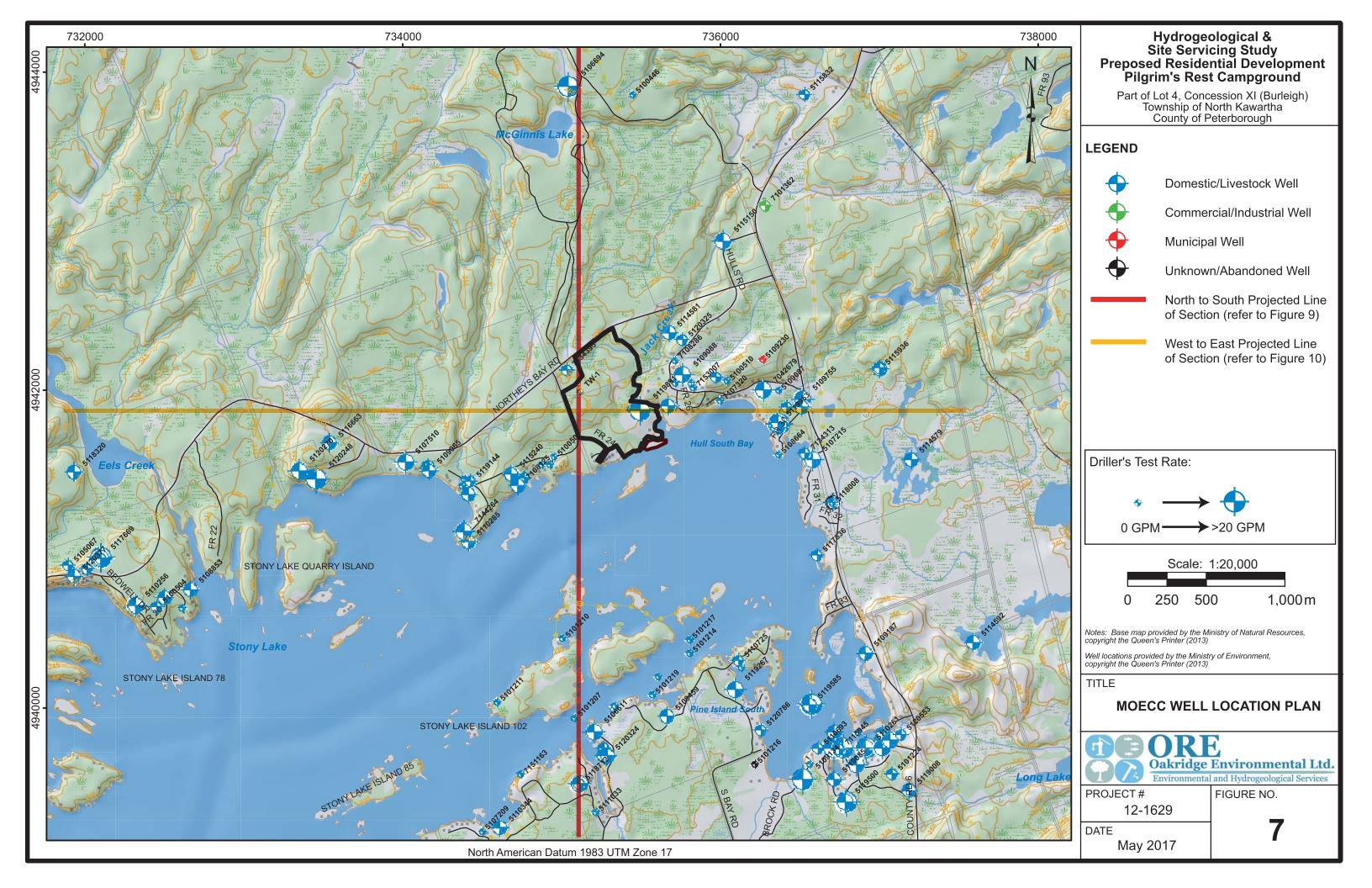


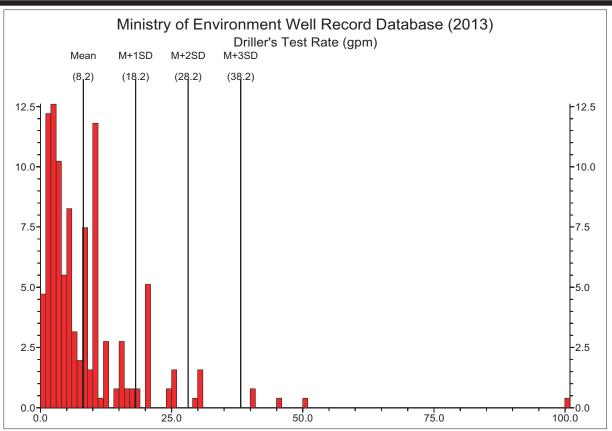


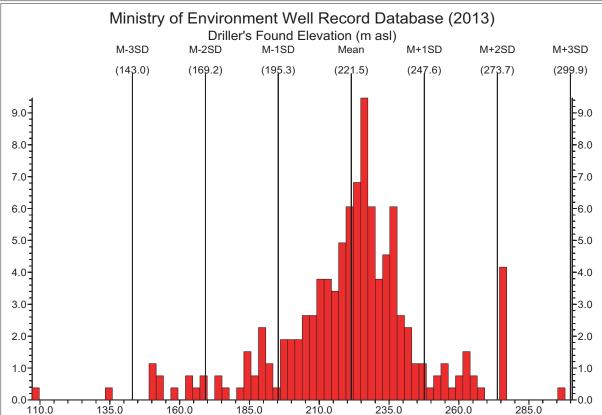












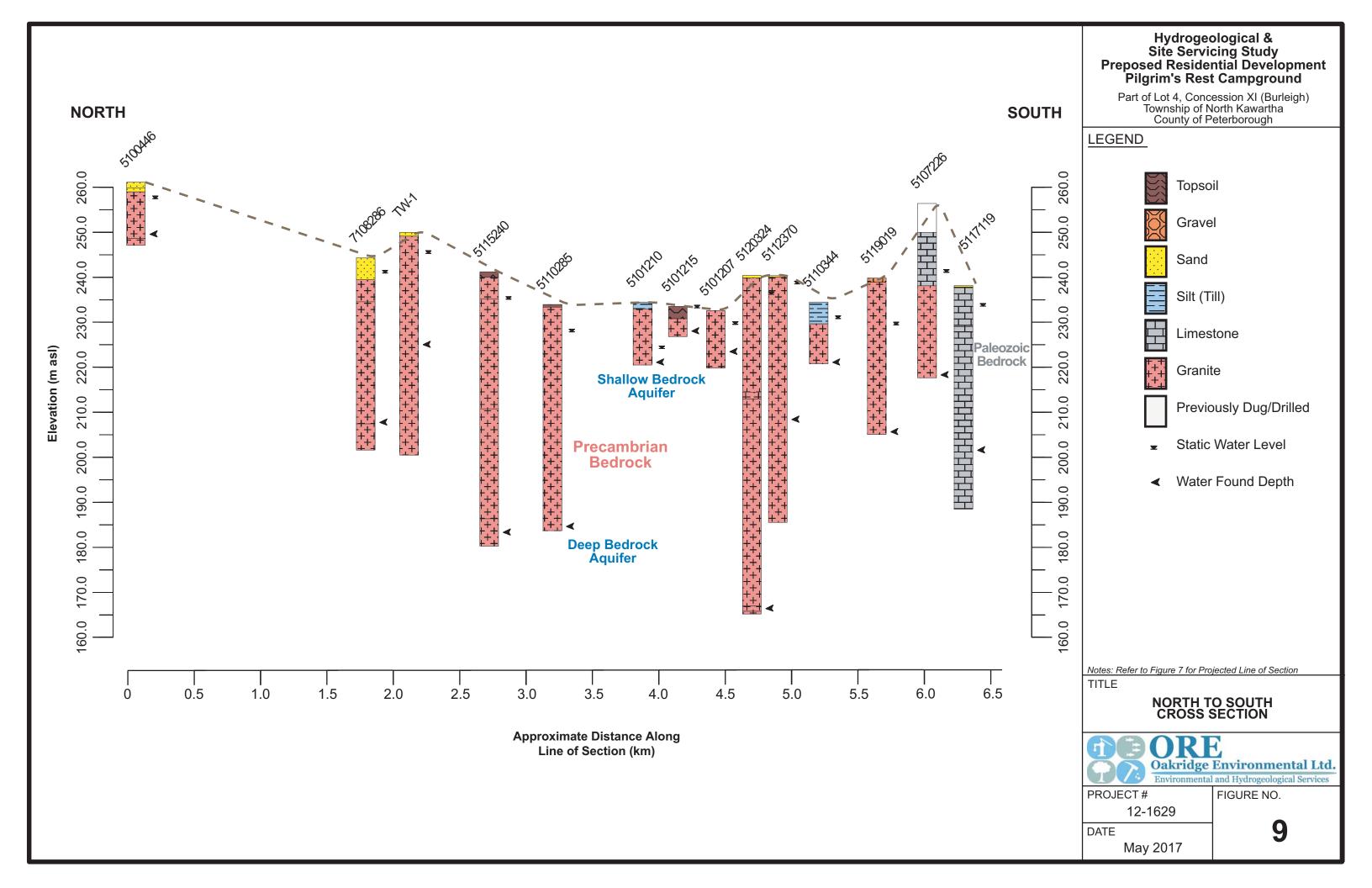


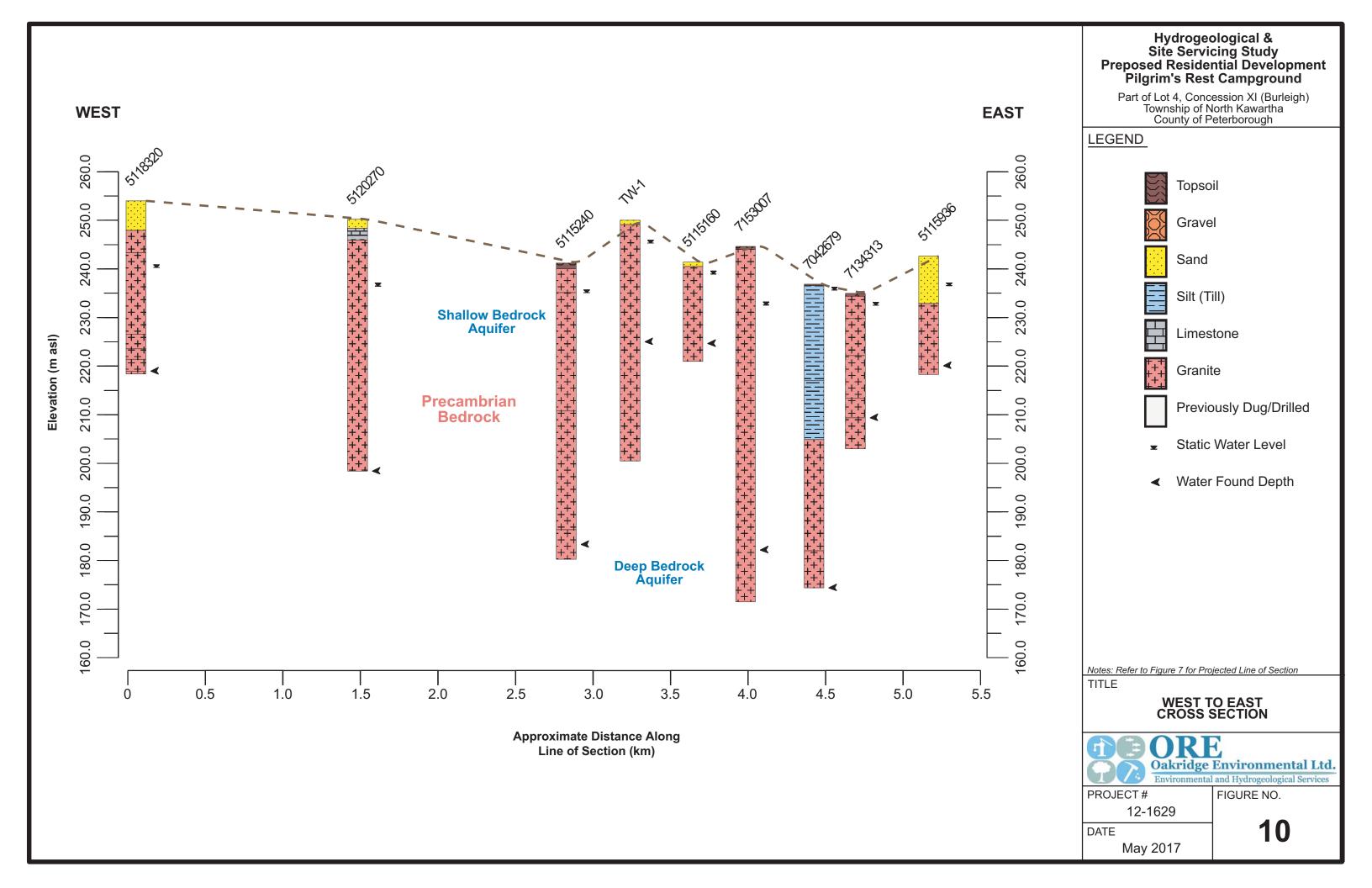
Part of Lot 4, Concession XI (Burleigh), Township of North Kawartha, County of Peterborough TITLE WELL STATISTICS

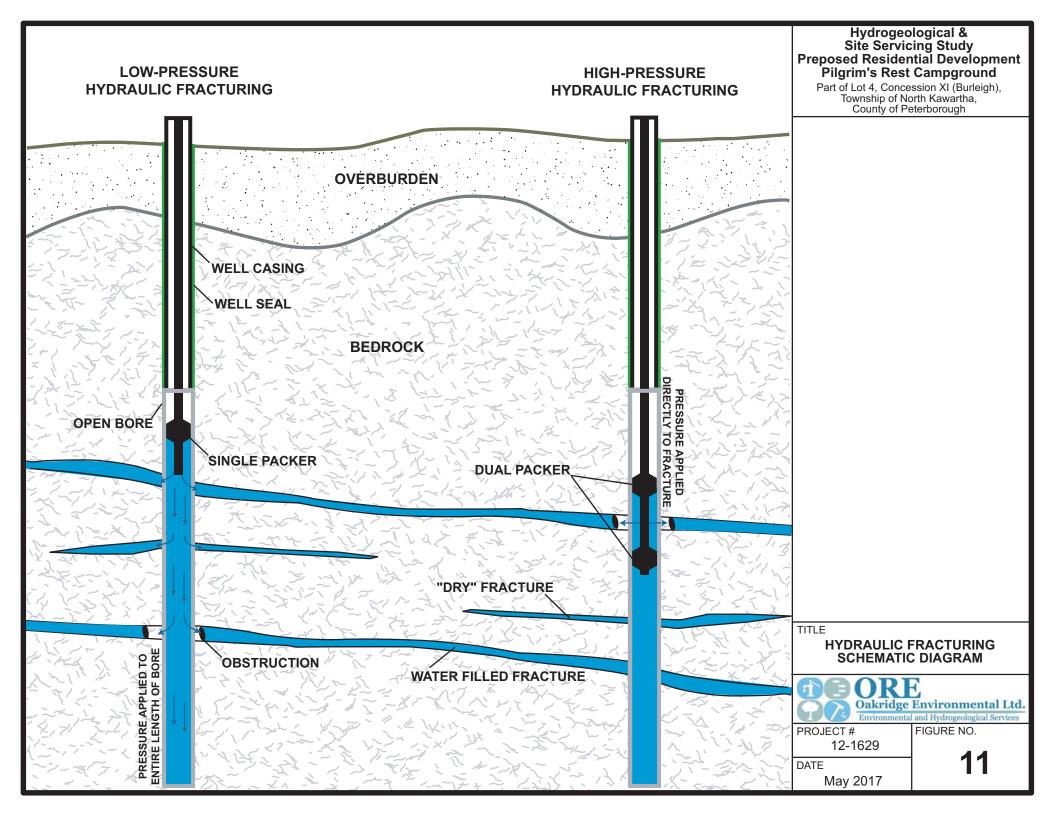


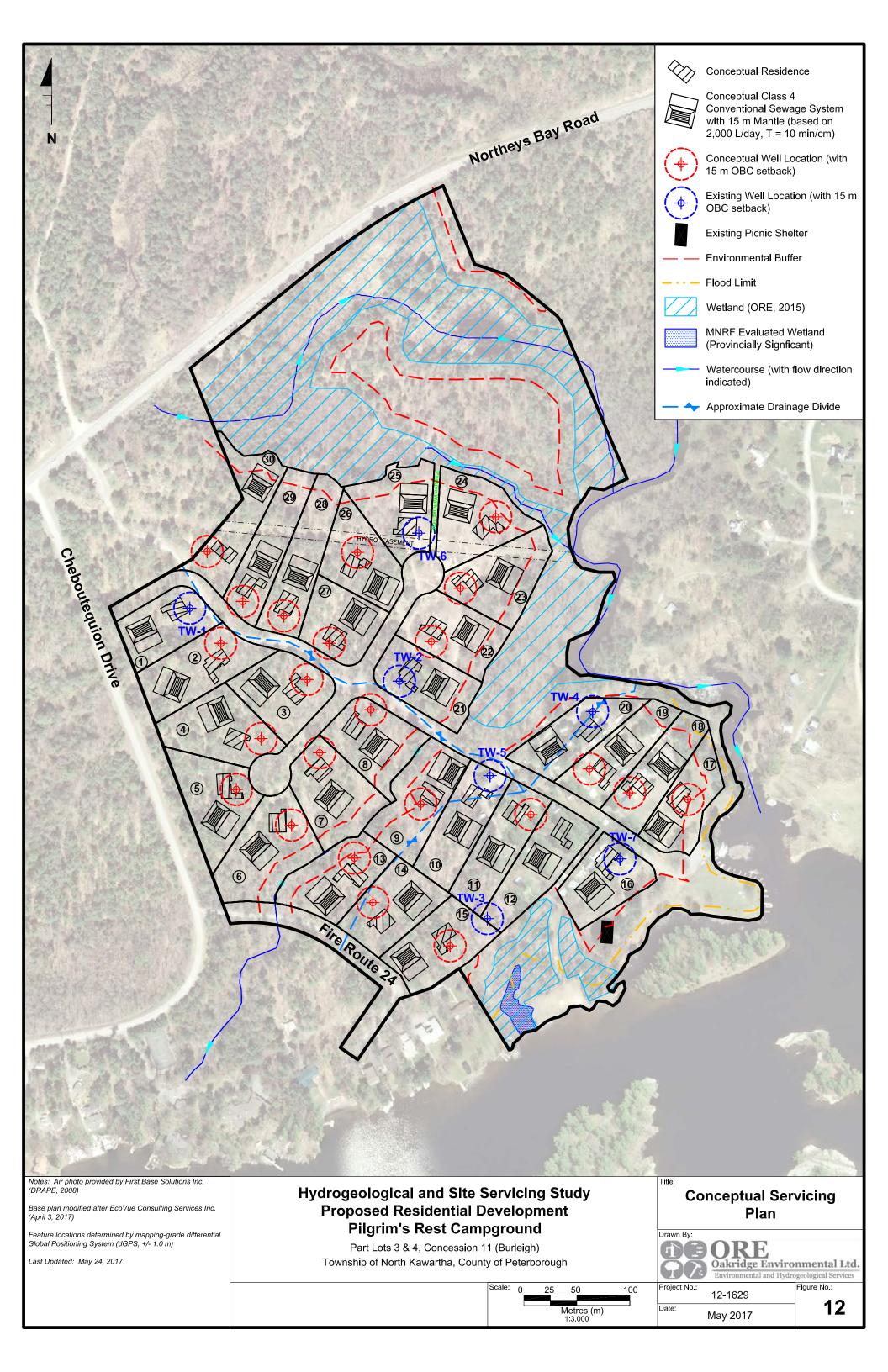
PROJECT # FIGURE NO. 12-1629

DATE May 2017 8









## APPENDIX A

Test Pit Logs and Grain Size Analysis

P.O. Box 431,

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3

**HA-1 TESTPIT NO.:** 

1.25 **TOTAL DEPTH:** 

**BACKHOE INFORMATION** 

UTM Coordinates : 735158, 4942010

Elevation (masl):

254

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: DM

DATES ASSESSED: May 6, 2013

**EXCAVATION CO.:** 

BACKHOE TYPE: Hand Auger

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Water Level

△ Moist

## FIELD TEST PIT LOG

	Water	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
Depth    0.00 =	Water	Piezometer Installation	Special Notes	Sample #		Depth  0.05 m -	Symbol	TOPSOIL: Black silty topsoil with rootlets  SM: Oxidized silty sand.  Perched water table at 0.15 m  End @ 1.25 m
1.00 -								
1.10 -								

NOTES: 20 °C, Cloudy

P.O. Box 431, Phone: 705-745-1181 Peterborough,

**TOTAL DEPTH:** UTM Coordinates : 735104, 4941964

0.94 Elevation (masl):

HA-2

248.5

PROJECT INFORMATION

Fax: 705-745-4163 ON., K9J 6Z3

PROJECT NO: 12-1629 **EXCAVATION CO.:** 

SITE LOCATION: Pilgrim's Rest Campground BACKHOE TYPE: Hand Auger

LOGGED BY: DM STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

**TESTPIT NO.:** 

DATES ASSESSED: May 6, 2013

Water Level

△ Moist

**BACKHOE INFORMATION** 

## FIELD TEST PIT LOG

	Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10 -						0.05 m —		TOPSOIL: Black silty topsoil with rootlets  SM: Dry oxidized silty sand.  End @ 0.94 m
0.20 -								
0.30 -								
0.50 —								
0.60 -								
0.70 -								
0.90 —								

NOTES: 20 °C, Cloudy

P.O. Box 431, Phone : 705-745-1181 Peterborough, Fax : 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **HA-3** 

TOTAL DEPTH: 0.74

**BACKHOE INFORMATION** 

UTM Coordinates : 735183, 4942110

Elevation (masl):

246.5

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 13, 2013

EXCAVATION CO.:

BACKHOE TYPE: Hand Auger

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

## FIELD TEST PIT LOG

	Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 -								TOPSOIL: Dark brown silty topsoil with rootlets
0.20 -						0.2 m -		SM: Light brown silty sand. No plasticity, holds form.  No seepage
0.30 -	-							Refused on presumed bedrock @ 0.74 m End @ 0.74 m
0.40 -	-							
0.50 -	-			composite sample taken				
0.60 -	-							
0.70 —	-		refused on presumed bedrock @ 0.74 m					

NOTES: 20 °C, Cloudy

P.O. Box 431, Phone: 705-745-1181

Peterborough, ON., K9J 6Z3

**HA-4 TESTPIT NO.:** 

0.75 TOTAL DEPTH:

**BACKHOE INFORMATION** 

UTM Coordinates : 735287, 4941712

Elevation (masl):

239.9

PROJECT INFORMATION

Fax: 705-745-4163

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 13, 2013

**EXCAVATION CO.:** 

BACKHOE TYPE: Hand Auger

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Water Level △ Moist

#### FIELD TEST PIT LOG

		Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10	-							TOPSOIL: Black silty topsoil with rootlets
0.20 -	_					0.25 m -		SM: Saturated light brown sitly sand.
0.30								Saturated beyond 0.40 m  Refused on presumed bedrock @ 0.75
0.40	- - - - -		saturated beyond 0.4 m					End @ 0.75 m
0.50	_			composite sample taken				
0.60 -								
0.70 -	_		refused on presumed bedrock @ 0.75 m					

NOTES: 20 °C, Cloudy

P.O. Box 431, Phone: 705-745-1181 Peterborough.

Peterborough, ON., K9J 6Z3 TESTPIT NO.: **HA-5** 

TOTAL DEPTH: 0.18

**BACKHOE INFORMATION** 

Elevation (masl):

252.8

PROJECT INFORMATION

Fax: 705-745-4163

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 13, 2013

**EXCAVATION CO.:** 

BACKHOE TYPE: Hand Auger

UTM Coordinates :

735432, 4942358

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

#### FIELD TEST PIT LOG

3.00				. 🔼	
0.10 -	rootlets end at 0.12 m	composite sample taken	0.12m —	\(\dagger\) \(\dag	ML: Dark brown clayey silt trace sand.  Refused on presumed bedrock @ 0.18 m  End @ 0.18 m

NOTES: 20 °C, Cloudy

P.O. Box 431, Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3

**TESTPIT NO.: HA-6** 

0.84 **TOTAL DEPTH:** 

UTM Coordinates : Elevation (masl): 735811, 4942380 248.8

PROJECT INFORMATION **BACKHOE INFORMATION** 

PROJECT NO: 12-1629 **EXCAVATION CO.:** 

SITE LOCATION: Pilgrim's Rest Campground BACKHOE TYPE: Hand Auger

LOGGED BY: MC STANDPIPE/PIEZOMETERS: Not installed

DATES ASSESSED: June 13, 2013 SAMPLING METHODS: composite grab

> Water Level △ Moist

## FIELD TEST PIT LOG

	Water	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 -							\( \lambda \) \(	TOPSOIL: Black silty topsoil with rootlets
0.20 -						0.22 m -		SM: Oxidize silty sand. No gravel
0.30 -								Grey beyond 0.78 m  Refused on presumed bedrock @ 0.84 m  End @ 0.84 m
0.40 -								2.10 0 3.04 111
0.50 -				composite sample taken				
0.60 -								
0.70 -			grey beyond 0.78 m					
0.80 -			refused on presumed bedrock @ 0.84 m					

NOTES: 20 °C, Cloudy

P.O. Box 431, Phone: 705-745-1181 Peterborough.

Peterborough, ON., K9J 6Z3 TESTPIT NO.: **TP-1** 

TOTAL DEPTH: 1.39 m

UTM Coordinates : Ele 735045, 4941963

 $Elevation\ (masl):$ 

248.5

PROJECT INFORMATION

Fax: 705-745-4163

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

BACKHOE INFORMATION

△ Moist

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:** Supplied by Proponent

SAMPLING METHODS: composite grab

## FIELD TEST PIT LOG

	er Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 - 0.		rootlets end at 0.56 m	composite sample taken		0.13 m -		TOPSOIL: Black sand topsoil with rootlets.  SM: Reddish brown oxidized silty sand. Dry with no gravel.  SM: Grey sandy silt with trace clay. Low plasticity and no gravel. Some weathered granite.
1.30		minor seepage at 1.18 m			1.18 m —		Minor seepage at 1.18 m  Refused on presumed bedrock @ 1.39 m
-		presumed bedrock @ 1.39 m					End @ 1.39m

NOTES: 20 °C, Sunny

P.O. Box 431, Phone: 705-745-1181 Peterborough, Fax: 705-745-4163

ON., K9J 6Z3

**TESTPIT NO.: TP-2** 

1.06 m **TOTAL DEPTH:** 

UTM Coordinates : Elevation (masl): 735125, 4941912 251.1

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

**EXCAVATION CO.:**Supplied by Propenent

**BACKHOE INFORMATION** 

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Water Level △ Moist

## FIELD TEST PIT LOG

	Water	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 — - 0.10 —	-					0.1 m -		TOPSOIL: Black sandy topsoil with rootlets.  SW: Light brown well with trace silt.
0.20 —	-							Dry with no gravel.
0.30 —	-							
0.40 —								
0.50 -			rootlets end at					
0.60 — - 0.70 —			0.58 m					
0.70 -				composite sample taken		0.8 m -		
0.90 —						U.8 m —		SM: Grey silty oxidized sand . Low plasticity and no gravel.  No seepage
- 1.00 —								End @ 1.06 m

NOTES: 20 °C, Sunny

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **TP-3** 

**BACKHOE INFORMATION** 

TOTAL DEPTH: 0.84 m

UTM Coordinates : 735200, 4941878

 $Elevation\ (masl):$ 

247.8

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:** Supplied by Propenent

SAMPLING METHODS: composite grab

Water Level

△ Moist

## FIELD TEST PIT LOG

	er Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00		rootlets end at 0.45 m	composite sample taken		0.18 m -	A       A	TOPSOIL: Black sandy topsoil with rootlets  SW: Light brown sand with trace silt. Dry and no gravel, no plasticity.  No seepage  Refused on presumed bedrock @ 0.84 m  End @ 0.84 m

NOTES: 20 °C, Sunny

P.O. Box 431, Phone: 705-745-1181 Peterborough

P.O. Box 431, Peterborough, ON., K9J 6Z3 TESTPIT NO.: **TP-4** 

**BACKHOE INFORMATION** 

TOTAL DEPTH: 1.04 m

UTM Coordinates : 735256, 491857

 $Elevation\ (masl):$ 

243.1

PROJECT INFORMATION

Fax: 705-745-4163

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

**EXCAVATION CO.:**Supplied by Propenent

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Water Level

△ Moist

## FIELD TEST PIT LOG

	Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 -	-						\( \lambda \) \(	TOPSOIL: Black sandy topsoil with rootlets.
0.20 -	-					0.17 m -		SW: Light brown sand with trace silt. Dry and no gravel, no plasticity.
0.40 -	-						2	
0.50 -	-			composite sample taken		0.46 m -		SM: Light brown slightly moist well graded silty sand. No plasticity and holds form.  Oxidized beyond 0.73 m
0.60 -	-							End @ 1.04 m
0.80 -	_		Oxidization beyond 0.73 m rootlets end at 0.75 m					
0.90 -	-							
1.00 -	-							

NOTES: 20 °C, Sunny

P.O. Box 431,

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **TP-5** 

TOTAL DEPTH: 0.54 m

UTM Coordinates : 735237, 4941809

 $Elevation\ (masl):$ 

240.1

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

BACKHOE INFORMATION

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:** Supplied by Propenent

SAMPLING METHODS: composite grab

## FIELD TEST PIT LOG

	Water	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10 -						0.16 m —	\( \delta \) \( \d	TOPSOIL: Black sandy topsoil with rootlets  SW: Oxidized slightly moist silty sand trace sub-rounded gravel. No plasticity and holds form.  Refused on presumed bedrock @
0.30 -				Composite				0.54 m End @ 0.54 m
0.40 -			rootlets end at 0.36 m	sample taken				
0.50 -			refused on presumed bedrock @ 0.54 m					

NOTES: 20 °C, Sunny

P.O. Box 431, Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3

**TESTPIT NO.: TP-6** 

 $0.54 \mathrm{m}$ TOTAL DEPTH:

UTM Coordinates : 735347, 4941832

Elevation (masl):

241.9

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

**BACKHOE INFORMATION** 

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:**Supplied by Propenent

SAMPLING METHODS: composite grab

Water Level

△ Moist

## FIELD TEST PIT LOG

TOPSOIL: Black sandy topsoil with rootlets  o.10 —  rootlets end at 0.12 m  SM: Oxidized slightly moist silty sand trace sub-rounded gravel. No plasticity and holds form. No seepage.		Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
D.20 — patio stone at 0.23 m composite sample taken  composite sample taken  refused on presumed bedrock © 0.54 m  refused on presumed bedrock © 0.54 m	0.00 -			rootlets end at 0.12 m  patio stone at 0.23 m  refused on presumed bedrock @ 0.54	composite sample taken		0.12 m -	\( \lambda \) \(	SM: Oxidized slightly moist silty sand trace sub-rounded gravel. No plasticity and holds form.  No seepage.  Refused on presumed bedrock @ 0.54 m

NOTES: 20 °C, Sunny

P.O. Box 431,

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3

**TESTPIT NO.: TP-7** 

0.32 TOTAL DEPTH:

**BACKHOE INFORMATION** 

UTM Coordinates : 735320, 491776

Elevation (masl):

241.3

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

**EXCAVATION CO.:** Supplied by Propenent

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Water Level △ Moist

## FIELD TEST PIT LOG

Depth Water	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10 -		rootlets end at 0.32 m  refused on presumed bedrock @ 0.32 m	composite sample taken		0.13 m -		SM: Oxidized slightly moist silty sand trace sub-rounded gravel. No plasticity and holds form.  No seepage.  Refused on presumed bedrock @ 0.32 m  End @ 0.32 m

NOTES: 20 °C, Sunny

P.O. Box 431,

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3

**TESTPIT NO.: TP-8** 

0.53 **TOTAL DEPTH:** 

UTM Coordinates : 735362, 4941780

Elevation (masl):

241.5

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

**BACKHOE INFORMATION** 

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:**Supplied by Propenent

SAMPLING METHODS: composite grab

Water Level △ Moist

## FIELD TEST PIT LOG

		Vater	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10	-	Δ		moist beyond 0.16 m	composite sample taken		0.16 m -		SM: Oxidized moist silty sand trace sub-rounded gravel and clay. No plasticity and holds form.  No seepage.  Refused on presumed bedrock @ 0.53 m  End @ 0.53 m
0.50				0.53 m  refused on presumed bedrock @ 0.53 m					

NOTES: 20 °C, Sunny

P.O. Box 431,

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **TP-9** 

TOTAL DEPTH: 0.77

UTM Coordinates : 735433, 4941831

 $Elevation\ (masl):$ 

244.0

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

BACKHOE INFORMATION

**EXCAVATION CO.:**Supplied by Propenent

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Water Level

△ Moist

## FIELD TEST PIT LOG

	Water	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10 -  0.20 -  0.30 -  0.40 -  0.50 -  0.70 -	. \(\sum_{\text{\subset}}\)		rootlets end at 0.32 m bedrock ledge at 0.33 m seepage at 0.5 m	composite sample taken		0.35 m -		TOPSOIL: Black sandy topsoil with rootlets.  Bedrock ledge at 0.33 m.  SM: Light brown moist silty sand with some clay and sub-rounded gravel. Gravel 5 cm nominal size.  Seepage at 0.5 m  Oxidized at 0.77 m  Refused on presumed bedrock @ 0.77 m  End @ 0.77 m

NOTES: 20 °C, Sunny

P.O. Box 431,

Phone : 705-745-1181 Peterborough, Fax : 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **TP-10** 

TOTAL DEPTH: 0.28

UTM Coordinates : 735485, 4941901

 $Elevation\ (masl):$ 

243.1

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

BACKHOE INFORMATION

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:**Supplied by Propenent

SAMPLING METHODS: composite grab

## FIELD TEST PIT LOG

Depth Water Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
roi 0 rei pri	pist beyond 23 m potlets end at 0.28 m efused on resumed ddrock @ 0.28	Composite sample taken		0.23 m —	-         -	TOPSOIL: Black sandy topsoil with rootlets.  SM: Oxidized light brown moist silty sand with some clay and subrounded gravel.  No Seepage.  Refused on presumed bedrock @ 0.28 m  End @ 0.28 m

NOTES: 20 °C, Sunny

P.O. Box 431, Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3

**TESTPIT NO.: TP-11** 

0.52 **TOTAL DEPTH:** 

Elevation (masl):

239.1

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

**EXCAVATION CO.:**Supplied by Propenent

**BACKHOE INFORMATION** 

BACKHOE TYPE: Hitachi

UTM Coordinates :

735493, 4941796

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Water Level

△ Moist

#### FIELD TEST PIT LOG

	/ater	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10 -			rootlets end at 0.13 m			0.13 m -		TOPSOIL: Black sandy topsoil with rootlets.  SM (FILL): Light brown dry sand with sub-round gravel and trace silt.
0.30 -			refused on presumed bedrock @ 0.52	composite sample taken		0.32 m -		PEAT: Black silty organic soil with twigs and bark.  No seepage.  Refused on presumed bedrock @ 0.52 m  END @ 0.52 m

NOTES: 20 °C, Sunny

P.O. Box 431,

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3

**TESTPIT NO.: TP-12** 

0.49 TOTAL DEPTH:

UTM Coordinates : 735569, 4941822

Elevation (masl):

239.1

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

**BACKHOE INFORMATION** 

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:** Supplied by Propenent

SAMPLING METHODS: composite grab

Water Level

△ Moist

#### FIELD TEST PIT LOG

	Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10 - 0.20 - 0.40 -	Wate	Installation	rootlets end at 0.36 m	composite sample taken		0.18 m -	Symbol	Soil Description  TOPSOIL: Black sandy topsoil with rootlets.  SW: Oxidized dry sand with trace silt. Not plastic, holds form.  Refused on presumed bedrock @ 0.49 m  End @ 0.49 m
-	-		refused on presumed bedrock @ 0.49					

NOTES: 20 °C, Sunny

P.O. Box 431,

Phone : 705-745-1181 Peterborough, Fax : 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **TP-13** 

TOTAL DEPTH: 0.99

UTM Coordinates : 735579, 4941731

 $Elevation\ (masl):$ 

236.5

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

BACKHOE INFORMATION

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:** Supplied by Propenent

SAMPLING METHODS: composite grab

#### FIELD TEST PIT LOG

	Water	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 -						0.13 m -	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	TOPSOIL: Black sandy topsoil with rootlets.  SW (FILL): Oxidized dry sand with
0.20 -								trace silt. Not plastic, holds form.
0.30 -						0.42 m -		PEAT: Black spongy fibrous
0.50 -				composite				organics (i.e. sticks, blocks of wood, roots) with some sub-rounded cobbles . Decompositing odour.  Moist at 0.77 m
0.60 -				sample taken after 0.5 m				Seepage at 0.95 m End @ 0.99 m
0.70 —								
0.80 -	Δ		moist beyond 0.77 m					
0.90 -			0.05					
			seepage at 0.95					

NOTES: 20 °C, Sunny

P.O. Box 431, Phone: 705-745-1181 Peterborough

Peterborough, ON., K9J 6Z3 TESTPIT NO.: **TP-14** 

TOTAL DEPTH: 1.22

UTM Coordinates : 735502,4941701

 $Elevation\ (masl):$ 

237.1

PROJECT INFORMATION

Fax: 705-745-4163

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

BACKHOE INFORMATION

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:**Supplied by Propenent

SAMPLING METHODS: composite grab

#### FIELD TEST PIT LOG

	Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 -	Δ		moist beyond			0.22 m -	\(\frac{\lambda}{\lambda}\)\(\frac{\lambda}{\lam	TOPSOIL: Black sandy topsoil with rootlets.  SW: Light brown moist sand.
0.30 -			0.22 m					Moist at 0.22 m
- 0.50 —			rootlets end at 0.43 m					
0.60 — - 0.70 —	Σ			composite		0.62 m -		SW: Grey sand. No fines, no gravel. Seepage @ 0.76 End @ 1.22 m
0.80 — - 0.90 —			seepage @ 0.76 m					
1.00 — - 1.10 —								
1.20 —								

NOTES: 20 °C, Sunny

P.O. Box 431, Phone: 705-745-1181 Peterborough.

Peterborough, ON., K9J 6Z3 TESTPIT NO.: **TP-15** 

TOTAL DEPTH: 0.84

UTM Coordinates : 735459, 4941694

 $Elevation\ (masl):$ 

237.0

PROJECT INFORMATION

Fax: 705-745-4163

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

BACKHOE INFORMATION

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:**Supplied by Propenent

SAMPLING METHODS: composite grab

Water Level

△ Moist

#### FIELD TEST PIT LOG

	Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10 -						0.16 m -		TOPSOIL: Black sandy topsoil with rootlets.  SM: Light brown moist silty sand.  Moist beyond 0.43 m
0.30 -								
0.40 -	Δ		moist beyond 0.43 m rootlets end at 0.49 m	composite		0.43 m -	,-D	SW: Grey sand with gravel and rounded cobbles.  Seepage @ 0.75
0.60 -								Refused on presumed boulder @ 0.84 m End @ 0.84 m
0.70 -	. <u>\</u>							

NOTES: 20 °C, Sunny

P.O. Box 431, Phone : 705-745-1181 Peterborough, Fax : 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **TP-16** 

TOTAL DEPTH: 0.88

UTM Coordinates : 735400, 4941685

 $Elevation\ (masl):$ 

237.1

PROJECT INFORMATION

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

PROJECT NO: 12-1629

DATES ASSESSED: June 12, 2013

BACKHOE INFORMATION

△ Moist

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

**EXCAVATION CO.:**Supplied by Propenent

SAMPLING METHODS: composite grab

#### FIELD TEST PIT LOG

	Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 -							\( \lambda \) \(	TOPSOIL: Black sandy topsoil with rootlets.
0.20 —							\$ \$ \$	
0.30 -						0.25 m -		SM: Light brown moist silty sand.  Bedrock ledge at 0.43 m  Seepage @ 0.88 m
0.40 -			bedrock ledge at 0.43 m					Refused on presumed bedrock @ 0.88 m End @ 0.88 m
0.50 —			rootlets end at 0.47 m					
0.60 —				composite sample taken				
0.70 —	-							
0.80 -	Ā		seepage at 0.88 m refused on presumed bedrock @ 0.88					

NOTES: 20 °C, Sunny

P.O. Box 431,

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3

**TESTPIT NO.: TP-17** 

1.07 TOTAL DEPTH:

Elevation (masl):

239.1

PROJECT INFORMATION

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

PROJECT NO: 12-1629

DATES ASSESSED: June 12, 2013

**BACKHOE INFORMATION** 

**EXCAVATION CO.:** Supplied by Propenent

BACKHOE TYPE: Hitachi

UTM Coordinates :

735345, 4941674

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Water Level △ Moist

#### FIELD TEST PIT LOG

	Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 -						0.13 m —	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	TOPSOIL: Black sandy topsoil with rootlets.
0.20 -						0.13 m —		SM: Light brown moist silty sand.  Oxidized from 0.13 m - 0.77 m  Seepage @ 0.77 m
0.30 -								End @ 0.77 m
0.40 -			rootlets end at 0.47 m					
0.60 —			U.47 III	composite				
0.70 —	¥							
0.80 -	<del>-</del> <u>-</u>		seepage @ 0.77 m					
0.90 -								
-								

NOTES: 20 °C, Sunny

P.O. Box 431, Phone : 705-745-1181 Peterborough, Fax : 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **TP-18** 

TOTAL DEPTH: **1.70** 

UTM Coordinates : 735329, 4941941

 $Elevation\ (masl):$ 

240.8

PROJECT INFORMATION

TROOLOT IN ORIVINATION

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

PROJECT NO: 12-1629

DATES ASSESSED: June 12, 2013

BACKHOE INFORMATION

**EXCAVATION CO.:**Supplied by Propenent

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

#### FIELD TEST PIT LOG

	Wateı	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 - 0.10 - 0.20 - 0.30 - 0.40 - 0.50 - 0.60 - 0.70 - 1.00 - 1.10 - 1.20 - 1.30 - 1.40 - 1.50 - 1.60 -	Δ		moist beyond 0.10 m	Composite sample taken		0.89 m -		SM (FILL): Brown silty sand with some gravel.  Moist at 0.10 m  SM: Grey clayey silty sand. Not plastic holds form. No gravel.  Seepage at 0.89 m  Refused on presumed bedrock @ 1.70 m  End @ 1.70 m
]			bedrock @ 1.70					

NOTES: 20 °C, Sunny

P.O. Box 431,

Phone : 705-745-1181 Peterborough, Fax : 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **TP-19** 

TOTAL DEPTH: 0.98

**BACKHOE INFORMATION** 

UTM Coordinates : 735340, 4942033

 $Elevation\ (masl):$ 

243.0

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 12, 2013

EXCAVATION CO.: Supplied by Propenent

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

#### FIELD TEST PIT LOG

1	Vate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
Depth V	Water	Piezometer Installation	Special Notes	Sample #		Depth		SM: Grey clayey silty sand. Not plastic, holds form. No gravel.  Seepage at 0.98 m  Refused on presumed bedrock @ 0.98 m  End @ 0.98 m
0.70 -								
0.80 -			seepage at 0.98 m refused on presumed bedrock @ 0.98					
	Ž		m					

NOTES: 20 °C, Sunny

P.O. Box 431,

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3

**TESTPIT NO.: TP-20** 

1.10 TOTAL DEPTH:

UTM Coordinates : 735297, 4942108

Elevation (masl): 245.0

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 13, 2013

**BACKHOE INFORMATION** 

**EXCAVATION CO.:** Supplied by Propenent

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Water Level

△ Moist

#### FIELD TEST PIT LOG

	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 - 0.10 - 0.20 - 0.30 - 0.40 - 0.50 - 0.60 - 0.70 - 0.80 - 0.90 - 0.		rootlets end @ 0.33 m	composite sample taken		0.33 m —	◇ ◇ ◇ ◇ ◇ ◇        ◇	TOPSOIL: Dark brown silty topsoil with rootlets  SW: Light brown sand trace silt. No gravel, no plasticity, holds form.  No seepage End @ 1.10 m

NOTES: 20 °C, Cloudy

P.O. Box 431,

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **TP-21** 

TOTAL DEPTH: 1.82

UTM Coordinates : 735329,4942175

 $Elevation\ (masl):$ 

245.3

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

OGGED DT. MC

DATES ASSESSED: June 13, 2013

BACKHOE INFORMATION

**EXCAVATION CO.:**Supplied by Propenent

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

#### FIELD TEST PIT LOG

	Water	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
Depth  0.00 -  0.10 -  0.20 -  0.30 -  0.40 -  0.50 -  0.60 -  0.70 -  0.80 -	Water	Piezometer Installation	rootlets end @ 0.49 m oxidized beyond 0.58 m	Sample #		Depth 0.21 m -	Symbol	Soil Description  TOPSOIL: Dark brown silty topsoil with rootlets  SM: Light brown silty sand. No plasticity, holds form.  Beyond 0.58 m oxidized with greater amounts of sub-rounded gravel and silt. nominal size 20 mm.  No seepage End @ 1.82 m
0.90 - 1.00 - 1.10 - 1.20 - 1.30 - 1.50 - 1.70 - 1.80 - 1.				composite sample taken				

NOTES: 20 °C, Cloudy

P.O. Box 431, Phone: 705-745-1181 Fax: 705-745-4163

Peterborough, ON., K9J 6Z3

**TESTPIT NO.: TP-22** 

1.2 **TOTAL DEPTH:** 

**BACKHOE INFORMATION** 

UTM Coordinates : 735274, 4942000

Elevation (masl):

246.2

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 13, 2013

**EXCAVATION CO.:**Supplied by Propenent

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Water Level △ Moist

#### FIELD TEST PIT LOG

Depth Water	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00		rootlets end @ 0.61 m	composite sample taken		0.32 m -		TOPSOIL: Dark brown silty topsoil with rootlets  SM: Light brown silty sand with trace angular gravel. No plasticity, holds form.  No seepage End @ 1.2 m

NOTES: 20 °C, Cloudy

P.O. Box 431, Phone: 705-745-1181 Peterborough

Peterborough, ON., K9J 6Z3 TESTPIT NO.: **TP-23** 

TOTAL DEPTH: 0.23

**BACKHOE INFORMATION** 

UTM Coordinates : 735211, 4942017

 $Elevation\ (masl):$ 

254.2

PROJECT INFORMATION

Fax: 705-745-4163

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 13, 2013

EXCAVATION CO.:Supplied by Propenent

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

#### FIELD TEST PIT LOG

	Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10 -				composite sample taken		0.08 m -	A     A <td>SW: Dark brown sand with silt trace clay and subrounded gravel.  No seepage  Refused on presumed bedrock @ 0.23 m  End @ 0.23 m</td>	SW: Dark brown sand with silt trace clay and subrounded gravel.  No seepage  Refused on presumed bedrock @ 0.23 m  End @ 0.23 m
0.20 -			rootlets end @ 0.23 m  refused on presumed bedrock @ 0.23 m					

NOTES: 20 °C, Cloudy

P.O. Box 431, peterborough,

Phone: 705-745-1181 Fax: 705-745-4163 TESTPIT NO.: **TP-24** 

**BACKHOE INFORMATION** 

TOTAL DEPTH: 1.03

UTM Coordinates : 735069, 4941861

 $Elevation\ (masl):$ 

251.2

PROJECT INFORMATION

NEORMATION

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

PROJECT NO: 12-1629

DATES ASSESSED: June 13, 2013

**EXCAVATION CO.:**Supplied by Propenent

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

Seepage

Water Level

ON., K9J 6Z3

△ Moist

#### FIELD TEST PIT LOG

	Wateı	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.10 -  0.30 -  0.40 -	water	Installation	oxidized from 0.24 m to 0.73 m	Sample #		0.24 m —	Symbol A A A A A A A A A A A A A A A A A A A	Soil Description  TOPSOIL: Dark brown silty topsoil with rootlets  SM: Light brown silty sand. No gravel Oxidized from 0.24 m to 0.73 m No seepage End @ 1.03 m
0.50 - 0.60 - 0.70 - 0.80 -			rootlets end @ 0.53 m	composite sample taken				

NOTES: 20 °C, Cloudy

P.O. Box 431,

Phone: 705-745-1181 Peterborough, Fax: 705-745-4163 ON., K9J 6Z3 TESTPIT NO.: **TP-25** 

TOTAL DEPTH: 1.63

UTM Coordinates : 735112, 4941804

 $Elevation\ (masl):$ 

245.5

PROJECT INFORMATION

PROJECT NO: 12-1629

SITE LOCATION: Pilgrim's Rest Campground

LOGGED BY: MC

DATES ASSESSED: June 13, 2013

EXCAVATION CO.: Supplied by Propenent

**BACKHOE INFORMATION** 

△ Moist

BACKHOE TYPE: Hitachi

STANDPIPE/PIEZOMETERS: Not installed

SAMPLING METHODS: composite grab

#### FIELD TEST PIT LOG

	Wate	Piezometer Installation	Special Notes	Sample #	Vapour ppm	Depth	Soil Symbol	Soil Description
0.00 - 0.10 -						0.12 m -		TOPSOIL: Black silty topsoil with rootlets
0.20 -						0.12 m		SM: Oxidized red silty sand with trace poorly sorted subrounded gravel and cobles. No plasticity, holds form.
0.30 -								Seepage @ 1.63 m
0.40 -	-							Refused on presumed bedrock @ 1.63 m
0.50 -								End @ 1.63 m
0.60 -			rootlets end at 0.58 m					
0.70 -	1 1							
0.80 -	-							
0.90 -				composite sample taken				
1.00 -	-			bampie careii				
1.10 -	1 1							
1.20 -	-							
1.30 -	1 1							
1.40 -			seepage at 1.62					
1.50 -			m refused on					
1.60 -	호		presumed bedrock @ 1.63					

NOTES: 20 °C, Cloudy

### APPENDIX B

MOECC Well Record Database

#### **Ministry of Environment Well Records**

<b>Well I.D.</b> 5100446	Easting N 735449.1	lorthing El 4943840			i <b>nd (m)</b> 1		Status Water Supply	<b>Use</b> Domestic	Water Kind FRESH	Lith. Depth Lith. Cold	ur Lith. Materials	
							,			1.22	MEDIUM SAND	
										1.22	MEDIUM SAND	
										2.13	MEDIUM SAND	STONES
										2.13	MEDIUM SAND	STONES
										12.5 BLACK	GRANITE	
										12.5 BLACK	GRANITE	
										14.0 RED	GRANITE	
										14.0 RED	GRANITE	
5100499	730566.1	4939370	241.2	22 ft	12.80	4 GPM	Water Supply	Domestic	FRESH			
										6.1	CLAY	HARDPAN
										6.1	CLAY	HARDPAN
										7.01	GRAVEL	
										7.01	GRAVEL	
										17.1 BROWN	GRANITE	
										17.1 BROWN	GRANITE	
5100501	730498.1	4939896	243.2	5 ft	29.26	1 GPM	Water Supply	Domestic	FRESH			
										1.22 RED	GRANITE	
										1.22 RED	GRANITE	
										8.23 GREY	GRANITE	
										8.23 GREY	GRANITE	
										10.7 BLACK	GRANITE	
										10.7 BLACK	GRANITE	
										27.4 GREY	GRANITE	
										27.4 GREY	GRANITE	
										32.0 WHITE	GRANITE	
										32.0 WHITE	GRANITE	
										44.2 GREY	GRANITE	
										44.2 GREY	GRANITE	
5100502	730746.1	4940488	243.1	10 ft	6.401	5 GPM	Water Supply	Domestic	FRESH			
										6.40	MEDIUM SAND	GRAVEL
5100503	732500.1	4940675	244.3	10 ft	10.36	7 GPM	Water Supply	Domestic	FRESH			
										0.30	TOPSOIL	
										0.30	TOPSOIL	
										1.52 BROWN	MEDIUM SAND	
										1.52 BROWN	MEDIUM SAND	
										11.6 RED	GRANITE	
										11.6 RED	GRANITE	
5100504	732452.1	4940613	245.4	20 ft	32.00	3 GPM	Water Supply	Domestic	FRESH			

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0.30   TOPSOIL   1.83 BROWN   MEDIUM SAND   STONES   ST	Well I.D.	Easting I	Northing	Elev.	WL Four	nd (m) 1	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Colo	ur Lith. Materials	
											0.30	TOPSOIL	
STONES											0.30	TOPSOIL	
Second   S											1.83 BROWN	MEDIUM SAND	STONES
1											1.83 BROWN	MEDIUM SAND	STONES
											3.96 GREY	LIMESTONE	
6.1 GREY   GRANTE   7.92 RED   7.											3.96 GREY	LIMESTONE	
											6.1 GREY	GRANITE	
											6.1 GREY	GRANITE	
11.6 WHITE   GRANITE   GRANITE   11.6 WHITE   THE WITTER											7.92 RED	GRANITE	
11   12   13   14   14   14   14   14   14   14											7.92 RED	GRANITE	
14.9 BLACK GRANITE   13.7 RED GRANITE   13.9 BLACK GRANITE   13.7 BLACK GRANITE   14.3 RED GR											11.6 WHITE	GRANITE	
14.9 BLACK   GRANITE   G											11.6 WHITE	GRANITE	
Recommendation											14.9 BLACK	GRANITE	
Recommendation											14.9 BLACK	GRANITE	
1.7   1.8											28.3 WHITE	GRANITE	
Second   S											28.3 WHITE	GRANITE	
Second   S											31.7 RED	GRANITE	
Sample   S											31.7 RED	GRANITE	
Table   Tabl											32.9 WHITE	GRANITE	
5.18 RED   GRANITE   5.18 RED   GRANITE   5.18 RED   GRANITE   5.18 RED   GRANITE   6.18 RE											32.9 WHITE	GRANITE	
Second   S	5100505	734878.1	494151	1 237.1	I 3ft	13.72	2 GPM	Water Supply	Domestic	FRESH			
13.7 BLACK   GRANITE   13.7 BLACK   GRANITE   13.7 BLACK   GRANITE   13.7 BLACK   GRANITE   14.3 RED   GRANITE   14.3 RED   GRANITE   14.3 RED   GRANITE   14.3 RED   GRANITE   16.8 BLACK   16.8 BLACK   GRANITE   16.8 BLACK   16.8 BLACK													
13.7 BLACK   GRANITE   14.3 RED   GRANITE   16.8 BLACK   16.8 BLACK   GRANITE   16.8 BLACK   16.8 BLACK   GRANITE   16.8 BLACK   16.8 BLACK													
14.3 RED   GRANITE   16.8 BLACK   16.8 BLAC													
14.3 RED   GRANITE   16.8 BLACK   16.8 BLACK   GRANITE   16.8 BLACK   GRANITE   16.8 BLACK													
16.8 BLACK GRANITE 16.8 BLACK GRANITE 16.8 BLACK GRANITE  5100506 734923.1 4941508 239.1 4 ft 1.829 2 GPM Water Supply Domestic FRESH  2.44 RED CLAY 2.44 RED CLAY 2.44 RED CLAY 2.44 RED CLAY 3.05 GREY CLAY													
16.8 BLACK   GRANITE   18.8													
5100506 734923.1 4941508 239.1 4 ft 1.829 2 GPM Water Supply Domestic FRESH  2.44 RED CLAY 3.05 GREY CLAY													
2.44 RED CLAY 2.44 RED CLAY 2.44 RED CLAY 2.44 RED CLAY 3.05 GREY CLAY											16.8 BLACK	GRANITE	
2.44 RED CLAY 2.44 RED CLAY 2.44 RED CLAY 3.05 GREY CLAY	5100506	734923.1	4941508	8 239.1	1 4 ft	1.829	2 GPM	Water Supply	Domestic	FRESH			
2.44 RED CLAY 2.44 RED CLAY 3.05 GREY CLAY													
2.44 RED CLAY 3.05 GREY CLAY 13.1 BROWN GRANITE													
3.05 GREY CLAY 13.1 BROWN GRANITE													
3.05 GREY CLAY 3.05 GREY CLAY 3.05 GREY CLAY 13.1 BROWN GRANITE													
3.05 GREY CLAY 3.05 GREY CLAY 13.1 BROWN GRANITE													
3.05 GREY CLAY 13.1 BROWN GRANITE													
13.1 BROWN GRANITE													
13.1 BROWN GRANITE													
											13.1 BROWN	GRANIIE	

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,	Well I.D.	Easting	Northing	g Ele	ev. W	/L Foun	d (m) T	est Rate	Status	Use	Water Kind			ur Lith. Materials		
														GRANITE		
													BROWN	GRANITE		
													RED	GRANITE		
													RED	GRANITE		
													RED	GRANITE		
													RED	GRANITE		
													BLACK	GRANITE		
													BLACK	GRANITE		
													BLACK	GRANITE		
												21.3	BLACK	GRANITE		
;	5100507	734956.	1 49415	559 2	239.5	3 ft	6.706	1 GPM	Water Supply	Domestic	FRESH					
												0.61		TOPSOIL		
												0.61		TOPSOIL		
													RED	MEDIUM SAND		
													RED	MEDIUM SAND		
														GRANITE		
												15.2	BROWN	GRANITE		
;	5100508	734393.	1 49414	119 2	237.3	6 ft	38.71	0 GPM	Water Supply	Domestic	FRESH					
												0.30		TOPSOIL		
												0.30		TOPSOIL		
												4.88	BROWN	MEDIUM SAND		
													BROWN	MEDIUM SAND		
												5.49	)	GRANITE		HALE
												5.49		GRANITE	SH	HALE
												7.01	BROWN	GRANITE		
												7.01	BROWN	GRANITE		
												40.5	GREY	GRANITE		
												40.5	GREY	GRANITE		
;	5100510	736041.2	2 49420	036 2	245.9	7 ft	4.877	1 GPM	Water Supply	Domestic	FRESH					
												1.83		MEDIUM SAND		OULDERS
												1.83		MEDIUM SAND	BC	DULDERS
												5.49	RED	GRANITE		
												5.49	RED	GRANITE		
												6.1		SANDSTONE		
												6.1		SANDSTONE		
	5100511	735439.	1 49418	365 2	242.8	5 ft	5.486	1 GPM	Water Supply	Domestic	FRESH					
												1.83	3	MEDIUM SAND		
												1.83	3	MEDIUM SAND		
												6.40	BLUE	GRANITE		
												6.40	BLUE	GRANITE		

Monday, November 23, 2015

<b>Well I.D.</b> 5100512	<b>Easting N</b> 735971.2	lorthing E 4942063			n <b>d (m) T</b> o 7.62	est Rate \$	Status Water Supply	<b>Use</b> Domestic	Water Kind FRESH	Lith. Depth Lith	h. Colou	r Lith. Materials	
										0.30		TOPSOIL	
										0.30		TOPSOIL	
										2.44 BR	ROWN	CLAY	MEDIUM SAND
										2.44 BR	ROWN	CLAY	MEDIUM SAND
										18 REI	D	GRANITE	
										18 REI		GRANITE	
5101087	736795.2	4939036	241.1	11 ft	4.572	10 GPM	Water Supply	Domestic	FRESH				
										0.91		TOPSOIL	MEDIUM SAND
										0.91		TOPSOIL	MEDIUM SAND
										6.1 BR		LIMESTONE	
												LIMESTONE	
5101181	733064.1	4937823	248.8	1 ft	3.658	1 GPM	Water Supply	Domestic	FRESH	-			
										0.30		TOPSOIL	
										0.30		TOPSOIL	
										7.62 BL	ACK	GRANITE	
										7.62 BL		GRANITE	
5101182	732990.1	4938084	247	10 ft	6.401	0 GPM	Water Supply	Domestic	FRESH				
0.002	. 02000	.00000.			0	o <b>o</b>	rate. Capp.y	20000		2.74 REI	D	CLAY	BOULDERS
										2.74 REI		CLAY	BOULDERS
										9.14 BLU		GRANITE	
										9.14 BLU		GRANITE	
5101183	732854.1	4938413	249.2				Abandoned-Su	vlac		022		· · · · · · · ·	
0.01.00	70200 111	1000110	210.2				Abanaonoa Ca	2019		0.91		TOPSOIL	CLAY
										19.5 GR		GRANITE	
5101184	732802.1	4938601	241.5	16 ft	10.97	0 GPM	Water Supply	Domestic	FRESH				
0.00.	. 02002	.000001			. 0.0.	0 0	rate. Capp.y	20000		0.61		TOPSOIL	
										0.61		TOPSOIL	
										15.5 GR		GRANITE	
										15.5 GR		GRANITE	
5101196	733874.1	4938175	269.8	5 ft	3.658	GPM	Water Supply	Commerical	FRESH				
0.000		.000110	_00.0	0.1	0.000	<b>O</b>	rate. Capp.y	00		0.61		TOPSOIL	
										0.61		TOPSOIL	
										5.49 REI		LIMESTONE	
										5.49 REI		LIMESTONE	
5101197	733875.1	4938140	270.4	12 ft	10.36	2 GPM	Water Supply	Commerical	FRESH	0. 10 IVE			
0.01101	. 5557 511	.555110	0. 1		. 0.00	_ 3	a.c. Cappiy	23		11.6 REI	D	GRANITE	
										11.6 RE		GRANITE	
5101198	733904.1	4938032	270 6	2 ft	8.839	2 GPM	Water Supply	Domestic	FRESH				
0.01.00	. 0000 7.1	1000002	0.0		5.000	2 0	a.c. Cappiy	2011100110		2.44 REI	D	CLAY	SHALE
										2.44 REI		CLAY	SHALE
											•		· · · ·

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Well I.D.	Easting N	Northing Ele	ev. \	WL Fou	nd (m) Te	est Rate	Status	Use	Water Kind	Lith. Depth Lit 11 RE 11 RE	ED G	<b>Lith. Materials</b> RANITE RANITE	
F101100	702075 4	4020420	2242	154	10.10	1 GPM	Motor Cumbi	Domostio	FRESH	IIKE	<u>-</u> D G	IKANITE	
5101199	733875.1	4939128	234.2	1511	12.19	I GPIVI	Water Supply	Domestic	FRESH	2.44	C	LAY	MEDIUM SAND
										2.44		LAY	MEDIUM SAND
										14.9 BL		RANITE	MEDIOW ON WE
										14.9 BL		RANITE	
5101200	733305.1	4938768	233.3	9 ft	6.706	20 GPM	Water Supply	Domestic	FRESH	11.002	-01		
0.0.200	, , , , , , , , , , , , , , , , , , , ,	1000.00	_00.0	0	000		riato. Capp.,	20000		2.74	Т	OPSOIL	
										2.74		OPSOIL	
										3.05		HALE	
										3.05		HALE	
										8.23		RANITE	
										8.23		RANITE	
5101201	732999.1	4938683	232.8	16 ft	5.791	1 GPM	Water Supply	Domestic	FRESH				
										0.91	C	LAY	TOPSOIL
										0.91	C	LAY	TOPSOIL
										8.53 GF	REY L	IMESTONE	
										8.53 GF	REY L	IMESTONE	
5101204	735293.1	4938933	240.1	6 ft	18.29	0 GPM	Water Supply	Domestic	FRESH				
										5.49 RE	ED C	LAY	BOULDERS
										5.49 RE		LAY	BOULDERS
										8.53 RE	ED L	IMESTONE	
										8.53 RE	ED L	IMESTONE	
										12.2 BL		IMESTONE	
										12.2 BL	UE L	IMESTONE	
										23.5 WI	HITE L	IMESTONE	
										23.5 WI	HITE L	IMESTONE	
5101205	735275.1	4938758	239.8	10 ft	3.353	5 GPM	Water Supply	Domestic	FRESH				
										1.83		OPSOIL	STONES
										1.83		OPSOIL	STONES
										5.49 RE		RANITE	
										5.49 RE	ED G	RANITE	
5101206	735938.2	4939041	238.9	13 ft	5.182	2 GPM	Water Supply	Domestic	FRESH	0.74		DAY (5)	
										2.74		RAVEL	
										2.74		RAVEL	
										5.18 RE		RANITE	
F404007	705004.4	4000040	000.0	40 "	0.444	4.00%	Matan Original	Dames	EDEOU	5.18 RE	בט פ	RANITE	
5101207	735084.1	4939918	232.6	10 ft	9.144	1 GPM	Water Supply	Domestic	FRESH	12.8		RANITE	
										12.8		RANITE	
										12.0	G	II AINI L	

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<b>Well I.D.</b> 5101208	<b>Easting N</b> 735331.1	_			n <b>d (m) T</b> 9.144	est Rate	Status Water Supply	<b>Use</b> Domestic	Water Kind FRESH	Lith. Depth	Lith. Colour	Lith. Materials	
3101200	733331.1	4939912	200.0	Oit	3.144	OI W	Water Supply	Domestic	TICLOTT	1.22	RED CI	LAY	SHALE
												RANITE	J
5101209	735336.1	4939992	236.1	8 ft	16.15	1 GPM	Water Supply	Domestic	FRESH				
0.0.200		.000002		0.1			rate. Capp.)	20000		9.45	PI	REV. DRILLED	
										9.45	PI	REV. DRILLED	
										16.2	RED G	RANITE	
										16.2	RED G	RANITE	
5101210	735007.1	4940414	234.5	34 ft	13.41	2 GPM	Water Supply	Domestic	FRESH				
										1.52	CI	LAY	STONES
										1.52	CI	LAY	STONES
										14.0	G	RANITE	
										14.0	G	RANITE	
5101211	734597.1	4940016	237.8	92 ft	30.48	0 GPM	Water Supply	Domestic	FRESH				
										30.5	RED G	RANITE	
										30.5	RED G	RANITE	
5101214	735808.1	4940323	233.5	13 ft		1 GPM	Water Supply	Domestic	FRESH				
										0.30		OPSOIL	
										0.30		OPSOIL	
												EDIUM SAND	STONES
												EDIUM SAND	STONES
												RANITE	
										21.3	RED G	RANITE	
5101215	735613.1	4940176	233.5	1 ft	5.486	1 GPM	Water Supply	Domestic	FRESH			0000	25.11/51
										2.74		OPSOIL	GRAVEL
										2.74		OPSOIL	GRAVEL
												RANITE	
											RED G	RANITE	
5101216	736215.2	4939623	236.4				Abandoned-Su	pply	Not stated	d 0.61	т/	OPSOIL	
										0.61		OPSOIL OPSOIL	
										2.13			
												EDIUM SAND	
										2.13		EDIUM SAND	
										79.2		RANITE	
F404047	705005.4	4040407	000.0	0.4	F 400	4 CDM	Matan Commb	Damastia	EDECH	79.2	G	RANITE	
5101217	735805.1	4940407	∠3∠.9	эπ	5.486	1 GPM	Water Supply	Domestic	FRESH	1.52	Τſ	OPSOIL	MEDIUM SAND
										1.52		OPSOIL	MEDIUM SAND
												MESTONE	WILDIOW SAIND
												MESTONE	
5101218	736130.2	4940234	23/1 2	Λft	4.572	2 GPM	Water Supply	Domestic	FRESH	5.14	IVED II	IVILOTOINL	
3101210	730130.2	+340234	204.0	711	4.512	2 OI W	vvater Suppry	Domestic	TILOTT				

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Well I.D.	Easting N	Northing El	ev. V	VL Found (	(m) Test R	ate \$	Status	Use	Water Kind	Lith. Depth Lith. Colo		
										0.91	CLAY	TOPSOIL
										0.91	CLAY	TOPSOIL
										6.71 RED	GRANITE	
										6.71 RED	GRANITE	
5101219	735573.1	4940060	234.2	27 ft 1:	2.80 2.6	SPM	Water Supply	Livestock	FRESH			
										4.88	PREVIOUSLY DUG	
										4.88	PREVIOUSLY DUG	
										7.92	CLAY	BOULDERS
										7.92	CLAY	BOULDERS
										12.8 WHITE	LIMESTONE	
										12.8 WHITE	LIMESTONE	
5101224	737081.2	4939560	242.9	12 ft 7	.010 3 G	PM	Water Supply	Domestic	FRESH			
										4.27	MEDIUM SAND	
										4.27	MEDIUM SAND	
										7.32 BROWN	GRANITE	
										7.32 BROWN	GRANITE	
5101976	737337.2	4945453	304.5	8 ft 6	.706 1 0	SPM	Water Supply	Domestic	FRESH	.= . B. A.O.	00444	
										17.1 BLACK	GRANITE	
										17.1 BLACK	GRANITE	
5105016	732515.1	4938573	237.6	20 ft 2	4.38 0 0	SPM	Water Supply	Domestic	FRESH	0.00	TORCOIL	
										0.30	TOPSOIL	
										0.30	TOPSOIL	0701150
										1.83 BROWN	MEDIUM SAND	STONES
										1.83 BROWN	MEDIUM SAND	STONES
										33.5 GREY	GRANITE	
										33.5 GREY	GRANITE	
5105037	736035.2	4942923	246	20 ft 1	1.58	SPM	Water Supply	Domestic	FRESH	44.0	DDEV DDULED	
										14.0	PREV. DRILLED	
5405000	700045.4	40.40050	050.0	05 (1 0	0.50 0.6	D14	\\\-\-\-\-\-\-\-\-\-\-\-\-\-\-\-\-\-\-	Democific	EDEOU	32.0 GREY	GRANITE	
5105066	732015.1	4940853	252.2	35 π 2	6.52 3 G	PM	Water Supply	Domestic	FRESH	0.61 BROWN	SHALE	
										0.61 BROWN	SHALE	
										27.4 RED	GRANITE	
E40E007	704005 4	4040070	050.0	454 0	404 00	<b>.</b> DM	Matan Comple	Damastia	EDECH	27.4 RED	GRANITE	
5105067	731895.1	4940873	252.2	45 II 2	1.34 3 0	SPM	Water Supply	Domestic	FRESH	1.52 BROWN	MEDIUM SAND	
										1.52 BROWN	MEDIUM SAND	
										21.3 GREY	GRANITE	
											GRANITE	
5105300	734565.1	4938173	272.0	5# F	.486 5 0	SPM	Water Supply	Domestic	FRESH	21.3 GREY	GRANITE	
3103300	7 34303. I	4330113	213.0	3 IL 3	.+00 3 0	יר ווו	vvalei Suppiy	Domestic	FRESH	0.30	TOPSOIL	
										0.50	TOT SOIL	

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Well I.D.	Easting N	Northing El	lev.	WL Four	nd (m) To	est Rate	Status	Use	Water Kind	•	Colour Lith. Materials	
										0.30	TOPSOIL	
										2.74	LIMESTONE	
										2.74	LIMESTONE	
										6.71	LIMESTONE	
										6.71	LIMESTONE	
5105521	732615.1	4940603	238.1	30 ft	33.83	1 GPM	Water Supply	Domestic	FRESH			
										36.3 GRE		
										36.3 GRE	' GRANITE	
5105863	734380.1	4941373	235.9	20 ft	24.38	2 GPM	Water Supply	Domestic	FRESH			
										0.30	TOPSOIL	
										0.30	TOPSOIL	
										11 BROV	VN MEDIUM SAND	
										11 BROV	VN MEDIUM SAND	
										29 GRE	' GRANITE	
										29 GRE	GRANITE	
5105952	732815.1	4938448	248.4		32.00		Water Supply	Domestic	FRESH			
										0.30	TOPSOIL	
										0.30	TOPSOIL	
										0.61 BROV	VN CLAY	SAND
										0.61 BROV	VN CLAY	SAND
										32.0 GRE	GRANITE	
										32.0 GRE	' GRANITE	
										38.4 BLAC		
										38.4 BLAC		
5106047	736255.2	4939843	236.3	30 ft	10.97	1 GPM	Water Supply	Domestic	FRESH			
0.000	. 55255.2	.0000.0					riato. Cupp.,	20000		4.57	SAND	
										4.57	SAND	
										28.3	GRANITE	
										28.3	GRANITE	
5106074	736365.2	4941763	235.6	O ft	5 182	50 GPM	Water Supply	Domestic	FRESH	20.0	0.0.0.0.1	
0100074	700000.2	4041700	200.0	Oit	0.102	00 OI W	Water Cappiy	Domestio	TREOTT	1.83 BROV	VN CLAY	MEDIUM SAND
										1.83 BROV		MEDIUM SAND
										6.1	GRANITE	MEDIOM O, II VD
										6.1	GRANITE	
5106268	734725.1	4041422	226.6	20 ft	18.9	2 GPM	Water Supply	Domostic	FRESH	0.1	OKANITE	
3100200	7 347 Z3. I	4941423	230.0	20 II	10.9	Z GFIVI	Water Supply	Domestic	FRESH	0.61	TOPSOIL	
										0.61	TOPSOIL	
										19.5 RED	GRANITE	
E400440	7044004	40.44.400	220.4	£1	70.54	10.00%	Motor Commit	Domestic	EDECL	19.5 RED	GRANITE	
5106440	734160.1	4941488	∠36.1	ft	12.54	10 GPM	Water Supply	Domestic	FRESH	0.14 PDOL	VN SAND	CLAY
										9.14 BROV	VIN SAIND	CLAT

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Well I.D.	Easting N	Northing E	lev. V	VL Fou	nd (m) To	est Rate	Status	Use	Water Kind			ur Lith. Materials	
										9.1	4 BROWN	SAND	CLAY
										72.	5 BLACK	GRANITE	
											5 BLACK	GRANITE	
											2 BLACK	GRANITE	
										73.:	2 BLACK	GRANITE	
5106449	736859.2	4939623	238.9	6 ft	10.67	12 GPM	Water Supply	Domestic	FRESH				
											BROWN	SAND	
											BROWN	SAND	
											4 RED	GRANITE	
											4 RED	GRANITE	
											7 BLACK	GRANITE	
											7 BLACK	GRANITE	
											6 RED	GRANITE	
										11.0	6 RED	GRANITE	
5106572	733135.1	4938041	247.9	4 ft	21.95	8 GPM	Water Supply	Domestic	FRESH	0.0	4 DDOMAL	01.437	
											1 BROWN	CLAY	
											1 BROWN	CLAY	
											5 RED	STONES	
											5 RED	STONES	
											7 GREY	STONES	
											7 GREY	STONES	
											3 BLACK	STONES	
											3 BLACK	STONES	
											3 GREY	LIMESTONE	
5400570	700007.4	4000405	040.0	0.11	0.700	4.0004	W-1 0	Damasiis	EDEOU	22.	3 GREY	LIMESTONE	
5106573	733067.1	4938495	246.3	8 π	6.706	1 GPM	Water Supply	Domestic	FRESH	0.9	1	FILL	
										0.9		FILL	
											5 BROWN	SHALE	
											5 BROWN	SHALE	
											2 GREY	SANDSTONE	
											2 GREY	SANDSTONE	
5106611	735204.1	4939827	226.6	5 ft	10.67	15 GPM	Water Supply	Domestic	FRESH	12	ZGKLI	SANDSTONE	
3100011	733204.1	4939021	230.0	Jit	10.07	13 GFINI	water Supply	Domestic	FRESH	0.30	BLACK	TOPSOIL	
											BLACK	TOPSOIL	
											9 GREY	GRANITE	
											9 GREY	GRANITE	
											1 RED	GRANITE	
											1 RED	GRANITE	
											6 GREY	GRANITE	
											·· <b>-</b> ·	=:= " =	

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Well I.D.	Easting N	Northing E	lev. V	VL Foun	nd (m) Te	est Rate S	Status	Use	Water Kind	Lith. Depth Lith. Cold	ur Lith. Materials GRANITE	
5106694	735038.1	4943883	261.0	35 ft	35 36	45 GPM	Water Supply	Domestic	FRESH	11.0 GRET	GRANITE	
3100034	7 3 3 0 3 0 . 1	4343003	201.5	55 It	55.50	45 OI W	water cupply	Domestic	TILOTT	1.22	SAND	
										1.22	SAND	
										36	LIMESTONE	
										36	LIMESTONE	
5106990	735286.1	4939669	240.6				Abandoned-Supp	oly				
								•		0.30	TOPSOIL	
										0.61 BROWN	SAND	
										1.52 BROWN	SAND	SHALE
										92.0 GREY	GRANITE	
5106991	735281.1	4939670	240.5	5 ft	46.63	10 GPM	Water Supply	Domestic	FRESH			
										0.30	TOPSOIL	
										0.30	TOPSOIL	
										0.91 BROWN	CLAY	SAND
										0.91 BROWN	CLAY	SAND
										45.7 GREY	GRANITE	
										45.7 GREY	GRANITE	
										47.2 RED	GRANITE	
										47.2 RED	GRANITE	
5107181	733734.1	4938914	236.8	20 ft	15.24	0 GPM	Water Supply	Domestic	FRESH			
										0.30	TOPSOIL	
										0.30	TOPSOIL	
										2.44 BROWN	CLAY	
										2.44 BROWN	CLAY	
										10.7 GREY	LIMESTONE	
										10.7 GREY	LIMESTONE	
										18.3 GREY	GRANITE	
										18.3 GREY	GRANITE	
5107209	734501.1	4939205	238.2	4 ft	13.41	2 GPM	Water Supply	Domestic	FRESH			
										0.91	PREVIOUSLY DUG	
										0.91	PREVIOUSLY DUG	
										2.44 GREY	SHALE	
										2.44 GREY	SHALE	
										6.1 RED	GRANITE	
										6.1 RED	GRANITE	
										14.0 BLACK	GRANITE	
						_		_		14.0 BLACK	GRANITE	
5107215	736578.2	4941540	238.1	40 ft	35.66	20 GPM	Water Supply	Domestic	FRESH	0.30	TOPSOIL	

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Well I.D.	Easting N	lorthing E	lev. WL Fo	ound (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold		
									0.30	TOPSOIL GRANITE	
									41.1 RED 41.1 RED	GRANITE	
E107226	724760 4	4029400	256 / 50#	20.1	8 GPM	Water Supply	Domostio	FRESH	41.1 KED	GRANITE	
5107226	734768.1	4930490	256.4 50 ft	38.1	o GPIVI	water Supply	Domestic	FRESH	6.40	PREVIOUSLY DUG	
									6.40	PREVIOUSLY DUG	
									18.3 GREY	LIMESTONE	
									18.3 GREY	LIMESTONE	
									38.7 RED	GRANITE	
									38.7 RED	GRANITE	
5107244	730830.1	4940157	240.3 18 ft	33.53	1 GPM	Water Supply	Domestic	FRESH	50 K.=5	O	
0.0.2				00.00		rate. Capp.,	20000		0.30	TOPSOIL	
									0.30	TOPSOIL	
									2.44 BROWN	SAND	CLAY
									2.44 BROWN	SAND	CLAY
									6.1 GREY	LIMESTONE	
									6.1 GREY	LIMESTONE	
									24.4 GREY	GRANITE	
									24.4 GREY	GRANITE	
									35.1 BLACK	GRANITE	
									35.1 BLACK	GRANITE	
5107441	735265.1	4939723	235.4+02 ft	76.2	3 GPM	Water Supply	Domestic	FRESH			
									1.52	SAND	FILL
									3.66 BLACK	GRANITE	SHALE
									24.4 BLACK	GRANITE	HARD
									27.4 RED	GRANITE	HARD
									102 BLACK	GRANITE	HARD
5107502	735265.1	4939673	239.8 18 ft	27.43	8 GPM	Water Supply	Domestic	FRESH			
									2.44 BROWN	SAND	STONES
									27.4 BLACK	GRANITE	HARD
									28.3 RED	GRANITE	HARD
5107510	734015.1	4941523	237.7 10 ft	45.11	20 GPM	Water Supply	Domestic	FRESH		0.4415	
									3.66	SAND	
									3.66	SAND	
									8.53 BLUE	SAND	WATER BEARING
									8.53 BLUE	SAND	WATER-BEARING
									47.5 RED	GRANITE	
E407070	700745 4	4000070	000 0 40 "	00.04	00.0014	Matan Orași	Daws	EDEO!!	47.5 RED	GRANITE	
5107679	733715.1	4938873	239.3 40 ft	89.31	20 GPM	Water Supply	Domestic	FRESH	22.9	PREV. DRILLED	
									<b>22.</b> 3	I ILV. DIVILLED	

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Well I.D.	Easting N	lorthing E	lev. WL F	Found (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Col		
									88.4 GREY	GRANITE	
									88.7 RED	GRANITE	
									91.4 WHITE	QUARTZ	
5108087	736565.2	4939623	238.5 10 f	t 14.33	2 GPM	Water Supply	Domestic	FRESH	0.20 DDOWN	SAND	LOOSE
									0.30 BROWN 0.30 BROWN		LOOSE
									10.4 RED	SAND GRANITE	HARD
											HARD
									10.4 RED	GRANITE	
									16.2 BLACK	GRANITE	HARD
E400000	704405.4	40.40700	040.0.004	4 7045	2 CDM	Matan Comple	Damastia	EDECH	16.2 BLACK	GRANITE	HARD
5108360	731165.1	4940723	240.6 22 f	t 7.315	3 GPM	Water Supply	Domestic	FRESH	2.44 WHITE	GRANITE	
									9.75 RED	GRANITE	
									17.7 WHITE	GRANITE	
5108654	735815.1	4042022	243.6+02 f	t 57.61	1 GPM	Motor Cupply	Domestic	FRESH	17.7 VVIIIE	GRANITE	
3100034	733013.1	4942023	243.0 +02 1	1 37.01	I GFIVI	Water Supply	Domestic	FRESH	7.92 BROWN	SAND	
									7.92 BROWN	SAND	
									12.2 GREY	GRANITE	
									12.2 GREY	GRANITE	
									60.0	GRANITE	
									60.0	GRANITE	
5108664	736365.2	4941573	234.4 20 f	t 11.58	2 GPM	Water Supply	Domestic	FRESH	00.0	OKANITE	
3100004	730303.2	4341373	254.4 201	11.50	2 OI W	Water Supply	Domestic	TICLOTT	0.61 BROWN	SAND	SOFT
									2.74 BLACK	GRANITE	HARD
									16.2 BLACK	GRANITE	HARD
5108724	734765.1	4941423	235.4 40 f	t 67.97	4 GPM	Water Supply	Domestic	FRESH	10.2 02 1010	GIVANIE	TIME
0100721	701700.1	1011120	200.1 101	07.07	. 0	Water Cappiy	Bomodio		1.83 BROWN	SAND	SOFT
									31.1 BLACK	GRANITE	HARD
									33.2 RED	GRANITE	HARD
									65.8 BLACK	GRANITE	HARD
									68 RED	GRANITE	HARD
5108853	732665.1	4940723	239.8 35 f	t 31.7	10 GPM	Water Supply	Domestic	FRESH			
									1.22 BROWN	TOPSOIL	
									33.5 RED	GRANITE	
5109029	733865.1	4938023	269.6 15 f	t 8.23	8 GPM	Water Supply	Domestic	FRESH			
						,			0.30 BLACK	TOPSOIL	SOFT
									8.23 GREY	LIMESTONE	HARD
									8.53 BROWN	LIMESTONE	SOFT
									11.3 GREY	LIMESTONE	HARD
									14.9 GREY	GRANITE	HARD
5109064	736165.2	4938823	242.5 15 f	t 52.73	20 GPM	Water Supply	Domestic	FRESH			

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Well I.D.	Easting	Northina	Elev.	WL Four	nd (m)	Test Rate	Status	Use	Water Kind	Lith. Depth Lith. Colo	ur Lith. Materials	
					,					3.05	SAND	
										3.05	SAND	
										4.57 RED	SANDSTONE	
										4.57 RED	SANDSTONE	
										39.6 WHITE	QUARTZ	
										39.6 WHITE	QUARTZ	
										52.7	GRANITE	
										52.7	GRANITE	
										53.9 RED	GRANITE	
										53.9 RED	GRANITE	
5109065	734165.1	4941473	3 235.0	25 ft	14.94	10 GPM	Water Supply	Domestic	FRESH			
										0.61	SAND	
										0.61	SAND	
										16.5 RED	GRANITE	
										16.5 RED	GRANITE	
5109087	736365.2	4941973	3 237.1	1 3 ft	10.36	GPM	Water Supply	Domestic	FRESH			
										3.05	SAND	
										3.05	SAND	
										11.3 RED	GRANITE	
										11.3 RED	GRANITE	
5109088	735765.1	4942073	3 244.3	3 11 ft	8.23	18 GPM	Water Supply	Domestic	FRESH		0.115	0./555.1555.1
										0.30 BROWN	SAND	OVERBURDEN
										0.30 BROWN	SAND	OVERBURDEN
										9.14 RED	ROCK	GRANITE
5400000	705745.4	40.4407			4400	0.0014		<b>5</b>	EDE011	9.14 RED	ROCK	GRANITE
5109089	/35/15.1	4941873	3 238.6	25π	14.63	0 GPM	Water Supply	Domestic	FRESH	8.53 BROWN	SILT	SAND
										8.53 BROWN		SAND
										14.6 RED	ROCK	SHALE
										14.6 RED	ROCK	SHALE
										27.4 RED	ROCK	GRANITE
										27.4 RED	ROCK	GRANITE
5109092	735715 1	4942023	3 243 3	2 ft	17.68	GPM	Water Supply	Domestic	FRESH	ZI.TILD	ROOK	SIVAINTE
3103032	755715.1	7572020	2 - 2 - 2 - 2	- 10	17.00	OI W	water Supply	Domestic	TREON	6.40	SAND	CLAY
										6.40	SAND	CLAY
										6.40	SAND	CLAY
										6.40	SAND	CLAY
										21.3 GREY	ROCK	
										21.3 GREY	ROCK	
										21.3 GREY	ROCK	
										· · · <del>- ·</del>		

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Well I.D.	Easting	Northing E	lev.	WL Fou	nd (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Colour Lith	ı. Materials	
5109100	733915.1	4938023	271	38 ft	11.58	1 GPM	Water Supply	Domestic	FRESH	2.10 0.12		
										0.30 BROWN TOPSO	IL	SOFT
										21.6 GREY LIMEST	ONE	POROUS
										35.4 RED LIMEST	ONE	POROUS
										39.3 RED GRANIT	Έ	HARD
										42.7 GREEN GRANIT	Έ	HARD
										46.6 GREY GRANIT	E.	HARD
5109134	736515.2	4939523	237.9	12 ft	38.71	30 GPM	Water Supply	Domestic	FRESH			
										0.61 BROWN SAND		SOFT
										38.7 RED GRANIT	Έ	HARD
5109176	736265.2	4938823	242.7	6 ft	10.67	30 GPM	Water Supply	Domestic	Not stated			
										1.83 BROWN CLAY		
										1.83 BROWN CLAY		
										10.7 GREY LIMEST		
										10.7 GREY LIMEST	ONE	
5109187	736915.2	4940323	235.5	20 ft	19.51	10 GPM	Water Supply	Domestic	FRESH	40.5.050		
										19.5 RED GRANIT		
										19.5 RED GRANIT		
										21.3 BROWN GRANIT		
										21.3 BROWN GRANIT		
										22.6 GREY GRANIT		
F100220	700005 0	4040470	252.6	CE #	62.4	1 CDM	Motor Cumply	Municipal	EDECH	22.6 GREY GRANIT	Е	
5109230	736265.2	4942173	253.6	п со	63.4	1 GPM	Water Supply	Municipal	FRESH	3.35 BROWN SAND		SOFT
										3.35 BROWN SAND		SOFT
										7.32 GREY SAND		GRAVEL
										7.32 GREY SAND		GRAVEL
										65.5 BLACK GRANIT	·F	HARD
										65.5 BLACK GRANIT		HARD
5109294	730265.1	4939473	248.5	4 ft	19.81	2 GPM	Water Supply	Domestic	FRESH	00.0 22 (0).	_	11,1110
0.0020.		1000110	0.0				riato. Capp.y	20000		0.30 TOPSO	IL	
										7.32 BROWN CLAY		SANDY
										19.8 BLACK GRANIT	Έ	STONES
										30.5 GREY GRANIT	Έ	
5109459	735665.1	4939923	238.4	0 ft	6.096	10 GPM	Water Supply	Domestic	FRESH			
										0.61 BROWN TOPSO	IL	SOFT
										2.44 BROWN SAND		STONES
										3.35 GREY SHALE		STONES
										6.1 GREY STONE	S	HARD
5109560	730715.1	4940323	233.8	15 ft	8.23	24 GPM	Water Supply	Domestic	FRESH			

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Well I.D.	Easting N	lorthina E	lev. V	<b>VL</b> Fou	nd (m) T	est Rate S	Status	Use	Water Kind	Lith. Depth Lith. Colo	our Lith. Materials	
	J	·			` ,					0.30 BROWN		SOFT
										3.66 BROWN	GRAVEL	STONES
										8.23 GREY	GRANITE	HARD
5109615	736415.2	4941873	235.7	5 ft	27.74	3 GPM	Water Supply	Domestic	FRESH			
0.000.0				0		o o	rate. Capp.)	20000		1.83	FILL	
										1.83	FILL	
										29 RED	GRANITE	
										29 RED	GRANITE	
5109755	736515.2	4941923	237.8	3 ft	11 58	15 GPM	Water Supply	Domestic	FRESH	201122	O	
0100100	700010.2	1011020	201.0	011	11.00	10 01 111	Water Cupply	Bomoodo		5.18	SAND	GRAVEL
										5.18	SAND	GRAVEL
										11.3 GREY	GRANITE	0.0
										11.3 GREY	GRANITE	
										11.9 RED	GRANITE	
										11.9 RED	GRANITE	
5109980	737015.2	4939723	227 /	7 ft	5.486	20 GPM	Water Supply	Domestic	FRESH	II.9KLD	OKANITE	
3109960	737013.2	4939123	231.4	7 11	5.460	20 GFIVI	water Supply	Domestic	FRESH	3.35 BROWN	SAND	LIGHT-COLOURED
										3.35 BROWN	SAND	LIGHT-COLOURED
										6.71 RED	GRANITE	HARD
										6.71 RED	GRANITE	HARD
5109981	737015.2	4939773	2/1 0	2 ft	3.048	8 GPM	Water Supply	Domestic	Not stated		OKANITE	TIAND
3109961	737013.2	4939113	241.0	311	3.040	0 GFIVI	water Supply	Domestic	พบเ รเลเซน	0.30 BROWN	TOPSOIL	SOFT
										1.83 BROWN	SAND	CLAY
										7.92 RED	GRANITE	HARD
5109982	737015.2	4939823	225.7	2 #	9.754	5 GPM	Water Supply	Domestic	FRESH	7.92 NLD	GRANITE	HAND
5109962	737015.2	4939023	233.7	311	9.754	3 GPIVI	water Supply	Domestic	FRESH	0.30 BROWN	TOPSOIL	SOFT
										2.74 BROWN	SAND	CLAY
										9.75 BLACK	GRANITE	HARD
5110006	731715.1	4940723	225 1	15 ft	18.9	4 GPM	Water Supply	Domestic	FRESH	9.73 BLACK	GRANITE	HAND
3110006	731713.1	4940723	233.1	1311	10.9	4 GPIVI	water Supply	Domestic	FRESH	1.52	TOPSOIL	
										18 WHITE	GRANITE	HARD
										18.9 RED	GRANITE	HARD
5110011	736915.2	4939723	224.4	2 ft	5.791	25 GPM	Water Supply	Domestic	Not stated		OKANITE	TIAND
3110011	730913.2	4939123	234.4	211	5.791	25 GFIVI	water Supply	Domestic	พบเ รเลเซน	0.30 BROWN	TOPSOIL	DARK-COLOURED
										3.66 BROWN	SAND	CLAY
										5.79 RED	GRANITE	HARD
5110256	732315.1	4940623	251.6	40 ft	60.66	15 GPM	Water Supply	Domestic	FRESH	3.73 NLD	CIVAINIL	HAND
3110230	132313.1	4340023	231.0	<del>-1</del> 0 π	00.00	13 GF W	water Supply	Domestic	FNLOH	1.22 BROWN	FILL	SOFT
										1.22 BROWN	FILL	SOFT
										59.4 WHITE	GRANITE	HARD
										59.4 WHITE	GRANITE	HARD
										JJ.4 WITH	CIVAINIL	ווחוזט

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W	ell I.D.	Easting	Northing	Elev.	WL Fou	und (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold		
											60.7 BROWN	GRANITE	HARD
											60.7 BROWN	GRANITE	HARD
51	10261	736915.2	2 4939723	3 234.4	4 ft	6.401	20 GPM	Water Supply	Domestic	FRESH	0.00 PL ACK	TODCOII	COFT
											0.30 BLACK 1.52 BROWN	TOPSOIL SAND	SOFT SOFT
											6.71 BLACK	GRANITE	HARD
51	10285	734415.1	1 4941023	3 234	20 ft	10.38	10 GPM	Water Supply	Domestic	FRESH	0.71 BLACK	OKANITE	HAND
01	10203	754415.	1 4541020	204	2011	45.50	10 OI W	water Supply	Domestic	TREON	0.61 BROWN	TOPSOIL	SOFT
											0.61 BROWN	TOPSOIL	SOFT
											50.3 RED	GRANITE	HARD
											50.3 RED	GRANITE	HARD
51	10344	734615.1	1 4939223	3 234.5	12 ft	13.41	10 GPM	Water Supply	Domestic	Not stated			
											4.88 BROWN	CLAY	SAND
											13.7 RED	GRANITE	
51	10484	730615.1	1 4939923	3 237.3				Abandoned-Su	ipply				
											5.49 BROWN	SAND	SHALE
											61 GREY	GRANITE	
51	10570	735215.1	1 4939323	3 240.0	8 ft	4.877	2 GPM	Water Supply	Domestic	FRESH			
											0.30	TOPSOIL	
											4.27 BROWN	FINE SAND	
											4.88 BROWN	COARSE GRAVEL	
											7.32 GREY	LIMESTONE	
E 1	10504	700745 (	402002		0.4	44.00	8 GPM	Motor Cumply	Domostia	FRESH	8.53 WHITE	GRANITE	
51	10584	730713.2	2 4939823	233.0	OIL	11.28	6 GPIVI	Water Supply	Domestic	FRESH	0.30 BROWN	TOPSOIL	
											0.30 BROWN	TOPSOIL	
											0.30 BROWN	TOPSOIL	
											0.30 BROWN	TOPSOIL	
											13.7 BLACK	GRANITE	
											13.7 BLACK	GRANITE	
											13.7 BLACK	GRANITE	
											13.7 BLACK	GRANITE	
51	10585	736715.2	2 4939723	3 233.6	20 ft	44.50	40 GPM	Water Supply	Domestic	FRESH			
											0.61 BROWN	SAND	
											0.61 BROWN	SAND	
											9.14 RED	GRANITE	
											9.14 RED	GRANITE	
											14.6 GREEN	GRANITE	
											14.6 GREEN	GRANITE	
											45.1	GRANITE	
											45.1	GRANITE	

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<b>Well I.D.</b> 5110586	<b>Easting N</b> 736765.2	lorthing E 4939723				est Rate 40 GPM		<b>Use</b> Domestic	Water Kind FRESH	Lith. Depth	Lith. Colou	r Lith. Materials	
							117			0.61	1 BROWN	SAND	
										0.61	1 BROWN	SAND	
										14.0	GREEN	GRANITE	
										14.0	GREEN	GRANITE	
										40.8	3 RED	GRANITE	
										40.8	3 RED	GRANITE	
5110615	737015.2	4939723	237.4	2 ft	18.59	20 GPM	Water Supply	Domestic	Not stated				
												SAND	SOFT
												SAND	SOFT
										18.6		GRANITE	HARD
												GRANITE	HARD
												GRANITE	
												GRANITE	
												GRANITE	
												GRANITE	
												GRANITE	
												GRANITE	
												GRANITE	
												GRANITE	
												GRANITE	
										75.3	3 RED	GRANITE	
5110680	734415.1	4941323	235	4 ft	8.534	8 GPM	Water Supply	Domestic	FRESH				0.115./
												TOPSOIL	SANDY
												TOPSOIL	SANDY
												MARL	SANDY
												MARL	SANDY
										8.84		MEDIUM GRAVEL	SANDY
										8.84		MEDIUM GRAVEL	SANDY
												GRANITE	STONES
												GRANITE	STONES
												GRANITE	QUARTZ
5440000	700045.0	4000700	040.7	0.11	4.504	5 ODM	\\\-\ \ - \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	Democific	EDEOU	28.7	7 GREY	GRANITE	QUARTZ
5110693	736615.2	4939723	240.7	2π	1.524	5 GPM	Water Supply	Domestic	FRESH	0.61	1 BLACK	CLAY	FILL
												GRANITE	LAYERED
5110725	736115.2	4940273	234.6	6 ft	11.28	5 GPM	Water Supply	Domestic	FRESH	3.14		ONAINTE	LAILNED
3110723	130113.2	7370213	204.0	υπ	11.20	JOFIN	water Supply	Domestic	INLOH	0.61	1 BROWN	SAND	
												SAND	
											2 BROWN		
										12.2		J. J. 120	

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Well I.D.	Easting N	lorthing E	lev. \	WL Fou	nd (m) T	est Rate S	Status	Use	Water Kind	•	Lith. Colo	ur Lith. Materials		
											GREEN	STONES		
E440700	7047054	40.44.400	005.4	4 4	0.444	0 CDM	Matan Cumulu	Damastia	Niet etete d		GREEN	STONES		
5110768	734765.1	4941423	235.4	тπ	9.144	3 GPM	Water Supply	Domestic	Not stated		BROWN	MEDIUM SAND		
											BROWN	MEDIUM SAND		
											GREY	MEDIUM SAND		
											GREY	MEDIUM SAND		
											BLACK	GRANITE	HARD	
											BLACK	GRANITE	HARD	
											RED	GRANITE	HARD	
											RED	GRANITE	HARD	
											BLACK	GRANITE	HARD	
											BLACK	GRANITE	HARD	
5110814	736865.2	4939823	234	20 ft	28.96	2 GPM	Water Supply	Domestic	Not stated		22.0.0	· · · · · · · ·		
000	. 00000.2	.000020	_0.	_0	20.00	_ 0	rate. Cupp.)	20000	. 101 014104		BLACK	GRANITE	MEDIUM-	GRAINED
										29	BLACK	GRANITE	MEDIUM-	GRAINED
											RED	GRANITE	MEDIUM-	GRAINED
										34.1	RED	GRANITE	MEDIUM-	GRAINED
5110849	730615.1	4940273	236.0	5 ft	22.56	12 GPM	Water Supply	Domestic	Not stated					
										2.13	BROWN	SAND	GRAVEL	
										5.79	BLACK	GRANITE	HARD	
										22.3	GREEN	GRANITE	HARD	
										22.6	RED	GRANITE	HARD	
5111033	735215.1	4939323	240.0	3 ft	5.791	1 GPM	Water Supply	Domestic	FRESH					
										7.92		PREV. DRILLED		
										14.3	GREY	GRANITE		
5111240	735495.6	4941848	241.4	6 ft	1.829	2 GPM	Water Supply	Domestic	FRESH					
											BROWN	SAND	STONES	
										13.4	RED	GRANITE		
5111457	736677.2	4945068	265.7	9 ft	8.534	30 GPM	Water Supply	Domestic	FRESH	4.50	VELLOW/	CAND	CDANITE	
											YELLOW		GRANITE	
E44447E	700007.4	4040004	055.0	20.4	40.70	4 CDM	Matan Cumulu	Damastia	Nint state d		GREY	GRANITE		
5111475	732097.1	4940921	255.9	20 π	13.72	1 GPM	Water Supply	Domestic	Not stated		WHITE	GRANITE	ROCK	
											BROWN	GRANITE	ROCK	
											BLACK	GRANITE	ROCK	
											WHITE	GRANITE	ROCK	
5111577	731548.1	4936593	248 4	O ft	12.19	30 GPM	Water Supply	Domestic	Not stated		VVI II I L	CIVAINIL	NOCK	
3111377	131340.1	4330333	240.4	υπ	12.19	JU GF W	vvalei Suppiy	Domestic	เพอเ รเสเซน		BROWN	CLAY	STONES	
											RED	GRANITE	ROCK	
										10.1		<b>○.</b> ₩₩₩	110011	

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Well I.D.	Easting N	_						Use	Water Kind	Lith. Depth Lith. Colo	ur Lith. Materials	
5111655	732097.1	4940921	255.9	14 ft	28.04	1 GPM	Water Supply	Domestic	FRESH	0.00 DD 0.444	0041/51	CANDY
										3.66 BROWN		SANDY
										3.66 BROWN	GRAVEL	SANDY
										6.71 RED	GRANITE	
										6.71 RED	GRANITE	
										13.7 GREY	GRANITE	
										13.7 GREY	GRANITE	
										19.8 RED	GRANITE	
										19.8 RED	GRANITE	
										22.6 GREY	GRANITE	
										22.6 GREY	GRANITE	
										26.2 RED	GRANITE	
										26.2 RED	GRANITE	
										28.7 GREY	GRANITE	
										28.7 GREY	GRANITE	
										31.1 GREY	GRANITE	
										31.1 GREY	GRANITE	
										53.0 RED	GRANITE	
										53.0 RED	GRANITE	
5112155	730593.1	4936911	239.3	28 ft	14.63	10 GPM	Water Supply	Domestic	FRESH	0.00 DD 014/14	TODOOU	
											TOPSOIL	
										0.30 BROWN	TOPSOIL	2001
										3.05 BROWN	SHALE	ROCK
											SHALE	ROCK
										16.2 BROWN		SHALE
										16.2 BROWN	STONES	SHALE
5112262	732513.1	4946191	277.4	0 ft	20.73	10 GPM	Water Supply	Domestic	FRESH	4.00	CANDOTONE	
										1.83	SANDSTONE	
										1.83	SANDSTONE	
										25.3 RED	GRANITE	
										25.3 RED	GRANITE	
5112277	736567.2	4939997	233.4	8 ft	19.81	5 GPM	Water Supply	Domestic	Not stated		MEDILIM CAND	
										0.91 BROWN	MEDIUM SAND	11400
5440004	700507.0	4000007	000.4	4.4.6	00.00	0.0004	Matan Orașile	Damasiis	NI-1-1-1-1	20.1 GREY	GRANITE	HARD
5112324	736567.2	4939997	233.4	14π	82.60	8 GPM	Water Supply	Domestic	Not stated	1.83 BROWN	FILL	MEDIUM-GRAINED
										83.8 GREY		HARD
5112325	736567.2	4939997	222.4	A #4	50.29	2 GPM	Water Supply	Domestic	Not stated		GRANITE	HARD
5112325	730307.2	4939991	233.4	411	30.29	2 GPIVI	Water Supply	Domestic	NOI Stated	2.13 BROWN	MEDIUM SAND	
										51.8 GREY	LIMESTONE	LAYERED
5112340	732007 1	4940021	255 0	10 ft	12 10	3 GPM	Water Supply	Domestic	FRESH	JI.O OILE	LINILOTOINL	LATENED
3112310	. 02007.11	.010021	_00.0		12.10	5 01 171	a.c. Cappiy	2011100110				
5112349	732097.1	4940921	255.9	10 ft	12.19	3 GPM	Water Supply	Domestic	FRESH			

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Well I.D.	Easting N	lorthing E	lev. \	<b>NL</b> Fou	nd (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold		
										0.61	SAND	
										0.61	SAND	
										19.2 WHITE	GRANITE	
										19.2 WHITE	GRANITE	
5112350	732097.1	4940921	255.9	15 ft	12.19	3 GPM	Water Supply	Domestic	FRESH	0.04	ODAVE!	
										0.91	GRAVEL	
										0.91	GRAVEL	
										19.2	GRANITE	
E440070	7054404	4000400	0.40.4	0.6	00.00	4.0014		5	EDEOL	19.2	GRANITE	
5112370	735110.1	4939499	240.4	ьπ	32.00	1 GPM	Water Supply	Domestic	FRESH	0.30	SAND	
										54.9 GREY	GRANITE	
E110/10	720422.6	4040242	241	10 ft	26 50	4 GPM	Water Supply	Domostio	Not stated	54.9 GRE 1	GRANITE	
5112413	730432.6	4940313	241	12 ft	36.58	4 GPIVI	Water Supply	Domestic	Not stated	0.61 BROWN	FILL	MEDIUM-GRAINED
										38.4 GREY	GRANITE	HARD
5112499	736567.2	4939997	233 4	30 ft	30.48	8 GPM	Water Supply	Domestic	Not stated	30.4 GIVE I	OIVAINIL	HARD
3112433	730307.2	4000001	200.4	30 II	30.40	O OI W	Water Supply	Domestic	Not Stated	0.30 GREY	GRAVEL	LOOSE
										32.3 RED	GRANITE	HARD
5112612	730288.8	4939733	254.3	23 ft	42.67	5 GPM	Water Supply	Domestic	Not stated			
										0.91 BROWN	FILL	MEDIUM-GRAINED
										44.5 GREY	GRANITE	HARD
5112701	731548.1	4936593	248.4	18 ft	48.77	2 GPM	Water Supply	Domestic	Not stated			
										0.30 BROWN	TOPSOIL	SOFT
										21.6 RED	GRANITE	HARD
										57.6 BLACK	GRANITE	HARD
5112879	731814.1	4938445	233.5	30 ft	35.97	4 GPM	Water Supply	Domestic	Not stated			
										1.83 BROWN	FILL	MEDIUM-GRAINED
										38.4 GREY	GRANITE	HARD
5113023	735495.6	4941848	241.4	12 ft	24.38	10 GPM	Water Supply	Domestic	FRESH			
										7.92	QUICKSAND	
										7.92	QUICKSAND	
										36.6 BLACK	GRANITE	
										36.6 BLACK	GRANITE	
5113190	732097.1	4940921	255.9	3 ft	102.1	25 GPM	Water Supply	Domestic	FRESH			
										0.91	SAND	
										102 WHITE	GRANITE	
5113199	731548.1	4936593	248.4	8 ft	20.12	1 GPM	Water Supply	Domestic	Not stated	4 07 DDO\\\\	EILI	MEDILIM ODAINED
										4.27 BROWN	FILL	MEDIUM-GRAINED
										4.27 BROWN	FILL	MEDIUM-GRAINED
										39.9 GREY	GRANITE	HARD
										39.9 GREY	GRANITE	HARD

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Well I.D.	Easting N	lorthing E	lev. WL Fo	und (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Colo	ur Lith. Materials	
	•			` ,					47.9 BLACK	GRANITE	HARD
									47.9 BLACK	GRANITE	HARD
									51.8 RED	GRANITE	HARD
									51.8 RED	GRANITE	HARD
5113757	732513.1	4946191	277.4 10 ft	12.19	7 GPM	Water Supply	Domestic	FRESH			
						117			0.61	CLAY	
									0.61	CLAY	
									12.2	LIMESTONE	
									12.2	LIMESTONE	
									38.1 GREY	GRANITE	
									38.1 GREY	GRANITE	
5113764	730432.6	4940313	241			Water Supply	Domestic	FRESH			
						,			0.61	SAND	
									85.3 BLACK	GRANITE	
5113850	736567.2	4939997	233.4 15 ft	80.77	0 GPM	Water Supply	Domestic	FRESH			
									2.13 BROWN	SAND	
									85.3 GREY	GRANITE	
5113851	736567.2	4939997	233.4 2 ft	35.66	4 GPM	Water Supply	Domestic	FRESH			
									1.22 BROWN	SAND	
									38.1 GREY	GRANITE	
5113961	736787.2	4939383	243.2 18 ft	20.73	25 GPM	Water Supply	Domestic	FRESH			
									20.7 BLACK	GRANITE	HARD
									20.7 BLACK	GRANITE	HARD
5114007	730432.6	4940313	241 13 ft	19.20	5 GPM	Water Supply	Domestic	FRESH	4.00 P.P.O.W.	0)/500/10051	
									1.22 BROWN	OVERBURDEN	
									19.2 BROWN	ROCK	
5114008	731548.1	4936593	248.4 21 ft	10.97	16 GPM	Water Supply	Domestic	FRESH	2.74.DDOWN	OVEDBUBBEN	
									2.74 BROWN	OVERBURDEN	
									2.74 BROWN	OVERBURDEN	LIMESTONE
									10.1 GREY	ROCK	LIMESTONE
									10.1 GREY	ROCK	LIMESTONE
									17.7 RED	ROCK	SHALE
E44440E	700077.0	40.45000	005.7			Matan Comple	Damastia		17.7 RED	ROCK	SHALE
5114165	730077.2	4945068	265.7			Water Supply	Domestic		2.44 BROWN	SAND	
									158 BLACK	GRANITE	GRANITE
5114295	730432.6	4940313	241 20 ft	48.77	20 GPM	Water Supply	Domestic	FRESH	130 BLACK	OKANIL	OKANITE
3114233	130432.0	<del>494</del> 0313	441 ZUIL	40.11	ZU GF IVI	Water Supply	Domestic	I NESI1	3.66 BROWN	SAND	BOULDERS
									48.8 BLACK	GRANITE	MEDIUM-GRAINED
									51.8 BLACK	GRANITE	SOFT
5114507	736709.2	4941270	237.5 10 ft	9.144	6 GPM	Water Supply	Domestic	FRESH	31.0 DE/1010	J	23. 1
311-007	. 007 00.2	70-1270	237.0 1010	J.1-4	0 O. W	Tatol Capply	Domosiio	TILLOTT			

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Well I.D.	Easting N	Northing El	lev.	WL Fou	ınd (m) To	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold	our Lith. Materials GRANITE	
5114579	737200.2	4941542	238.2	10 ft	27.74	10 GPM	Water Supply	Domestic	FRESH	1.22 BROWN	SAND	
										30.5 RED	GRANITE	
5114580	737003.2	4942114	242.7	20 ft	29.26	6 GPM	Water Supply	Domestic	FRESH	10.4 BROWN	SAND	CLAY
										10.4 BROWN	SAND	CLAY
										30.5 RED	GRANITE	OLA
										30.5 RED	GRANITE	
5114581	735676.1	4942337	240.4	20 ft	29.26	10 GPM	Water Supply	Domestic	FRESH			
										0.91 BROWN	SAND	
										30.5 RED	GRANITE	
5114592	737591.2	4940389	240.7	20 ft	23.16	10 GPM	Water Supply	Domestic	FRESH	0.05.00.01441	01.437	0701150
										3.05 BROWN	CLAY	STONES
										3.05 BROWN 25.3 RED	CLAY GRANITE	STONES
										25.3 RED	GRANITE	
5114737	730593.1	4936911	239.3	25 ft	14.63	12 GPM	Water Supply	Domestic	FRESH	25.5 NLD	OIVAINIL	
0							riato. Capp.y	20000		1.22 BROWN	SAND	FILL
										1.83 BLACK	TOPSOIL	BOULDERS
										19.2 GREY	GRANITE	
5114760	736567.2	4939997	233.4	ft	69.8	GPM	Water Supply	Domestic	FRESH			
										2.74 BROWN	OVERBURDEN	CLAY
										69.8 BROWN	ROCK	HARD
5114784	735495.6	4941848	2/1 /	10 ft	13.72	8 GPM	Water Supply	Domestic	FRESH	80.8 GREY	ROCK	
3114704	733493.0	4941040	241.4	1011	13.72	o GFIVI	water Supply	Domestic	FRESH	1.22 GREY	FILL	STONES
										6.1 BROWN	SAND	2.22
										13.7 GREY	GRANITE	
										14.6 RED	GRANITE	
										21.3 GREY	GRANITE	
										25 WHITE	GRANITE	
5114891	732533.1	4937715	250.2	18 ft	9.144	4 GPM	Water Supply	Domestic	FRESH		0.1115	00.005
										3.05 BROWN	SAND	GRAVEL
										3.05 BROWN 8.53 RED	SAND	GRAVEL
										8.53 RED	GRANITE GRANITE	
										18.3 GREY	GRANITE	
										18.3 GREY	GRANITE	
5114895	735495.6	4941848	241.4	10 ft	35.05	2 GPM	Water Supply	Domestic	FRESH			
										0.61 BROWN	SAND	

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Well I.D.	Easting N	Northing Elev	/. WI	L Found (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Colo		
									35.1 GREY	GRANITE	
									35.4 GREEN	GRANITE	
									43.3 RED	GRANITE	
5114981	730036.1	4941430 2	60.6 1	0 ft 13.72	5 GPM	Water Supply	Domestic	FRESH	0.30 BROWN	TOPSOIL	SOFT
									0.30 BROWN	TOPSOIL	SOFT
									5.49 GREY	SHALE	SAND
									5.49 GREY	SHALE	SAND
									11 RED	CLAY	SANDY
									11 RED	CLAY	SANDY
									14.9 RED	LIMESTONE	HARD
									14.9 RED	LIMESTONE	HARD
5115084	732325.1	4938177 2	<b>51 Q</b>	ft 4.267	1 GPM	Water Supply	Domestic	FRESH	14.9 KED	LIMESTONE	HAKD
5115064	732323.1	4930177 2	31.0	11 4.207	I GFIVI	water Supply	Domestic	FRESH	0.30 BROWN	TOPSOIL	
									3.66 BROWN	CLAY	STONES
									4.27	SHALE	0.01120
									14.3	GRANITE	ROCK
5115150	736019.2	4942917 2	44.8 2	24 ft 22.86	20 GPM	Water Supply	Domestic	FRESH	11.0	01011112	Noon
00.00		.0.20				rate. Capp.)	20000		7.32 RED	SAND	GRAVEL
									7.32 RED	SAND	GRAVEL
									24.4 GREY	GRANITE	
									24.4 GREY	GRANITE	
5115159	735495.6	4941848 2	41.4	0 ft 6.096	5 GPM	Test Hole	Domestic	FRESH			
									0.91 BROWN	SAND	
									9.14 BLACK	GRANITE	
5115160	735495.6	4941848 2	41.4	8 ft 16.76	3 GPM	Water Supply	Domestic	FRESH			
									0.91 BROWN	SAND	
									20.4 BLACK	GRANITE	
5115240	734685.1	4941451 2	41.2 2	20 ft 57.91	8 GPM	Water Supply	Domestic	FRESH			
									1.22 BLACK	TOPSOIL	
									6.1 WHITE	GRANITE	
									30.5 WHITE	GRANITE	
									54.9 WHITE	GRANITE	
									61 WHITE	GRANITE	
5115306	735110.1	4939499 2	40.4 2	20 ft 24.38	6 GPM	Water Supply	Domestic	FRESH	0.04.01.4.01/	TODOOU	
									0.61 BLACK	TOPSOIL	
									6.1 BLACK	GRANITE	
									24.4 BLACK	GRANITE	
E44E440	726700 0	4044070 0	27.5	6# 40.00	7.0014	Motor Commit	Dominatio	EDECL	42.7 BLACK	GRANITE	
5115449	736709.2	4941270 2	31.5	6 ft 10.06	7 GPM	Water Supply	Domestic	FRESH	0.61 BROWN	SAND	
									O.OT DICOVIN	OLIND	

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Well I.D.	Easting N	Northing E	lev. WL	Found (m) T	est Rate S	Status	Use	Water Kind	Lith. Depth Lith. Colo 3.66 GREY 19.2 RED	ur Lith. Materials CLAY GRANITE	
5115450	735495.6	4941848	241.4								
5115590	730432.6	4940313	241 40	) ft 35.05	7 GPM	Water Supply	Domestic	FRESH			
3113330	730432.0	4340313	241 40	711 33.03	7 GI W	Water Supply	Domestic	TICLOTT	1.52 BROWN	SAND	
									1.52 BROWN	SAND	
									6.1 WHITE	GRANITE	
									6.1 WHITE	GRANITE	
									24.4 BLACK	GRANITE	SAND
									24.4 BLACK	GRANITE	SAND
									37.2 BLACK	GRANITE	
									37.2 BLACK	GRANITE	
5115829	730432.6	4940313	241 15	5 ft 12.19	10 GPM	Water Supply	Domestic	FRESH			
									1.52 BROWN	SAND	BOULDERS
									1.52 BROWN	SAND	BOULDERS
									4.88 BROWN	ROCK	WEATHERED
									4.88 BROWN	ROCK	WEATHERED
									12.2	GRANITE	
									12.2	GRANITE	
									22.9 BLACK	GRANITE	
									22.9 BLACK	GRANITE	
									38.1 RED	GRANITE	
E44E022	706500 7	4042044	047.0.07	7.4 0.111	4 CDM	Motor Cumply	Domostia	FRESH	38.1 RED	GRANITE	
5115832	730029.7	4943841	241.3 21	7 ft 9.144	4 GPM	Water Supply	Domestic	FRESH	0.30 BROWN	TOPSOIL	
										TOPSOIL	
										TOPSOIL	
										TOPSOIL	
									0.30 BROWN		
										TOPSOIL	
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									0.91 BROWN	TOPSOIL	SANDY
									0.91 BROWN	TOPSOIL	SANDY
									0.91 BROWN	TOPSOIL	SANDY
									0.91 BROWN	TOPSOIL	SANDY
									0.91 BROWN	TOPSOIL	SANDY
									0.91 BROWN	TOPSOIL	SANDY

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Well I.D.	Easting	Northing	Elev.	WL Foun	d (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold	our Lith. Materials	
	•				` ,					0.91 BROWN		SANDY
										0.91 BROWN	TOPSOIL	SANDY
										0.91 BROWN	TOPSOIL	SANDY
										5.49 BLACK	GRANITE	ROCK
										5.49 BLACK	GRANITE	ROCK
										5.49 BLACK	GRANITE	ROCK
										5.49 BLACK	GRANITE	ROCK
										5.49 BLACK	GRANITE	ROCK
										5.49 BLACK	GRANITE	ROCK
										5.49 BLACK	GRANITE	ROCK
										5.49 BLACK	GRANITE	ROCK
										5.49 BLACK	GRANITE	ROCK
										13.4 BLACK	GRANITE	ROCK
										13.4 BLACK	GRANITE	ROCK
										13.4 BLACK	GRANITE	ROCK
										13.4 BLACK	GRANITE	ROCK
										13.4 BLACK	GRANITE	ROCK
										13.4 BLACK	GRANITE	ROCK
										13.4 BLACK	GRANITE	ROCK
										13.4 BLACK	GRANITE	ROCK
										13.4 BLACK	GRANITE	ROCK
										21.3 BLACK	GRANITE	ROCK
										21.3 BLACK	GRANITE	ROCK
										21.3 BLACK	GRANITE	ROCK
										21.3 BLACK	GRANITE	ROCK
										21.3 BLACK	GRANITE	ROCK
										21.3 BLACK	GRANITE	ROCK
										21.3 BLACK	GRANITE	ROCK
										21.3 BLACK	GRANITE	ROCK
										21.3 BLACK	GRANITE	ROCK
										21.9 BLACK	GRANITE	
										21.9 BLACK	GRANITE	
										21.9 BLACK	GRANITE	
										21.9 BLACK	GRANITE	
										21.9 BLACK	GRANITE	
										21.9 BLACK	GRANITE	
										21.9 BLACK	GRANITE	
										21.9 BLACK 21.9 BLACK	GRANITE GRANITE	
E11E0E1	721510 4	4026502	2 240	4 22 <del>f</del>	10.07	CDM	Mater Supply	Domostio	EDECLI	21.9 DLACK	GRANITE	
5115851	731548.1	4936593	248	.4 ZZ II	10.97	GPM	Water Supply	Domestic	FRESH			

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Well I.D.	Easting N	Northing E	lev. \	WL Four	nd (m) 1	Test Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold	our Lith. Materials OVERBURDEN	
										3.96 BROWN	OVERBURDEN	
										8.23 GREY	ROCK	LIMESTONE
										8.23 GREY	ROCK	LIMESTONE
										13.7 RED	ROCK	SHALE
										13.7 RED	ROCK	SHALE
										15.2 GREEN	ROCK	OTALL
										15.2 GREEN	ROCK	
5115852	733858.1	4938270	271	9 ft	9.144	10 GPM	Water Supply	Domestic	FRESH	10.2 01(221)	r.cor.	
00002		.0002.0		0.1	0		rate. Cupp.y	20000		3.96 RED	SAND	CLAY
										3.96 RED	SAND	CLAY
										9.14 RED	GRANITE	ROCK
										9.14 RED	GRANITE	ROCK
										10.1 RED	GRANITE	QUARTZ
										10.1 RED	GRANITE	QUARTZ
										12.2 RED	GRANITE	ROCK
										12.2 RED	GRANITE	ROCK
5115882	730844.1	4938257	241.9	18 ft	7.62	6 GPM	Water Supply	Domestic	FRESH			
										1.22 BROWN	CLAY	SANDY
										3.66 BLUE	GRANITE	FRACTURED
										15.2 BLACK	GRANITE	LAYERED
5115936	737003.2	4942114	242.7	20 ft	22.56	4 GPM	Water Supply	Domestic	FRESH			
										9.75 YELLOW	SAND	LOOSE
										24.4 GREEN	GRANITE	
5115968	736567.2	4939997	233.4	7 ft	3.048	100 GPM	Water Supply	Domestic	Not stated		T0000#	
										0.30 BROWN	TOPSOIL	LOOSE
										0.30 BROWN	TOPSOIL	LOOSE
										3.05 BROWN	SAND	DENSE
										3.05 BROWN	SAND	DENSE
										3.05 RED	ROCK	DENSE
5440044	700070.4	40.40007	055.0	00.0	04.44	40 ODM	\\\-\ \ - \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	Democific	EDEOLI	3.05 RED	ROCK	DENSE
5116011	730273.1	4940867	255.9	22 ft	34.44	10 GPM	Water Supply	Domestic	FRESH	0.91 BROWN	SAND	STONES
										36.6 WHITE	GRANITE	ROCK
5116095	737515.2	4945412	262.1	8 ft	7.925	5 GPM	Water Supply	Industrial	FRESH	30.0 WHITE	GRANITE	ROCK
5116085	131313.2	4343412	202. I	οπ	1.323	3 GFIVI	Water Supply	Industrial	FRESH	2.44 BROWN	SAND	GRAVEL
										2.44 BROWN	SAND	GRAVEL
										2.44 BROWN	SAND	GRAVEL
										2.44 BROWN	SAND	GRAVEL
										36.6 WHITE	DOLOMITE	0.0.0.22
										33.0 WITH L		

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Well I.D.	Easting	Northing El	ev. V	VL Four	nd (m) Te	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Colo	ur Lith. Materials DOLOMITE	
										36.6 WHITE	DOLOMITE	
										36.6 WHITE	DOLOMITE	
5116124	735495.6	4941848	241.4	8 ft	27.43	3 GPM	Water Supply	Domestic	FRESH			
										1.22 BROWN	TOPSOIL	
										25.9 BLACK	GRANITE	
										28.0 RED	GRANITE	
5116190	735110.1	4939499	240.4	4 ft	5.486	15 GPM	Water Supply	Domestic	FRESH			
										0.61 BROWN	SAND	TOPSOIL
										2.13 RED	SAND	GRAVEL
										6.71 RED	GRANITE	ROCK
										12.2 BLACK	GRANITE	ROCK
5116279	736709.2	4941270	237.5	3 ft	14.63	8 GPM			FRESH			
										0.30	TOPSOIL	
										15.2 GREY	GRANITE	
5116315	733695.1	4938777	241.1	15 ft	15.85	4 GPM	Water Supply	Domestic	FRESH			
										0.61 BROWN	SAND	STONES
										0.61 BROWN	SAND	STONES
										2.44 GREY	GRANITE	HARD
										2.44 GREY	GRANITE	HARD
										3.05 RED	GRANITE	
										3.05 RED	GRANITE	
										15.8 GREY	GRANITE	HARD
										15.8 GREY	GRANITE	HARD
										16.5 GREY	GRANITE	QUARTZ
										16.5 GREY	GRANITE	QUARTZ
										18.3 GREY	GRANITE	HARD
										18.3 GREY	GRANITE	HARD
5116355	733695.1	4938777	241.1	14 ft	11.58	2 GPM	Abandoned-Qualit	y Not Used	FRESH			
										3.35 BROWN	SAND	CLAY
										3.35 BROWN	SAND	CLAY
										11.6 RED	GRANITE	HARD
										11.6 RED	GRANITE	HARD
										13.1 BLACK	GRANITE	HARD
										13.1 BLACK	GRANITE	HARD
5116386	733695.1	4938777	241.1	10 ft	24.38	2 GPM	Water Supply	Domestic	FRESH	0.445501	0.4415	01.437
										2.44 BROWN	SAND	CLAY
										2.44 BROWN	SAND	CLAY
										6.1 RED	GRANITE	HARD
										6.1 RED	GRANITE	HARD

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Well I.D.	Easting	Northing E	lev.	WL Four	nd (m) Te	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold		
										24.4 GREY	GRANITE	HARD
										24.4 GREY	GRANITE	HARD
5116388	731108.1	4937737	242.3	18 ft	12.5	8 GPM	Water Supply	Domestic	FRESH	0.04.0000000	CAND	01.47/
										0.91 BROWN 0.91 BROWN	SAND SAND	CLAY CLAY
												QUARTZ
										3.05 RED 3.05 RED	GRANITE GRANITE	QUARTZ
										4.57 GREY	GRANITE	HARD
										4.57 GREY	GRANITE	HARD
										10.7 RED	GRANITE	QUARTZ
										10.7 RED	GRANITE	QUARTZ
										13.7 GREEN	GRANITE	HARD
										13.7 GREEN	GRANITE	HARD
5116417	736979.2	4938802	246 5	10 ft	28.96	5 GPM	Water Supply	Domestic	FRESH	15.7 OKLEN	OIVAINIL	HARD
0110417	700070.2	4000002	240.0	1011	20.00	O OI W	water Supply	Domestio	TREON	1.83 BROWN	SAND	
										31.4 GREY	LIMESTONE	
5116516	736567.2	4939997	233.4	20 ft	7.62	1 GPM	Water Supply	Domestic	FRESH			
							,			0.91 BLACK	TOPSOIL	
										122 GREY	GRANITE	
5116654	733695.1	4938777	241.1	16 ft	19.51	3 GPM	Water Supply	Domestic	FRESH			
										1.83 BROWN	SAND	CLAY
										1.83 BROWN	SAND	CLAY
										3.35 RED	GRANITE	HARD
										3.35 RED	GRANITE	HARD
										12.8 GREY	GRANITE	HARD
										12.8 GREY	GRANITE	HARD
										14.6 GREEN	GRANITE	LOOSE
										14.6 GREEN	GRANITE	LOOSE
										21.0 GREY	GRANITE	HARD
										21.0 GREY	GRANITE	HARD
5116663	733543.6	4941643	253.8	20 ft	90.53	10 GPM	Water Supply	Domestic	FRESH	0.04.0000000	CAND	
										0.91 BROWN	SAND	
										0.91 BROWN	SAND	
										25.9 GREY	GRANITE	
										25.9 GREY	GRANITE	
										27.4 RED	GRANITE	
										27.4 RED	GRANITE	
										79.2 GREY	GRANITE	
										79.2 GREY 80.8 RED	GRANITE GRANITE	
										00.0 KED	GRANITE	

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Well I.D.	Easting N	lorthing Elev	. WL Fo	ound (m) T	est Rate S	Status	Use	Water Kind	Lith. Depth Lith. Cold		
									80.8 RED	GRANITE	
									92.4 GREY	GRANITE	
									92.4 GREY	GRANITE	
5116801	735491.1	4938243 23	38.1 10 ft	7.62	2 GPM	Water Supply	Domestic	FRESH	0.30 BROWN	TOPSOIL	
									0.30 BROWN		
									0.30 BROWN	TOPSOIL	
									0.30 BROWN		
									0.30 BROWN		
									0.30 BROWN		
									2.44 BROWN	ROCK	LAYERED
									2.44 BROWN	ROCK	LAYERED
									2.44 BROWN		LAYERED
									2.44 BROWN		LAYERED
									2.44 BROWN		LAYERED
									2.44 BROWN		LAYERED
									5.49 BROWN		
									5.49 BROWN	LIMESTONE	
									5.49 BROWN	LIMESTONE	
									5.49 BROWN	LIMESTONE	
									5.49 BROWN	LIMESTONE	
									5.49 BROWN	LIMESTONE	
									6.71 BROWN	ROCK	
									6.71 BROWN	ROCK	
									6.71 BROWN	ROCK	
									6.71 BROWN	ROCK	
									6.71 BROWN	ROCK	
									6.71 BROWN	ROCK	
									33.5 WHITE	ROCK	
									33.5 WHITE	ROCK	
									33.5 WHITE	ROCK	
									33.5 WHITE	ROCK	
									33.5 WHITE	ROCK	
									33.5 WHITE	ROCK	
5116804	735495.6	4941848 2	41.4 5 ft	10.67	3 GPM	Water Supply	Domestic	FRESH			
									1.22 BROWN	SAND	BOULDERS
									1.22 BROWN	SAND	BOULDERS
									3.35 RED	GRANITE	HARD
									3.35 RED	GRANITE	HARD
									13.4 GREY	GRANITE	HARD

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Well I.D.	Easting N	Northing E	lev.	WL Fou	nd (m) T	est Rate	Status	Use	Water Kind		olour Lith. Materials	
										13.4 GREY	GRANITE	HARD
5116805	732325.1	4938177	251.8	20 ft	12.19	0 GPM	Water Supply	Domestic	FRESH	0.04.050	04115	0701150
										0.91 RED	SAND	STONES
										0.91 RED	SAND	STONES
										5.49 RED	SANDSTONE	BOULDERS
										5.49 RED	SANDSTONE	BOULDERS
										45.1 GREY	GRANITE	HARD
										45.1 GREY	GRANITE	HARD
5116816	736709.2	4941270	237.5	3 ft	5.486	5 GPM	Water Supply	Domestic	FRESH			
										0.30 BLACK	TOPSOIL	
										0.30 BLACK		
										0.30 BLACK		
										0.30 BLACK		
										4.57 RED	GRANITE	
										4.57 RED	GRANITE	
										4.57 RED	GRANITE	
										4.57 RED	GRANITE	
										7.62 BLACK	GRANITE	
										7.62 BLACK	GRANITE	
										7.62 BLACK	GRANITE	
										7.62 BLACK	GRANITE	
										12.8 RED	GRANITE	
										12.8 RED	GRANITE	
										12.8 RED	GRANITE	
										12.8 RED	GRANITE	
5117119	735491.1	4938243	238.1	15 ft	30.48	3 GPM	Water Supply	Domestic	FRESH			
										0.30 BROWI	N SAND	
										0.30 BROWI	N SAND	
										9.14 RED	LIMESTONE	
										9.14 RED	LIMESTONE	
										49.7 GREY	LIMESTONE	GRANITE
										49.7 GREY	LIMESTONE	GRANITE
5117147	735495.6	4941848	241.4	12 ft	21.34	2 GPM	Water Supply	Domestic	FRESH			
· · · · · · ·		.5510				_ 0		20000		5.49 RED	SAND	SOFT
										5.49 RED	SAND	SOFT
										5.49 RED	SAND	SOFT
										5.49 RED	SAND	SOFT
										10.7 RED	SANDSTONE	HARD
										10.7 RED	SANDSTONE	HARD
										10.7 RED	SANDSTONE	HARD
										10.7 11.20	3/ IIIDO I OINE	IIAND

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Well I.D.	Easting N	Northing El	lev.	WL Four	nd (m) Te	st Rate	Status	Use	Water Kind	Lith. Depth	Lith. Colo	ur Lith. Materials	
					` ,					10.7	7 RED	SANDSTONE	HARD
										13.1	1 RED	GRANITE	HARD
										13.1	1 RED	GRANITE	HARD
										13.	1 RED	GRANITE	HARD
										13.	1 RED	GRANITE	HARD
										15.	5 GREY	GRANITE	POROUS
										15.	5 GREY	GRANITE	POROUS
										15.	5 GREY	GRANITE	POROUS
										15.	5 GREY	GRANITE	POROUS
										21.3	3 BLACK	GRANITE	HARD
										21.3	3 BLACK	GRANITE	HARD
										21.3	3 BLACK	GRANITE	HARD
											3 BLACK	GRANITE	HARD
										22.3	3 GREY	GRANITE	POROUS
											3 GREY	GRANITE	POROUS
										22.3	3 GREY	GRANITE	POROUS
										22.3	3 GREY	GRANITE	POROUS
											4 BLACK	GRANITE	HARD
											4 BLACK	GRANITE	HARD
										24.4	4 BLACK	GRANITE	HARD
										24.4	4 BLACK	GRANITE	HARD
5117378	732097.1	4940921	255.9	18 ft	5.486	2 GPM	Water Supply	Domestic	FRESH				
											4 BROWN	SAND	FILL
											4 BROWN	SAND	FILL
											2 BLACK	GRANITE	ROCK
				_						40.2	2 BLACK	GRANITE	ROCK
5117379	730432.6	4940313	241	ft	2.438	3 GPM	Water Supply	Domestic	FRESH	0.00		CAND	
											BROWN	SAND	
											BROWN	SAND	
											2 GREY 2 GREY	GRANITE	
											2 GRET 3 WHITE	GRANITE	
												GRANITE	
											3 WHITE	GRANITE	
											2 RED 2 RED	GRANITE	
											2 KED 3 WHITE	GRANITE	
											3 WHITE 3 WHITE	GRANITE GRANITE	
5117201	730288.8	4939733	2542	25.4	15.24	8 GPM	Water Supply	Domestic	FRESH	55.8	O VVIII E	GNAINITE	
5117381	130200.8	4303133	204.3	201l	13.24	0 GFIVI	vvater Suppry	Domestic	FRESH	0.30	BROWN	TOPSOIL	
											BROWN		
										0.50	DICOVIN	. 3. 33.2	

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Well I.D.	Easting N	Northing E	lev. \	WL Fou	nd (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold		CANDY
										3.66 BROWN	CLAY	SANDY
										3.66 BROWN	CLAY	SANDY
										20.4 RED	GRANITE	
5447000	705405.0	40.440.40	044.4	45.0		ODM	M-1 0	D		20.4 RED	GRANITE	
5117663	735495.6	4941848	241.4	15 π		GPM	Water Supply	Domestic		1.52 BROWN	TOPSOIL	
										62.5 RED	GRANITE	
5117669	732097.1	4940921	255 0	20 ft	33.53	14 GPM	Water Supply	Domestic	FRESH	02.5 NED	ORANIE	
0117000	702007.1	4040021	200.0	2011	00.00	14 OI W	Water Cappiy	Domestio	TREOTT	1.22 BROWN	TOPSOIL	SANDY
										35.7 BLACK	GRANITE	ROCK
5117670	730273.1	4940867	255.9	23 ft	18.9	5 GPM	Water Supply	Domestic	FRESH			
							117			21.9 RED	GRANITE	
5117671	730432.6	4940313	241	8 ft	16.76	25 GPM	Water Supply	Domestic	FRESH			
										1.52 BROWN	TOPSOIL	SANDY
										1.52 BROWN	TOPSOIL	SANDY
										17.7 RED	GRANITE	ROCK
										17.7 RED	GRANITE	ROCK
5117722	732513.1	4946191	277.4	12 ft	11.58	GPM	Water Supply	Domestic	FRESH			
										0.91 BROWN	SAND	STONES
										0.91 BROWN	SAND	STONES
										9.14 GREY	GRANITE	QUARTZ
										9.14 GREY	GRANITE	QUARTZ
										11.6 RED	GRANITE	SOFT
										11.6 RED	GRANITE	SOFT
										21.3 WHITE	GRANITE	HARD
										21.3 WHITE	GRANITE	HARD
										32.0 RED	GRANITE	QUARTZ
										32.0 RED	GRANITE	QUARTZ
										45.7 GREY	GRANITE	HARD
										45.7 GREY	GRANITE	HARD
										54.3 WHITE	GRANITE	HARD
										54.3 WHITE	GRANITE	HARD
										58.2 RED	GRANITE	QUARTZ
										58.2 RED	GRANITE	QUARTZ
										65.5 WHITE	GRANITE	HARD
										65.5 WHITE	GRANITE	HARD
										73.2 GREY	GRANITE	HARD
E447000	700007 4	40.4000.1	055.0	444		0.0014	Matau O	Danisatia	EDEOL	73.2 GREY	GRANITE	HARD
5117820	732097.1	4940921	255.9	14 ft		3 GPM	Water Supply	Domestic	FRESH	1.22 BROWN	TOPSOIL	

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Well I.D.	Easting N	lorthing E	Elev.	WL Found	d (m) Tes	t Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold		
										1.22 BROWN		
										122 WHITE	GRANITE	
										122 WHITE	GRANITE	
5117836	736608.2	4940937	235.5	10 ft	17.37	3 GPM	Water Supply	Domestic	FRESH			
										1.22 BROWN	SAND	FILL
										1.22 BROWN	SAND	FILL
										18.6 GREY	GRANITE	HARD
										18.6 GREY	GRANITE	HARD
5118008	736709.2	4941270	237.5	2 ft	4.572	1 GPM	Water Supply	Domestic	FRESH	0.04.0000441	TOROGU	
											TOPSOIL	
										0.91 BROWN		
										0.91 BROWN		
										0.91 BROWN		
										1.52 BROWN		
										1.52 BROWN		
										1.52 BROWN	SAND	
										1.52 BROWN		
										15.2 RED	GRANITE	HARD
										15.2 RED	GRANITE	HARD
										15.2 RED	GRANITE	HARD
										15.2 RED	GRANITE	HARD
										18.3 BLACK	GRANITE	HARD
										18.3 BLACK	GRANITE	HARD
										18.3 BLACK	GRANITE	HARD
										18.3 BLACK	GRANITE	HARD
										23.8 RED	GRANITE	HARD
										23.8 RED	GRANITE	HARD
										23.8 RED	GRANITE	HARD
										23.8 RED	GRANITE	HARD
5118097	736564.7	4939997	233.4	19 ft	21.95	10 GPM	Water Supply	Domestic	FRESH			
										0.61 RED	SAND	SOFT
										0.61 RED	SAND	SOFT
										4.27 GREY	GRANITE	HARD
										4.27 GREY	GRANITE	HARD
										8.53 GREEN	GRANITE	HARD
										8.53 GREEN	GRANITE	HARD
										18.3 RED	GRANITE	HARD
										18.3 RED	GRANITE	HARD
										20.7 GREY	GRANITE	HARD
										20.7 GREY	GRANITE	HARD

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Well I.D.	Easting N	Northing Ele	v. WL Fo	und (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Colo	ur Lith. Materials	
									21.9 RED	GRANITE	POROUS
									21.9 RED	GRANITE	POROUS
									24.4 GREY	GRANITE	POROUS
									24.4 GREY	GRANITE	POROUS
5118320	731933.1	4941460 2	254.0 45 ft	35.05	8 GPM	Water Supply	Domestic	FRESH			
									6.1 BROWN	SAND	GRAVEL
									6.1 BROWN	SAND	GRAVEL
									10.7 GREY	GRANITE	HARD
									10.7 GREY	GRANITE	HARD
									27.4 WHITE	GRANITE	QUARTZ
									27.4 WHITE	GRANITE	QUARTZ
									30.5 GREY	GRANITE	HARD
									30.5 GREY	GRANITE	HARD
									32.6 WHITE	GRANITE	QUARTZ
									32.6 WHITE	GRANITE	QUARTZ
									35.1 RED	GRANITE	POROUS
									35.1 RED	GRANITE	POROUS
									35.7 GREY	GRANITE	HARD
									35.7 GREY	GRANITE	HARD
5118363	730555.6	4939266	239 21 ft	14.63	4 GPM	Water Supply	Domestic	FRESH			
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									3.35 WHITE	SHALE	GRANITE
									3.35 WHITE	SHALE	GRANITE
									14.6 WHITE	GRANITE	
									14.6 WHITE	GRANITE	
									24.4 RED	GRANITE	
									24.4 RED	GRANITE	
5118438	730837.6	4938241 2	242.4 17 ft	32.61	15 GPM	Water Supply	Domestic	FRESH			
									2.44	GRAVEL	
									2.44	GRAVEL	
									36.6 GREY	GRANITE	
									36.6 GREY	GRANITE	
5118449	738177.8	4946418 2	283.4 5 ft	58.22	3 GPM	Water Supply	Domestic	FRESH	0.00.000	TODOOU	
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	2445
									7.62 GREY	SILT	SAND
									7.62 GREY	SILT	SAND
									61 GREY	GRANITE	
									61 GREY	GRANITE	

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<b>Well I.D.</b> 5118589	<b>Easting N</b> 730834.6	•			` '	est Rate \$	Status Water Supply	<b>Use</b> Domestic	Water Kind FRESH	Lith. Depth Lith. Colo	our Lith. Materials	
							117			0.30 BROWN	SAND	SOFT
										0.30 BROWN	SAND	SOFT
										9.14 BLACK	GRANITE	QUARTZ
										9.14 BLACK	GRANITE	QUARTZ
										10.4 RED	GRANITE	HARD
										10.4 RED	GRANITE	HARD
										24.7 BLACK	GRANITE	HARD
										24.7 BLACK	GRANITE	HARD
										25.3 RED	GRANITE	POROUS
										25.3 RED	GRANITE	POROUS
										26.5 BLACK	GRANITE	HARD
										26.5 BLACK	GRANITE	HARD
5118590	732318.6	4938175	251.2	10 ft	13.72	1 GPM	Water Supply	Domestic	FRESH			
										3.05 RED	SAND	SOFT
										3.05 RED	SAND	SOFT
										10.7 BLACK	GRANITE	HARD
										10.7 BLACK	GRANITE	HARD
										22.3 GREY	GRANITE	
										22.3 GREY	GRANITE	
5118643	735673	4941879	238.9	11 ft	21.95	8 GPM	Water Supply	Domestic	FRESH			
										3.96 RED	SAND	SOFT
										3.96 RED	SAND	SOFT
										6.1 RED	GRANITE	HARD
										6.1 RED	GRANITE	HARD
										19.8 BLACK	GRANITE	HARD
										19.8 BLACK	GRANITE	HARD
										21.9 GREY	GRANITE	POROUS
										21.9 GREY	GRANITE	POROUS
										24.4 BLACK	GRANITE	HARD
										24.4 BLACK	GRANITE	HARD
5118660	732747.6	4937151	275.2	18 ft	12.19	10 GPM	Water Supply	Domestic	FRESH		T0700"	
										0.30 BROWN	TOPSOIL	
										3.96 BROWN	CLAY	STONES
										6.40 GREY	CLAY	STONES
										12.2 GREY	LIMESTONE	ROCK
5118712	735113.1	4939499	240.3	19 ft	107	4 GPM	Water Supply	Domestic	FRESH	0.40 DDOWAL	CAND	
										2.13 BROWN	SAND	
										2.13 BROWN	SAND	
										110 GREEN	GKAVEL	

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Well I.D.	Easting	Northing El	ev. WL Fou	und (m) Test Rate	Status (	Jse	Water Kind	Lith. Depth Lith. Col	our Lith. Materials GRAVEL	
5118736	732510.1	4946192	277.4	1.219	Water Supply	Not Used	FRESH			
					117			0.30 BROWN	TOPSOIL	
								0.91 GREY	SILT	CLAY
								2.44 GREY	LIMESTONE	SOFT
5118737	732510.1	4946192	277.4	1.219	Observation Wells	Not Used	FRESH			
								0.30 BROWN	TOPSOIL	
								0.91 GREY	SILT	CLAY
								2.44 GREY	LIMESTONE	SOFT
5118738	732510.1	4946192	277.4	1.219	Water Supply	Not Used	FRESH			
								0.30 BROWN	UNKNOWN TYPE	
								0.91 GREY	SILT	CLAY
								2.44 GREY	LIMESTONE	SOFT
5118739	732510.1	4946192	277.4	1.219	Observation Wells	Not Used	FRESH			
								0.30 BROWN	TOPSOIL	LOOSE
								0.91 GREY	SILT	CLAY
								2.44 GREY	LIMESTONE	SOFT
5118740	732510.1	4946192	277.4	1.219	Observation Wells	Not Used	FRESH			
								0.30 BROWN	TOPSOIL	LOOSE
								0.91 GREY	SILT	CLAY
								2.44 GREY	LIMESTONE	SOFT
5118741	732510.1	4946192	277.4	1.219	Observation Wells	Not Used	FRESH			
								0.30 BROWN	TOPSOIL	LOOSE
								0.91 BROWN	SILT	CLAY
								2.44 GREY	LIMESTONE	SOFT
5118742	732510.1	4946192	277.4	2.134	Observation Wells	Not Used	FRESH			
								0.30 BROWN	TOPSOIL	
								0.91 GREY	SILT	CLAY
								3.66 GREY	LIMESTONE	FRACTURED
5118743	732510.1	4946192	277.4	2.438	Observation Wells	Not Used	FRESH			
								0.30 BROWN	TOPSOIL	
								0.30 GREY	SILT	CLAY
								5.49 GREY	LIMESTONE	FRACTURED
5118744	732510.1	4946192	277.4	2.134	Observation Wells	Not Used	FRESH			
								0.30 BROWN	TOPSOIL	
								1.22 GREY	SILT	CLAY
								5.18 GREY	LIMESTONE	FRACTURED
5118745	732510.1	4946192	277.4	1.829	Observation Wells		FRESH	0.00.0000	TOROGU	
								0.30 BROWN	TOPSOIL	DENIOE
								0.61 GREY	SILT	DENSE
								3.96 GREY	LIMESTONE	FRACTURED

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<b>Well I.D.</b> 5118746	<b>Easting N</b> 732510.1	Northing E 4946192		<b>NL</b> Four	nd (m) To 2.134	est Rate	Status Observation Wel	<b>Use</b> Ils	Water Kind FRESH	Lith. Depth	Lith. Colo	ur Lith. Materials		
										0.30	BROWN	TOPSOIL		
										0.61	GREY	UNKNOWN TYPE		
										5.18	GREY	LIMESTONE	FRAC	TURED
5118827	740146.2	4945103	262.2	11 ft	7.010	5 GPM	Water Supply	Domestic	Not stated					
										0.30	BROWN	TOPSOIL		
										0.30	BROWN	TOPSOIL		
											GREY	GRANITE		
										30.5	GREY	GRANITE		
5118867	730835.1	4938242	242.3	15 ft	18.9	7 GPM	Water Supply	Domestic	FRESH					
											BROWN	SAND	SOFT	
											BROWN	SAND	SOFT	
											GREY	GRANITE	HARD	
											GREY	GRANITE	HARD	
											RED	GRANITE	HARD	
										1.83	RED	GRANITE	HARD	)
										17.7	BLACK	GRANITE	HARD	
											BLACK	GRANITE	HARD	)
										18.9	RED	GRANITE	PORC	DUS
										18.9	RED	GRANITE	PORC	DUS
										19.2	BLACK	GRANITE	HARD	)
										19.2	BLACK	GRANITE	HARD	)
5118905	736784.1	4939384	243	5 ft	10.67	10 GPM	Water Supply	Domestic	FRESH					
										4.88	BROWN	FINE SAND		
										4.88	BROWN	FINE SAND		
										4.88	BROWN	FINE SAND		
										4.88	BROWN	FINE SAND		
										5.49	BROWN	FINE GRAVEL		
										5.49	BROWN	FINE GRAVEL		
										5.49	BROWN	FINE GRAVEL		
										5.49	BROWN	FINE GRAVEL		
										10.4	GREY	GRANITE	SOFT	
										10.4	GREY	GRANITE	SOFT	
										10.4	GREY	GRANITE	SOFT	
										10.4	GREY	GRANITE	SOFT	
										24.4	BLACK	GRANITE		
										24.4	BLACK	GRANITE		
										24.4	BLACK	GRANITE		
											BLACK	GRANITE		
5118954	730702	4939831	239.9	14 ft	29.26	3 GPM	Water Supply	Domestic	FRESH					

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Well I.D.	Easting N	Northing Ele	v. WLF	ound (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold	our Lith. Materials	
									1.22 GREY	GRAVEL	SOFT
									1.22 GREY	GRAVEL	SOFT
									3.66 GREY	GRANITE	HARD
									3.66 GREY	GRANITE	HARD
									5.79 RED	GRANITE	HARD
									5.79 RED	GRANITE	HARD
									12.2 BLACK	GRANITE	HARD
									12.2 BLACK	GRANITE	HARD
									28.3 GREEN	GRANITE	HARD
									28.3 GREEN	GRANITE	HARD
									29.3 RED	GRANITE	POROUS
									29.3 RED	GRANITE	POROUS
									38.1 GREY	GRANITE	HARD
									38.1 GREY	GRANITE	HARD
5119008	737186	4939459 2	242.6 11 f	t 6.096	8 GPM	Water Supply	Domestic	FRESH			
									1.22 RED	GRANITE	
									6.1 GREY	GRANITE	
									12.2 BLACK	GRANITE	
5119018	733694.1	4938777 2	241.0 51	t 64.01	10 GPM	Water Supply	Domestic	FRESH	2 25 DDOWN	CLAY	ROCK
									3.35 BROWN 64.0 BLACK	GRANITE	ROCK
5119019	735292.1	4938830 2	20 0 24 f	t 18.29	10 GPM	Water Supply	Domestic	FRESH	04.0 BLACK	GRANITE	ROCK
3119019	733292.1	4930030 2	239.0 341	10.29	TO GEIM	water Supply	Domestic	FRESH	0.91	GRAVEL	
									0.91	GRAVEL	
									34.7 BLACK	GRANITE	
									34.7 BLACK	GRANITE	
5119085	736716	4939531 2	242.7 4 f	t 16.76	10 GPM	Water Supply	Domestic	FRESH			
						11.7			0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									1.22 BROWN	CLAY	SAND
									1.22 BROWN	CLAY	SAND
									1.22 BROWN	CLAY	SAND
									1.22 BROWN	CLAY	SAND
									1.52 BROWN	SHALE	STONES
									1.52 BROWN	SHALE	STONES
									1.52 BROWN	SHALE	STONES
									1.52 BROWN	SHALE	STONES
									4.57 BROWN	GRANITE	

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Well I.D.	Easting Northin	ıg Elev.	WL Found	l (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold	ur Lith. Materials	
									4.57 BROWN	GRANITE	
									4.57 BROWN	GRANITE	
									4.57 BROWN	GRANITE	
									5.79	GRANITE	
									5.79	GRANITE	
									5.79	GRANITE	
									5.79	GRANITE	
									17.7 BROWN	GRANITE	
									17.7 BROWN	GRANITE	
									17.7 BROWN	GRANITE	
									17.7 BROWN	GRANITE	
									18.3 GREY	GRANITE	
									18.3 GREY	GRANITE	
									18.3 GREY	GRANITE	
									18.3 GREY	GRANITE	
5119144	734429.1 4941	406 238.	0 3 ft	11.58	5 GPM	Water Supply	Domestic	FRESH			
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									10.7 BROWN	TOPSOIL	SANDY
									10.7 BROWN	TOPSOIL	SANDY
									10.7 BROWN	TOPSOIL	SANDY
									10.7 BROWN	TOPSOIL	SANDY
									12.5 BROWN	SAND	
									12.5 BROWN	SAND	
									12.5 BROWN	SAND	
									12.5 BROWN	SAND	
5119157	731288.1 4938	3535 241.	1 36 ft	29.26	10 GPM	Water Supply	Domestic	FRESH	0.04 Pt. 4.014	TOROGU	0057
									0.61 BLACK	TOPSOIL	SOFT
									0.61 BLACK	TOPSOIL	SOFT
									5.79 GREY	GRANITE	HARD
									5.79 GREY	GRANITE	HARD
									9.45 RED	GRANITE	HARD
									9.45 RED	GRANITE	HARD
									14.9 GREY	GRANITE	HARD
									14.9 GREY	GRANITE	HARD
									21.6 BROWN	GRANITE	HARD
									21.6 BROWN	GRANITE	HARD
									28.3 GREY	GRANITE	HARD

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Well I.D.	Easting I	Northing E	lev.	WL Four	nd (m) Te	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Colo	our Lith. Materials	
										28.3 GREY	GRANITE	HARD
										29.3	GRANITE	POROUS
										29.3	GRANITE	POROUS
										29.6 GREY	GRANITE	HARD
										29.6 GREY	GRANITE	HARD
5119158	731255.1	4938543	241.0	17 ft	16.15	2 GPM	Water Supply	Domestic	FRESH			
										0.30 BLACK	TOPSOIL	SOFT
										0.30 BLACK	TOPSOIL	SOFT
										3.66 BLACK	GRANITE	HARD
										3.66 BLACK	GRANITE	HARD
										4.27 RED	GRANITE	SOFT
										4.27 RED	GRANITE	SOFT
										16.2 GREY	GRANITE	HARD
										16.2 GREY	GRANITE	HARD
										32.9 BLACK	GRANITE	HARD
										32.9 BLACK	GRANITE	HARD
										35.4	GRANITE	HARD
										35.4	GRANITE	HARD
										35.7 BLACK	GRANITE	HARD
										35.7 BLACK	GRANITE	HARD
5119172	736650.1	4938957	242.2	9 ft	6.706	20 GPM	Water Supply	Domestic	FRESH			
										1.22 BROWN	TOPSOIL	SAND
										1.22 BROWN	TOPSOIL	SAND
										3.05 RED	SAND	SOFT
										3.05 RED	SAND	SOFT
										3.96 RED	SANDSTONE	SOFT
										3.96 RED	SANDSTONE	SOFT
										6.71 RED	GRANITE	HARD
										6.71 RED	GRANITE	HARD
										7.01 RED	SANDSTONE	POROUS
										7.01 RED	SANDSTONE	POROUS
										7.62 GREY	GRANITE	HARD
										7.62 GREY	GRANITE	HARD
5119267	736090	4940096	237.7	6 ft	17.37	16 GPM	Water Supply	Domestic	FRESH			
										0.61 BLACK	TOPSOIL	SOFT
										0.61 BLACK	TOPSOIL	SOFT
										5.18 RED	GRANITE	HARD
										5.18 RED	GRANITE	HARD
										8.84 GREY	GRANITE	HARD
										8.84 GREY	GRANITE	HARD

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Well I.D.	Easting	Northing E	lev. \	WL Fo	und (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold		
										16.8	GRANITE	HARD
										16.8	GRANITE	HARD
										17.4 RED	GRANITE	POROUS
										17.4 RED	GRANITE	POROUS
										19.2 GREY	GRANITE	HARD
										19.2 GREY	GRANITE	HARD
5119339	736512	4941873	237.2	0 ft	24.38	6 GPM	Water Supply	Domestic	FRESH			
										0.30 BROWN	TOPSOIL	
										7.32 BROWN	SAND	GRAVEL
										24.4 RED	GRANITE	ROCK
5119440	740125	4944603	262.0	16 ft	10.06	8 GPM	Water Supply	Domestic	FRESH	4.00.0051/	0041/51	2057
										1.83 GREY	GRAVEL	SOFT
										1.83 GREY	GRAVEL	SOFT
										4.57 RED	GRANITE	HARD
										4.57 RED	GRANITE	HARD
										9.14 BLACK	GRANITE	HARD
										9.14 BLACK	GRANITE	HARD
										10.1 RED	GRANITE	POROUS
										10.1 RED	GRANITE	POROUS
										11.9 BLACK	GRANITE	HARD
										11.9 BLACK	GRANITE	HARD
5119500	736784.1	4939384	243	21 ft	18.59	20 GPM	Water Supply	Domestic	FRESH	4.07.000.000	01.437	OTONEO
										4.27 BROWN	CLAY	STONES
										4.27 BROWN	CLAY	STONES
										4.27 BROWN	CLAY	STONES
										4.27 BROWN	CLAY	STONES
										8.84 RED	GRANITE	SHALE
										8.84 RED	GRANITE	SHALE
										8.84 RED	GRANITE	SHALE
										8.84 RED	GRANITE	SHALE
										22.9 RED	GRANITE	HARD
										22.9 RED	GRANITE	HARD
										22.9 RED	GRANITE	HARD
										22.9 RED	GRANITE	HARD
5119585	736561.1	4939998	233.4	16 ft	42.67	10 GPM	Water Supply	Domestic	FRESH	4.00.000	OAND	05 W.E.
										1.22 BROWN	SAND	GRAVEL
										1.22 BROWN	SAND	GRAVEL
										42.1 GREY	GRANITE	HARD
										42.1 GREY	GRANITE	HARD
										43.3 BROWN	UNKNOWN TYPE	GRANITE

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Well I.D.	Easting N	lorthing E	lev. \	VL Fou	nd (m) Test Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold	our Lith. Materials UNKNOWN TYPE	GRANITE
									48.8 GREY	GRANITE	
									48.8 GREY	GRANITE	
5119697	730288	4937539	237.3	3 m	11.6 22.7 LPM	Water Supply	Domestic	FRESH			
						,			0.30 BROWN	SAND	SOFT
									0.30 BROWN	SAND	SOFT
									5.2 RED	GRANITE	HARD
									5.2 RED	GRANITE	HARD
									10.9 GREY	GRANITE	HARD
									10.9 GREY	GRANITE	HARD
									11.6 RED	GRANITE	POROUS
									11.6 RED	GRANITE	POROUS
									17.3 BLACK	GRANITE	HARD
									17.3 BLACK	GRANITE	HARD
5119743	738364.1	4938044	267.8	18 ft	13.72 10 GPM	Water Supply	Domestic	FRESH			
									2.13 BROWN	TOPSOIL	
									4.57 BROWN	CLAY	SHALE
									13.7 GREY	LIMESTONE	ROCK
5119747	733694.1	4938777	241.0	7 ft	72.24 5 GPM	Water Supply	Domestic	Not stated	0.00	TOROGU	
									0.30	TOPSOIL	DOOK
									1.52 BROWN	CLAY	ROCK
									21.9 RED	GRANITE	ROCK
									36.6 BLACK	GRANITE	ROCK
									68.6 GREY	GRANITE	ROCK
5440004	704005	4000070	040.0	40	40 F 00 41 DM	W-1 O	Damadia	EDEOU	72.2 RED	GRANITE	ROCK
5119824	731225	4938276	243.2	10 m	19.5 36.4 LPM	Water Supply	Domestic	FRESH	0.30 BLACK	TOPSOIL	SOFT
									0.30 BLACK	TOPSOIL	SOFT
									2.74 BLACK	GRANITE	HARD
									2.74 BLACK	GRANITE	HARD
									6.40	GRANITE	HARD
									6.40	GRANITE	HARD
									19.5 BLACK	GRANITE	HARD
									19.5 BLACK	GRANITE	HARD
									21	GRANITE	QUARTZ
									21	GRANITE	QUARTZ
									26.2 BLACK	GRANITE	HARD
									26.2 BLACK	GRANITE	HARD
5119891	735493.1	4941849	241.6	3 m	16 112 LPM	Water Supply	Domestic	FRESH	20.2 22.1011		
2170007						وههها			0.30 BROWN	TOPSOIL	SOFT

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Well I.D.	Easting N	lorthing E	lev. V	VL Four	nd (m) Test Rate	Status	Use	Water Kind	Lith. Depth Lith. Colo		
									6.1 BROWN		SOFT
									14.8	GRAVEL	HARD
									23.2 RED	GRAVEL	HARD
5119952	736364	4941743	235.7	1 m	36 LPM	Water Supply	Domestic				
									2 BLACK	MUCK	WOOD FRAGMENTS
									4 GREY	SAND	SILT
									31 GREY	GRANITE	
5119953	736364	4941775	235.4	0.9 m	36 LPM	Water Supply	Domestic	FRESH			
									2.5 BLACK	MUCK	WOOD FRAGMENTS
									31 GREY	GRANITE	
5119964	730751	4940447	241.6	6 m	LPM	Abandoned-Othe	er				
5120020	730726	4938189	241.7	6 m	35 13.6 LPM	Water Supply	Domestic	FRESH	0.014.014	00444	
									6 BLACK	GRANITE	
									7 WHITE	QUARTZ	
									24 BLACK	GRANITE	
									24.4 RED	GRANITE	
									32.6 BLACK	GRANITE	
									35.6 RED	GRANITE	
									37.5 BLACK	GRANITE	
5120144	730888	4940546	244.4	8 m	33 68 LPM	Water Supply	Domestic	FRESH			
									1.5 RED	SAND	BOULDERS
									1.5 RED	SAND	BOULDERS
									4.80	GRANITE	HARD
									4.80	GRANITE	HARD
									22.8 GREY	GRANITE	HARD
									22.8 GREY	GRANITE	HARD
									32	GRANITE	HARD
									32	GRANITE	HARD
									33.2 BLACK	GRANITE	HARD
									33.2 BLACK	GRANITE	HARD
									35	GRANITE	GRANITE
									35	GRANITE	GRANITE
5120248	733452	4941419	241.3	8 m	90.9 LPM	Water Supply	Domestic	FRESH			
									1.52 BROWN	CLAY	ROCK
									1.52 BROWN	CLAY	ROCK
									67.1 RED	GRANITE	
									67.1 RED	GRANITE	
5120270	733350	4941473	250.2	10 m	51.81 68.2 LPM	Water Supply	Domestic	FRESH			
									1.82 BROWN	SAND	
									1.82 BROWN	SAND	

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Well I.D.	Easting N	Northing El	ev. V	VL Four	nd (m) Test	Rate S	Status	Use	Water Kind	Lith. Depth Lith. Cole		
										4.26 RED	SHALE	ROCK
										4.26 RED	SHALE	ROCK
										51.8 RED	GRANITE	
										51.8 RED	GRANITE	
5120299	737069	4939777	245.5	8 m	29 4	5 LPM	Water Supply	Domestic	FRESH			
										0.60 BROWN	SAND	STONES
										0.60 BROWN	SAND	STONES
										7 RED	GRANITE	HARD
										7 RED	GRANITE	HARD
										28.9 BLACK	GRANITE	HARD
										28.9 BLACK	GRANITE	HARD
										29.2 RED	GRANITE	POROUS
										29.2 RED	GRANITE	POROUS
										31.6 BLACK	GRANITE	HARD
										31.6 BLACK	GRANITE	HARD
5120324	735287	4939674	240.4	4 m	74 2	4 LPM	Water Supply	Domestic	FRESH			
										0.60 BROWN	SAND	SOFT
										0.60 BROWN	SAND	SOFT
										25.9 BLACK	GRANITE	HARD
										25.9 BLACK	GRANITE	HARD
										27.4 WHITE	QUARTZ	HARD
										27.4 WHITE	QUARTZ	HARD
										73.4 GREY	GRANITE	HARD
										73.4 GREY	GRANITE	HARD
										74.6 BLACK	GRANITE	POROUS
										74.6 BLACK	GRANITE	POROUS
										75.2 GREY	GRANITE	HARD
										75.2 GREY	GRANITE	HARD
5120325	735763	4942291	248.6	6 m	21.3 18.	2 LPM	Water Supply	Domestic	FRESH			
										0.30 BROWN	TOPSOIL	STONES
										0.30 BROWN	TOPSOIL	STONES
										21.3 GREY	GRANITE	
										21.3 GREY	GRANITE	
										30.5 BROWN	GRANITE	
										30.5 BROWN	GRANITE	
5120377	731934	4940804	247.1	34 ft	60.96	6 GPM	Water Supply	Domestic	FRESH			
										2.74 GREY	BOULDERS	SAND
										2.74 GREY	BOULDERS	SAND
										11.3 GREY	GRANITE	
										11.3 GREY	GRANITE	

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Well I.D.	Easting	Northing	Elev.	WL Fo	und (m) T	est Rate	Status	Use	Water Kind			ur Lith. Material	s
											8	GRANITE	
										1		GRANITE	
										36.	6 BLACK	GRANITE	
											6 BLACK	GRANITE	
										60.		GRANITE	
										60.	4	GRANITE	
										62.	2 RED	GRANITE	
										62.	2 RED	GRANITE	
										64.	0 BLACK	GRANITE	
										64.	0 BLACK	GRANITE	
5120553	737137	7 493981	2 241.	9 13 ft	23.77	5 GPM	Water Supply	Domestic	FRESH				
										6.7		GRANITE	HARD
										6.7		GRANITE	HARD
										8.8	4 GREY	GRANITE	HARD
											4 GREY	GRANITE	HARD
										23.	8 BLACK	GRANITE	HARD
										23.	8 BLACK	GRANITE	HARD
										2	5 RED	GRANITE	SOFT
										2	5 RED	GRANITE	SOFT
5120741	732899	9 493845	1 246.	7 13 ft	27.13	3 GPM	Water Supply	Domestic	FRESH				
											1 BLACK	TOPSOIL	
											1 BLACK	TOPSOIL	
										5.1	8 GREY	GRANITE	
										5.1	8 GREY	GRANITE	
										9.1	4 RED	GRANITE	
										9.1	4 RED	GRANITE	
										27.	1 BLACK	GRANITE	
										27.	1 BLACK	GRANITE	
										28.	0	GRANITE	
										28.	0	GRANITE	
										32.	0 BLACK	GRANITE	
										32.	0 BLACK	GRANITE	
5120749	730977	7 493837	3 239.	8 10 ft		14 GPM	Water Supply	Domestic					
											7 BLACK	GRANITE	
										10.	7 BLACK	GRANITE	
											1 BROWN	GRANITE	
										1	1 BROWN	GRANITE	
										22.	6 BLACK	GRANITE	
										22.	6 BLACK	GRANITE	
										22.	9	GRANITE	

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Well I.D.	Easting I	Northing E	lev. \	<b>NL</b> Four	nd (m) T	est Rate	Status	Use	Water Kind	Lith. Depth		ur Lith. Materials GRANITE	
5120786	726255	4020042	226.2	2.4	47.24	2 5 CDM	Motor Cupply	Domostio	FRESH	22.8	9	GRAINITE	
5120766	736255	4939842	230.3	311	47.24	3.5 GPM	Water Supply	Domestic	FRESH	3.04	5 BROWN	SAND	STONES
											5 BROWN	SAND	STONES
											5 BROWN	SAND	STONES
											7 GREY	CLAY	HARD
											7 GREY	CLAY	HARD
											7 GREY	CLAY	HARD
											2 GREY	GRANITE	HARD
											2 GREY	GRANITE	HARD
										37.2	2 GREY	GRANITE	HARD
										42.7	7	GRANITE	HARD
										42.7	7	GRANITE	HARD
										42.7	7	GRANITE	HARD
										46.0	0 BLACK	GRANITE	HARD
										46.0	0 BLACK	GRANITE	HARD
										46.0	0 BLACK	GRANITE	HARD
										47.2	2 RED	GRANITE	POROUS
										47.2	2 RED	GRANITE	POROUS
										47.2	2 RED	GRANITE	POROUS
										56.4	4 GREY	GRANITE	HARD
										56.4	4 GREY	GRANITE	HARD
										56.4	4 GREY	GRANITE	HARD
7042679	736270	4941974	236.9	1 m	62.48	45.5 LPM	Water Supply	Domestic	FRESH				
												TOPSOIL	
												TOPSOIL	
											8 BROWN	CLAY	SANDY
											8 BROWN	CLAY	SANDY
											2 GREY	CLAY	SANDY
											2 GREY	CLAY	SANDY
											9 BLACK	GRANITE	ROCK
											9 BLACK	GRANITE	ROCK
											5 RED	GRANITE	STONES
7044620	724020	4040764	250.0	6	70.0	4E E L DM	Motor Cumply	Domostic	EDECLI	02.3	5 RED	GRANITE	STONES
7044620	731039	4940764	250.8	0111	19.2	45.5 LPM	Water Supply	Domestic	FRESH	1 4	5 BROWN	SAND	
											5 BROWN	SAND	
											5 BROWN	SAND	
												SAND	
											3 GREY	GRANITE	
										02.0		=: # W W I	

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Well I.D.	Easting	Northing El	lev. \	<b>WL</b> Found	d (m) Te	est Rate	Status	Use	Water Kind	Lith. Depth Lit		Lith. Materials
										82.3 GR	REY GI	RANITE
										82.3 GR	REY GI	RANITE
7101362	736283	4943144	252	11 ft	15.54	3 GPM	Water Supply	Commerical	FRESH			
										1.22 BR	ROWN SA	AND
										7.92 RE		RANITE
										15.5 BL		RANITE
										19.2 RE	D GI	RANITE
7107320	736001	4941913	234.3	2 ft	50.29	2 GPM	Water Supply	Domestic	FRESH	5 40 DE	· D	
										5.49 RE		AND
										8.84 RE		RANITE
										50.3 GR		RANITE
74.07004	700070	4000004	000	0.6	40.0	47.ODM	M-1 0	Democific	FDFOU	62.5 RE	:D GI	RANITE
7107321	730670	4938001	239	9 ft	18.9	17 GPM	Water Supply	Domestic	FRESH	0.91 GR	PEV GI	RANITE
										11.9 BL		RANITE
										14.0 GR		RANITE
										18.9 BL		RANITE
										22.6 GR		RANITE
7108125	734726	4941377	234.2	0 m	23	36 LPM	Water Supply	Domestic	FRESH	22.0 01	(2)	0 11 11 2
7.00.120	701120	1011011	20 1.2	0	20	00 21 111	Water Cappiy	Bomoono	1112011	GR	REY	
											REY	
										5.2 BR	ROWN SA	AND
										5.2 BR	ROWN SA	AND
										29.6 WH	HITE GI	RANITE
										29.6 WH	HITE GI	RANITE
										33.5 GR	REY GI	RANITE
										33.5 GR	REY GI	RANITE
										88.4 WH	HITE GI	RANITE
										88.4 WH	HITE GI	RANITE
										113 WF	HITE GI	RANITE
										113 WH	HITE GI	RANITE
										122 GR	REY GI	RANITE
										122 GR	REY GI	RANITE
7108286	735719	4942169	244.3	11 ft	36.58	2 GPM	Water Supply	Domestic	FRESH			
										4.88 RE		AND
										9.75 RE		RANITE
										36.9 GR		RANITE
						_		_		42.7 BL	ACK G	RANITE
7115945	736750	4939720	237.7	8 ft	39.32	5 GPM	Water Supply	Domestic	Untested			

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Well I.D.	Easting	Northing El	ev.	WL Fou	nd (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith	ACK GR	Lith. Materials ANITE	
										14.0 BLA		ANITE	
										15.8 RED		ANITE	
										15.8 RED		ANITE	
										36.9 BLA		ANITE	
										36.9 BLA		ANITE	
										39.3 RED		ANITE	
										39.3 RED		ANITE	
										44.2 BLA		ANITE	
7404400	704440	4000040		2.4	70.40	C C DN	Matan Comple	Dublic	EDECH	44.2 BLA	ACK GR	ANITE	
7121483	734440	4939242		3 ft	70.10	5 GPM	Water Supply	Public	FRESH	3.66 BLA	ACK CLA	^~	
										80.8 RED		ANITE	
7132971	737363	4945747	270.7				Test Hole	Test Hole		00.0 KLL	D GR	ANTE	
7132971	131303	4343747	219.1				restriole	restrible		0.30 BRC	OWN ME	DIUM SAND	GRAVEL
										1.52 BRC			GRAVEL
										2.44 GRE		DIUM SAND	0.0
7132974	737447	4945666	301.4				Test Hole	Test Hole				2.0	
7134313	736534	4941581	235	8 ft	25.60	2.5 GPM	Water Supply	Domestic	Untested				
										0.61 BLA	ACK TO	PSOIL	
										0.61 BLA	ACK TO	PSOIL	
										5.18 GRE	EY GR	ANITE	
										5.18 GRE	EY GR	ANITE	
										12.8 BLA	ACK GR	ANITE	
										12.8 BLA	ACK GR	ANITE	
										21.6 RED	D GR	ANITE	
										21.6 RED	D GR	ANITE	
										25.6 BLA	ACK GR	ANITE	
										25.6 BLA	ACK GR	ANITE	
										32.0 RED	D GR	ANITE	
										32.0 RED	D GR	ANITE	
7134395	735032	4942105	249.9	28 ft	32.92	3 GPM	Water Supply	Domestic	Untested				
										1.22 BRC			STONES
										64.0 GRE	EY GR	ANITE	
7144264	734389	4941090	239.6	23 ft	37.49	15 GPM	Water Supply	Domestic	Untested	_			
										0.61 BRC			
										0.61 BRC			
										36 WHI		IESTONE	
										36 WHI	IITE LIM	IESTONE	

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Well I.D.	Easting N	Northing Ele	ev. WL Fou	ınd (m) 1	Test Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold	our Lith. Materials	
				,					42.7 RED	LIMESTONE	
									42.7 RED	LIMESTONE	
7144884	733917	4938978	241.4 25 ft	68.58	3 GPM	Water Supply	Domestic	FRESH			
									0.30 BLACK	TOPSOIL	
									0.30 BLACK	TOPSOIL	
									1.83 GREY	SHALE	
									1.83 GREY	SHALE	
									122 BLACK	GRANITE	
									122 BLACK	GRANITE	
7146501	731455	4940710	240.8 10 ft	89.92	2.5 GPM	Water Supply	Domestic	Untested			
									2.13 BROWN	SAND	GRAVEL
									49.4 WHITE	MARBLE	
									56.4 GREY	GRANITE	
									101 WHITE	MARBLE	
7151163	734745	4939558	238.5 18 ft	131.1	0.5 GPM	Water Supply	Domestic	FRESH			
									0.15 BROWN	TOPSOIL	
									0.91 BROWN	SAND	GRAVEL
									84.7 BLACK	GRANITE	
									99.1 BLACK	GRANITE	
									114 BLACK	GRANITE	
									171 BLACK	GRANITE	
7153007	735832	4941997	244.7 40 ft	62.48	2 GPM	Water Supply	Domestic	Untested			
									0.61 BROWN	TOPSOIL	
									73.2 GREY	GRANITE	
7153395	731256	4938456	238.3 15 ft	33.53	2 GPM	Water Supply	Domestic	Untested			
									1.22 BROWN	SAND	
									1.22 BROWN	SAND	
									29.3 BLACK	GRANITE	
									29.3 BLACK	GRANITE	
									33.5 RED	GRANITE	
									33.5 RED	GRANITE	
									63.1 BLACK	GRANITE	
									63.1 BLACK	GRANITE	
7153404	736355	4940258	14 ft	38.1	5 GPM	Water Supply	Domestic	Untested			
									1.83 GREY	GRAVEL	
									1.83 GREY	GRAVEL	
									38.1 GREY	GRANITE	
									38.1 GREY	GRANITE	
									40.2 RED	GRANITE	POROUS
									40.2 RED	GRANITE	POROUS
7155111	734745	4939558				Abandoned-Othe	er				

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Well I.D.	Easting	Northing Elev.	WL Fou	nd (m) T	est Rate	Status	Use	Water Kind	Lith. Depth	Lith. Colo	ur Lith. Materials	
7167920	735835	i 4941907	18 ft	22.86	12 GPM	Water Supply	Domestic	Untested	0.9	1 RED 1 RED	SAND SAND	
										7 GREY	GRANITE	
										7 GREY	GRANITE	
										4 RED	GRANITE	
74.07000	707000	4020440	444	00.70	40 CDM	Matan Comple	Damastia	Untratad	24.4	4 RED	GRANITE	
7167922	737328	4939112	44 ft	20.73	10 GPM	Water Supply	Domestic	Untested	1.8	3 RED	SAND	
										3 RED	SAND	
										2 GREY	GRANITE	
										2 GREY	GRANITE	
										3 RED	GRANITE	
										3 RED	GRANITE	
									24.4	4 GREY	GRANITE	
									24.4	4 GREY	GRANITE	
7169392	736541	4942035	13 ft	39.62	12 GPM	Water Supply	Domestic	Untested				
										7 RED	SAND	
										7 RED	SAND	
										6 GREY	GRANITE	
										6 GREY	GRANITE	
										7 RED	GRANITE	
									42.7	7 RED	GRANITE	
7175309	734245	4939217	16 ft	38.1	GPM	Water Supply	Domestic	Untested	20.	4		
									38. <sup>-</sup> 38. <sup>-</sup>			
									36. 38.			
7184437	736770	4939522	3 ft	7 315	8.5 GPM	Water Supply		FRESH	30.	1		
7 104437	730770	733322	311	7.010	0.5 GI W	Water Guppiy		TREOT	1.22	2 YELLOW	SAND	STONES
										2 YELLOW		STONES
										GREY	CLAY	SILT
									6.40	GREY	CLAY	SILT
									9.4	5 RED	GRANITE	
									9.4	5 RED	GRANITE	
7184439	735093	4942000	20 ft	27.13	5 GPM	Water Supply		FRESH				
										1 YELLOW		
										1 YELLOW		
										1 YELLOW		
									6.7	1 YELLOW	SAND	

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Well I.D.	Easting N	lorthing Elev.	WL Found	i (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith		Lith. Materials
									25.9 BLA		RANITE
									25.9 BLA		RANITE
									25.9 BLA		RANITE
									25.9 BLA	ACK GF	RANITE
									27.1 REI	D GF	RANITE
									27.1 REI	D GF	RANITE
									27.1 REI	D GF	RANITE
									27.1 REI	D GF	RANITE
									45.7 GRI	EY GF	RANITE
									45.7 GRI	EY GF	RANITE
									45.7 GRI	EY GF	RANITE
									45.7 GRI	EY GF	RANITE
									47.2 BR	OWN GF	RANITE
									47.2 BR	OWN GF	RANITE
									47.2 BR	OWN GF	RANITE
									47.2 BR	OWN GF	RANITE
									51.5 BLA		RANITE
									51.5 BLA		RANITE
									51.5 BLA		RANITE
									51.5 BLA		RANITE
7184698	730938	4940594	51 ft	25.91	12 GPM	Water Supply	Domestic	Untested			
						11,7			1.83 GRI	EY GF	RAVEL
									1.83 GRI	EY GF	RAVEL
									3.66 WH	IITE GF	RANITE
									3.66 WH	IITE GF	RANITE
									25.9 BLA	ACK GF	RANITE
									25.9 BLA	ACK GF	RANITE
									30.5 REI		RANITE
									30.5 REI		RANITE
7186951	732595	4940450	6 ft	37.49	4 GPM	Water Supply	Domestic	Untested			
						,			0.30 BR	OWN SA	AND
									0.30 BR	OWN SA	AND
									9.75 GRI	EY GF	RANITE
									9.75 GRI	EY GF	RANITE
									29 GRI		RANITE
									29 GRI		RANITE
									48.8 GRI		RANITE
									48.8 GRI		RANITE
7189267	736053	4939168	9 ft	91.44	4 GPM	Water Supply	Domestic	Untested			
									2.74 REI	D SA	AND

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STONES

Well I.D.	Easting N	Northing Elev.	WL Four	nd (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Cold		
									2.74 RED	SAND	STONES
									8.53 RED	GRANITE	
									8.53 RED	GRANITE	
									104 GREY	GRANITE	
									104 GREY	GRANITE	
7189663	736675	4939689	20 ft	38.1	8 GPM	Water Supply	Domestic	Untested			
									42.7 RED	GRANITE	
7198245	730875	4936697	12 ft	24.38	10 GPM	Water Supply	Domestic				
									2.44 RED	CLAY	BOULDERS
									24.4 RED	GRANITE	ROCK
7199568	734043	4941523	10 ft	64.01	15 GPM	Water Supply	Domestic	Untested			
									3.96 RED	SAND	
									3.96 RED	SAND	
									45.7 GREY	GRANITE	
									45.7 GREY	GRANITE	
									47.2 RED	GRANITE	
									47.2 RED	GRANITE	
									64.0 GREY	GRANITE	
									64.0 GREY	GRANITE	
									67.1 RED	GRANITE	
									67.1 RED	GRANITE	
7199570	736681	4940915	4 ft	44.81	3 GPM	Water Supply	Domestic	Untested			
									5.18 GREY	GRANITE	
									5.18 GREY	GRANITE	
									44.2 BLACK	GRANITE	
									44.2 BLACK	GRANITE	
									44.8 RED	GRANITE	
									44.8 RED	GRANITE	
									48.8 BLACK	GRANITE	
									48.8 BLACK	GRANITE	
7199571	734039	4941529				Abandoned-Qua	lity				
7203246	732038	4938471	20 ft	91.44	6 GPM	Water Supply	Domestic	Untested			
				•					0.61 RED	SAND	
									0.61 RED	SAND	
									91.4 GREY	GRANITE	HARD
									91.4 GREY	GRANITE	HARD
7203247	731036	4940692	17 ft	97.54	15 GPM	Water Supply	Domestic	Untested	OI.T OILE	<b>♥.</b> ₩ ₩ ₩ E	11/11/12
1200241	701000	-10-10002	1710	37.04	10 01 101	valor oupply	Domestic	Onicaled	1.22 BROWN	SAND	SOFT
									1.22 BROWN	SAND	SOFT
									64.0 GREY	GRANITE	HARD
									04.0 GNL 1	CITAINIL	HAND

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Well I.D.	Easting	Northing Elev.	WL Fou	ind (m) T	est Rate S	Status	Use	Water Kind	Lith. Depth Lith. Cold		
									64.0 GREY	GRANITE	HARD
									74.7 RED	GRANITE	
									74.7 RED	GRANITE	
									105 RED	GRANITE	HARD
7202206	726002	4944300							105 RED	GRANITE	HARD
7203396	736982	4944300							1.83 BROWN	SILT	SAND
7000440	70000	1011170							4.27 GREY	ROCK	
7203412	736860	4944179							0.76 GREY	FILL	ROCK
									4.57 GREY	ROCK	
7206040	735290	4941929	6 ft	8.534	2 GPM	Water Supply		Untested			
									0.91 YELLOW	SAND	
									0.91 YELLOW	SAND	
									5.49 BLUE	GRANITE	
									5.49 BLUE	GRANITE	
									7.32	GRANITE	
									7.32	GRANITE	
									57.9 WHITE	GRANITE	
									57.9 WHITE	GRANITE	
7209778	735071	4938512	6 ft	15.24	6 GPM	Water Supply	Domestic	Untested			
									0.30 BLACK	TOPSOIL	STONES
									0.30 BLACK	TOPSOIL	STONES
									3.66 RED	SAND	GRAVEL
									3.66 RED	SAND	GRAVEL
									11.3 RED	GRANITE	
									11.3 RED	GRANITE	
									18.9 BLACK	GRANITE	
									18.9 BLACK	GRANITE	
7212951	730737	4936790	9 ft	57.91	15 GPM	Water Supply	Domestic	Untested			
									0.61 BROWN	SAND	SOFT
									0.61 BROWN	SAND	SOFT
									8.84 GREY	GRANITE	HARD
									8.84 GREY	GRANITE	HARD
									42.7 RED	GRANITE	HARD
									42.7 RED	GRANITE	HARD
									57.9 GREY	GRANITE	HARD
									57.9 GREY	GRANITE	HARD
									61 BLACK	GRANITE	
									61 BLACK	GRANITE	

Monday, November 23, 2015 Page 53 of 56

<b>Well I.D.</b> 7215338	Easting Northing Ele 735361 4941843	v. WL Found (m) Test	Rate Status U	Use Water Kind Untested	Lith. Depth Lith. Color	ur Lith. Materials	
					1.52 YELLOW	SAND	
					1.52 YELLOW	SAND	
					5.49 GREY	GRANITE	
					5.49 GREY	GRANITE	
					36.6 BLACK	GRANITE	
					36.6 BLACK	GRANITE	
					42.7 WHITE	GRANITE	
					42.7 WHITE	GRANITE	
					76.2 BLACK	GRANITE	
					76.2 BLACK	GRANITE	
7215341	735477 4941878	20 ft 2	GPM Test Hole	Untested			
						TOPSOIL	
						TOPSOIL	
						GRANITE	
						GRANITE	
						GRANITE	
					38.1 GREY	GRANITE	
						GRANITE	
						GRANITE	
						GRANITE	QUARTZITE
						GRANITE	QUARTZITE
						GRANITE	
						GRANITE	
						GRANITE	
					89.0 BLACK	GRANITE	
7215344	735376 4941700	6 ft 20.48 6	GPM Test Hole	Untested			
					1.22 YELLOW		
					1.22 YELLOW		
					1.22 YELLOW		
					1.22 YELLOW		
					1.22 YELLOW		
					1.22 YELLOW		
						GRANITE	
					91.4 GREY	GRANITE	
						GRANITE	
						GRANITE	
						GRANITE	
					91.4 GREY	GRANITE	
7221696	735315 4942076	10 ft 33.53 3.5	GPM Test Hole	Test Hole Untested			

Monday, November 23, 2015 Page 54 of 56

w	/ell I.D.	Easting	Northing Elev	v. WL Fou	ınd (m) Te	st Rate	Status	Use	Water Kind			ır Lith. Mate	rials	
												SAND		GRAVEL
										2.13	3 RED	SAND		GRAVEL
										26.2	2 GREY	GRANITE		
										26.2	2 GREY	GRANITE		
										32.9	9 BLACK	GRANITE		
										32.9	9 BLACK	GRANITE		
										36.0	6 GREY	GRANITE		
										36.0	6 GREY	GRANITE		
										88.4	4 BLACK	GRANITE		
										88.4	4 BLACK	GRANITE		
										97.	5 RED	GRANITE		
										97.	5 RED	GRANITE		
72	225021	735500	4941762	9 ft	36.58	5 GPM	Water Supply	Test Hole	Untested					
												SAND		
										4.88	8 RED	SAND		
											9 BLACK	GRANITE		
												GRANITE		
												GRANITE		
										30.		GRANITE		
										39.0	0 BLACK	GRANITE		
										39.0	0 BLACK	GRANITE		
										42.	7 WHITE	GRANITE		
										42.		GRANITE		
										48.8	8 BLACK	GRANITE		
										48.8	8 BLACK	GRANITE		
72	225022	734948	3 4941450	6 ft	91.44	3 GPM	Water Supply	Domestic	Untested					
												SAND		
												SAND		
												GRANITE		
												GRANITE		
										92.0		GRANITE		
										92.0		GRANITE		
												GRANITE		
										104	4 GREY	GRANITE		
72	226465	734660	4938491	44 ft	21.34	2 GPM	Water Supply	Domestic	Untested					
												CLAY		
												CLAY		
												LIMESTONE		
												LIMESTONE		
										20.	7 RED	GRANITE		

Monday, November 23, 2015 Page 55 of 56

Well I.D.	Easting N	lorthing Elev.	WL Four	nd (m) T	est Rate	Status	Use	Water Kind	Lith. Depth Lith. Colo	ur Lith. Materials GRANITE	
									61 GREY	GRANITE	
									61 GREY	GRANITE	
7228857	736371	4941754	3 ft	39.62	2 GPM	Water Supply	Domestic	Untested			
									2.44 RED	SAND	
									2.44 RED	SAND	
									7.32 RED	GRANITE	
									7.32 RED	GRANITE	
									48.8 BLACK	GRANITE	
									48.8 BLACK	GRANITE	
7229219	731653	4938427	6 ft	9.144	8 GPM	Water Supply	Domestic	Untested			
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									0.30 BROWN	TOPSOIL	
									24.4 GREY	GRANITE	
									24.4 GREY	GRANITE	
									24.4 GREY	GRANITE	
7231661	731712	4938385	6 ft	13.72	10 GPM	Water Supply	Domestic	Untested			
									1.52 BROWN	SAND	GRAVEL
									1.52 BROWN	SAND	GRAVEL
									24.4 GREY	GRANITE	
									24.4 GREY	GRANITE	

Monday, November 23, 2015 Page 56 of 56

# **APPENDIX C**

Well Survey Letter & Questionnaire



### **Dear Homeowner or Occupant:**

On June 05, 2013, an Oakridge Environmental Ltd. staff member visited your property while conducting a door to door well survey. Our study is part of a hydrogeological study being commissioned by a neighbouring property.

Through this study, we are collecting data regarding water supplies and sewage treatment in the area. If you would like to participate, we would like to include information about your well and septic system in our study.

The purpose of the survey is to prepare an accurate database of local water supply conditions. If you are in receipt of this letter, it means that we attempted to contact you and you were unavailable. We have a brief questionnaire that we can complete with you by telephone, fax or e-mail (whichever is most convenient).

The success of our survey depends on obtaining accurate information. We view your water supply and sewage treatment system information as an important part of our survey. You are under no obligation to participate in our survey, however if you are interested in participating, please contact our office at your earliest convenience using the information provided below.

We thank you for your time.

Christa Lemelin

### Oakridge Environmental Ltd.

P.O. Box 431 380 Armour Road, Suite 127 Peterborough, Ontario K9J 6Z3

telephone: (705) 745-1181

1-888-OAKRIDGE (625-7434)

fax: (705) 745-4163

1-877-796-7781

email: christa.ore@bellnet.ca

www.oakridgeenvironmental.com

## **WATER SUPPLY SUMMARY**

	For Office Use Only
Township:	Ву:
Hamlet/Town:	Project No:
Lot: Concession:	MOEE #:
Well Owner:	Ref. No:
Mailing Address:	
Phone:	Date:
Type of Residence: (house, seasonal cottage,	business, etc.)
WATER SUPPLY SOURCE	
Dug Well: ☐ Drilled Well: ☐ Lake/River: ☐	Other:
Well Depth: Diameter:	
Well Construction:	
Well Drilled by:	
WATER QUANTITY	
Never Dry:   Occasionally Dry:   Often D	ry: 🗅 Last Date:
Ever hauled water? Last Date:	Contractor:
WATER QUALITY	
Odour Problems (describe):	
Taste Problems (describe):	
Turbidity Problems (describe):	
Staining (describe):	
Bacteria Problems (describe):	
Other:	
Ever had water sampled? Bacteria?   Che	mical?   Last Date:
WATER TREATMENT	
Water Softener:	
Chlorinator:	
Filter:	
Other:	

PROPERTY AND WATER USE	
Lot Size: No. of Residents: No. of Washrooms:	
No. of Bedrooms:	
SEWAGE DISPOSAL	
Tile Bed: Raised: ☐ In-ground: ☐	
Problems: Odours: ☐ Breakouts: ☐ No problems: ☐	
System Age: Constructed By:	
Distance to Well: Direction: (eg. Upgradient)	
Distance to Building:	
PROPERTY SKETCH	
(showing house, well, and tile bed locations)	

# **APPENDIX D**

Individual On-Site Well Records

eg Y			,000 j		errore.		
	• : N#11-	4 E	T A	<b>4112514</b>	n/Print BéloW)	77	1A/
V Onta	ario Minis	nvironment			$-H = H / M H_{\odot}$	)) ∖√/ Régulation 903 Ontari	Well Record  Water Resources Act
Measurements r	recorded in:	Metric   Impe	rial /	9112514		11	Page of
Well Owner's	Information	el agricultura de la companio del companio de la companio del companio de la companio della companio de la companio della comp					
First Name		Last Name / Orga	nization	20 - Than Ti	E-mail Address	Happing and the second	☐ Well Constructed
	(Street Number/Na	ime)	glam	Municipality	Province F	Postál Code 🔣 Teleph	by Well Owner one No(inc. area code)
".V., (DOX 5	10 04/	D (Ash	40/2	Kling Win	11/1/	444344957	1757776061
Well Location	ocation (Street Nu		· 居,在"走"。[2] 在"四	1. 4. 4.78 1. 8. 8. 8. 8.	可考定。在189世年,1987年		
2450	100 Street NO	imper/ivame)	Not.	Township	anasta 1	of 3 Conce	ssion/
County/District/M				City/Town/Village		Province	Postal Code
UTM Coordinates		n Northi	20 /	ACC RAGICAL		Ontario	MPERMIN
NAD 8 3		0931449	"ADDO	Municipal Plan and Sub	lot Number	Other /	97. 15.30
	d Bedrock Mater	lals/Abandonm	ent Sealing Reco	ord (see instructions on th	e back of this form)		
General Colour	l .	mon Material		ner Materials		Description	Depth (m/fi) From To
<u> 129jn) - </u>	Lence						0 100
	GROWILL	<u> </u>					33 55
	JAMES.	L.C.					F5 FF
Harry .	FRANCE	j.					F9 150
May 18	JARNER.						790 195
	GAARL						199 779
STATE OF ALL PARTS	1 2 <sup>0</sup> 4 Day (195, 24, 24, 34,	Annular Spa	rce 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	TO THE STATE OF CHARMAN	Por	suits of Well Yield Tes	11 - 12 - 12 - 12 - 12 - 12 - 12 - 12 -
Depth Set at (n	And the second s	Type of Sealant	Used	Volume Placed	After test of well yield, wat	er was: Draw Do	
a a		(Material and Ty	pe)	(m³/ft³)	☐ Clear and sand free☐ Other, specify	11	r Level Time Water Level
	12 N 12 C	A Comment of the Comm			If pumping discontinued, g	give reason: Static	
			1		Property Section 44 1-25	Level 1	70 17/9240
					Pump intake set at (m/ft)	2	
					160	1 105	2 1/3/90 90 a 1/0 m
	f Construction	44.75.4.7"   41.   3557.   -1.219 1.47.45.454.9	Well Us	ie za projekty	Pumping rate (I/min / GPI	M) 3 (75) -	3 /////
☐-Cable Tool  ✓☐ Rotary (Conver	☐ Diamon ntional) ☐ Jetting	d □ Public □ Domest	Comme		Duration of pumping	4 3/1	90 4 07,70
☐ Rotary (Reverse	e) Driving Digging	Liveston	k 🔲 Test Ho	le	hrs + min	() ' ["	-/0 5 KN 30
Air percussion	LJ vigging	☐ Industria	al	& Air Conditioning	Final water level end of pu	Imping (m/fl) 10	10 10 13370
Other, specify_		Other, s			If flowing give rate (I/min.	11 1 16 1	0 15 96 V
	Construction F en Hole OR Material	Record - Casing Wall	Depth ( <i>m/ft</i> )	Status of Well		20 97	190 20 10 20
Diameter (Gal	lvanized, Fibreglass, crete, Plastic, Steel)	Thickness	rom To	Replacement Well	Recommended pump de	25 /5 4	90 25 1 9 00
64 1	A CONTRACT	188 12	2 - 22	Test Hole Recharge Well	Recommended pump ra	() A.C.	30 77 30
	10000	2.	n 200	Dewatering Well	(l/min / GPM)		4.69
	the state of the s	5710	2 /0/	Observation and/or Monitoring Hole	Well production (I/min / G	(49.00	70 40 60 60
				Alteration (Construction)	Disinfected?	50,700	50 (2)
				Abandoned, Insufficient Supply	Yes No	60 / // /	90 David
Outside		Record - Screen	Depth ( <i>m/īt</i> )	Abandoned, Poor	Plance provide a resultati	Map of Well Location	Alanger (Section)
Diameter	Material ic, Galvanized, Steel)	Slot No.	rom To	Water Quality Abandoned, other,	Liease hipyine a map bei	ow following instructions or	the back.
	a company of the same of the s			specify		Mead The	<u> </u>
	- Article Control of the Control of			Other, specify		tal Sa	4757
Provide a Security Security	2 Marie 2 mg	ACTION OF STREET	Market 7 115 - AR (115) - AR (115)		<u> </u>	A.	
Water found at D	Water De	<b>tails</b> 2		Iole Diameter th (m/ft) Diameter	and the state of t	Control of the second s	N. 420
<i>○ / (m/ft)</i> [	Gas Other, sp	ecify <u>/</u>	From	To (cm/in)	STENER		
6 6015-3	epth Kind of Wate	4	ntested		1 January	and the state of	
	Gas Other, sp epth Kind of Wate		tested		I may be to	480	1
	Gas Other, sp					and the second s	
	Well Contract	or and Well Tec			A CONTRACTOR OF THE CONTRACTOR		1
Business Name o	f Well Contractor	4 600 1	We We	Contractor's Licence No.			
Business Address		ame) // //	MI MI	injoipality	Comments:		<u></u>
1013,1	Street Number/N		). Fo	inicipality  (Control of the Control			
Province	Postal Code	Business E-m	nail Address				
Bus.Telephone.No.	(inc. area code), N	Tame of Well-Techn	nician (Lást Name	First Name)	Well owner's   Date Pack	age Delivered Audit	Ministry Use Only No
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Well Technician's Lic	cence No. Signature	of Technician an	d/or Contractor Da	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	I I I/Vee	Completed	- L C I 4 J J
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County/Dist	trict/Municip	pality /	1807	74		City/Town/Village	usurre.		Provin	nce	Posta	I Code
eles	Esson	96				Va Refuela			Ont	ario		
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General Co			mon Material			<b>ord</b> (see instructions on the her Materials	pack of this form)	General Description				pth (m/ft)
- Ocheral Oc	oloui	12000	THOT Wateria		01	ner waterials		General Description			From	To
Stagn	1,11	wee	tun								4	200
Stall	11) 9	mani	U								2	18
19 (00)	2 9	sanite									18	24
1/1/1	Bill	man	vile								24	190
		7,									/	
								*				
												2
-												
			Annular	Space				Results of We	ell Yie	d Testing		
Depth Set	t at (m/ft)		Type of Sea		d	Volume Placed	After test of well	yield, water was:	-	aw Down	F	Recovery
From	То	2	(Material ar	nd Type)	,	(m³/ft³)	Clear and		Time			Water Level
0	00	Dean	onete	asc	ret	,	Other, spec		(min) Static	(m/ft)	(min)	(m/ft)
							If pumping disco	ontinued, give reason:	Level	600		15 30
									1	7.68	1	130.50
							Pump intake se	et at (m/ft)	2	8.70	2	129.70
							185		3	070	3	175.00
Meth	od of Cor	nstruction			Well U	se	Pumping rate (	l/min / GPM)	3	70/0	3	0000
Cable Too		Diamon			Comme		Duration of pur	mping	4	10-10	4	125010
Rotary (C		Jetting  Driving	A STATE OF THE STA	mestic	Municip		hrs +	min	5	11.70	5	127.80
Boring		Digging	□ Irri			& Air Conditioning	Final water level	l end of pumping (m/ft)	10	11 13	10	174.00
☐ Air percus				lustrial her, <i>speci</i>	·6.		199	9		3000		1222
Unter, spe						T 01 / 6111 II	If flowing give ra	ate (I/min / GPM)	15	00,00	15	100 -50
Inside		oR Material	Wall	T	epth (m/ft)	Status of Well  Water Supply	Recommended	pump depth (m/ft)	20	23.80	20	120.40
Diameter (cm/in)	(Galvanize	d, Fibreglass, Plastic, Steel)	Thickness	From		Replacement Well	145	pump deput (mm)	25	2730	25	17.30
(CHVIII)	Concrete, i	Plastic, Steel)	(cm/in)	100	1- 0-	Test Hole	Recommended	pump rate	30	20 70	30	1100 70
64	Sleet	, ,	100	1/0	I -de	Recharge Well Dewatering Well	(I/min / GPM)	,	30	2-70	/	19.00
6# X	3 pen	hote		20	190	Observation and/or	Well production	(I/min / GPM)	40	3/.60	40	11.60
						Monitoring Hole  Alteration	100	2	50	15.10	50	107.80
						(Construction)	Disinfected?  Yes N	do	60	52.30	60	1244
						Abandoned, Insufficient Supply	les l			n.v	/	110
Outside		onstruction F	Record - Scre	T	epth ( <i>m/ft</i> )	Abandoned, Poor Water Quality	Please provide a	Map of W a map below following			ack	
Diameter (cm/in)		aterial vanized, Steel)	Slot No.	From		Abandoned, other,	l load provide	Helroger pi	2			h
(CHVIII)						specify	1heros	1 Bearth				13
						Other, specify	10011/2	1				15
												×
	ay to Keep	Water De	tails	\ ,		Hole Diameter	$  \wedge \rangle$					12
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	en and Bedrock Mater	ials/Abandonment Se	aling Reco	ord (see instructions on the	e back of this form	7)			
General C		mon Material		ner Materials	and an area	General Description		Dep	th (m/ft)
10000	mond							1	2/
hand !	Water Man	anite						17	200
20/	onic gra	MULL						7	200
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Depth Se	et at ( <i>m/ft</i> )	Annular Space Type of Sealant Used		Volume Placed	After test of we	Results of We	Draw Down		ecovery
From	To	(Material and Type)	-	(m³/ft³)	Clear and	sand free	Time Water L	evel Time	Water Level
2	20 Bene	onite grou	Z		Other, sp		(min) (m/ft)	) (min)	(m/ft)
					It pumping also	continued, give reason:	Level 0 17	0	
							1/0,3	0 1	62.50
					Pump intake	set at (m/ft)	2/208	0 2	50,50
					0/	(Veries / OPA ()	3 15.9	0 3	572n
	hod of Construction		Well Us		Pumping rate	D COM	100		0/100
Cable To	conventional) Diamon	d Public Domestic	☐ Comme		Duration of pu	umping	4/80/	0 4	55.00
Rotary (F		Livestock	☐ Test Ho		hrs +	O min	5 3019	15 5	52.75
☐ Boring ☐ Air percu	Digging	☐ Irrigation ☐ Industrial	☐ Cooling	& Air Conditioning	Final water lev	el end of pumping (m/ft)	10 321	0 10	43.30
Other, sp		Other, specify			If flowing give	rate (I/min / GPM)	15 36.4	10 15	37.20
27.000	Construction R	Record - Casing	10 to 0000	Status of Well	I I nowing give	Tate (Milli / GFM)	161	,	20 7-
Inside Diameter	Open Hole OR Material		( <i>m/ft</i> )	☐ Water Supply	Recommende	ed pump depth (m/ft)	20 44.6	20	50.10
(cm/in)	(Galvanized, Fibreglass, Concrete, Plastic, Steel)	Thickness (cm/in) From	То	Replacement Well Test Hole	659	15	25	25	26-55
62	Most	188 1.5	-20	Recharge Well	Recommende (I/min / GPM)	ed pump rate	30 53-6	30	23.20
16	mee )	100	300	Dewatering Well		0	40 50	75 40	15.70
616	spen poll	20	000	Observation and/or Monitoring Hole	Well production	on (I/min / GPM)	50 ///	50	15 7a
				Alteration (Construction)	Disinfected?		50 64.0	50	210
				Abandoned,	Yes 🗌	No	60 6/1/	9 60	2,75
	Construction F	Record - Screen		Insufficient Supply  Abandoned, Poor			ell Location	ante	
Outside Diameter	Material (Start)	Slot No.	(m/ft)	Water Quality Abandoned, other,	Please provide	e a map below following	instructions on the	ne back.	
(cm/in)	(Plastic, Galvanized, Steel)	From	То	specify	11-				_
					1/1/	Table Inches			
				Other, specify	///				
	Water De	tails	-	lole Diameter	11				
Water foun	nd at Depth Kind of Water		Dep	th (m/ft) Diameter		The	0/		
	n/ft) Gas Other, sp		From	To (cm/in)		1 80			
7 (0)	nd at Depth Kind of Water								
	n/ft) ☐ Gas ☐ Other, sp nd at Depth Kind of Wate								
192	n/ft) Gas Other, sp				×		6		
	, ,	or and Well Technicia	n Informa	tion					
Business N	ame of Well Contractor	1/+//		ell Contractor's Licence No.				-	
Yell	supples.	walls well	0 6	5564	Comment				
Business A	ddress (Street Number/N	arne)	Mi	unicipality	Comments:				
Province	Postal Code	Business E-mail Add	Iress	ever -					
ON	* OXXX	3	)		Well owner's	Date Package Delivere		nistry Use	Only
Bus.Telepho	one No. (inc. area code) N	ame of Well Technician (I	ast Name,	First Name)	information package	2017 ROZV	Audit No		1606
Woll Took	ian's License No. Singer	would rec	(I)	to Cuberitte d	delivered	Date Work Completed		120	1686
vveii rechnic	ian's Licence No. Signature	or recrimician and/or Co	muactor Da	le Submitted	No	201307	613 Receive		
117	3 1	7//0///	and the same of th	Y Y WY LIKAN BA LILL	110				

<i>b</i> >0	)ntario	Minist the En	ry of vironment	W <sub>4</sub>	Tag#: A1309		Regulation 903 Onta	Well Record
	ents recorded		Metric 💢 Im	sperial	<u>A 130956</u>		j	Page of
First Name	6 <u>6 - 100 (000</u>		ast <b>Na</b> me / Or	roshization.				
i ii at i veime	•	,	Knal	Promote	extinutial	E-mail Address		Well Constructed by Well Owner, ∴
Mailing Ad	dress (Street N	lumber/Nar	me)	/ /_	Municipality	Province	Postal Code Tele	aphone No. (inc. area code)
	V 3/V	2/5	CAZA	4 PT	Wangnam		NOBDUO	
Address of	Well-Location	(Street Nur	mber/Name)	ニュース - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	Township		Lot , Cor	ncession ,
426	Helgsu	<i></i>	10/ X	Z	North K	runtipa	13	//
County/Dis	strict/Municipali	<b>Y</b>			City/Town Village		Province Ontari	Postal Code
UTM Coord	finates Zone	Easting	Nort	سے ہم ہر thing	Municipal Plan and Sut	lot Number	Other	•
	[8] 3 <b>/</b> ]	73 GK		34/8/8		and the same of th		,
Overhalia General C		ck Materi	ais <b>(</b> non Material		<b>ecorc</b> se <i>liqus on</i> a Other Materials	26		Depth ( <i>m/ft</i> )
P		2	Ilon Material	•	Other Materials	Gene	ral Description	From To
<u> </u>	20	<u>"                                    </u>	<u> </u>			- Para	•	2 1/2
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XY 74.	shitz	gsa.	nu _					49 129
Gef-g	(4)	9/54	mile			"		125 195
rack,	00 91	anel	<i>y</i>		styll			195 250
MITE, L	5f ( g	<u>Vanii</u>	T		<u> </u>			250 259
<u>Black</u>		Manil	<u> </u>			. :		259 292
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rem delicate e reference e e		and the second s					and state of the s	esding
Depth Se From	et at ( <i>m/ft</i> ) To		Type of Seala (Material and		Volume Placed (m²/ft²)	After test of well yield,  Clear and sand fr		
1)	20	Long	miti	asot	(,2)	Other, specify	(min)	(m/ft) (min) (m/ft)
μ		LIVIA	vicer g	1740no	<i>y</i> .	If pumping discontinue	d, give reason; Static	0,10
							1 1/2	1,95 1/22,40
	ļ					Pump intake set at (n	7/ft) 2 /A	6,00 2 27.88
		,				1 230		
ill made such		guellen 💆	Ven	- <b>W</b> ei	4 April 2 Apri	Pumping rate (Vmin /		706 7001,21
□ Cable To Rotary (		□ Diamond □ Jetting	Public		nmercial 🔲 Not used nicipal 🔲 Dewatering	Duration of pumping		0,08 4 121,62
🗀 Rotary (F	Reverse) [	Driving	Lives	tock Tes	t Hole	hrs +n	11 44 3	1,15 5/2/-30
☐ Boring ☐ Air percu		☐ Digging	☐ Irrigat		ling & Air Conditioning	Final water level end of	pumping (m/ft) 10 27	7.25 10/18-78
Other, sp		···		r, specify		If flowing give rate (I/m	nin / GPM) 15 3	3 05 15 1/628
Inside			o <b>děrda je na s</b>	Depth ( <i>m/ft</i> )	Stansipy Wall	"1 I	20 21	5.50 20 1/3.96
Diameter (cm/in)	Open Hole OF (Galvanized, F Concrete, Plas	ibreglass,	Wall Thickness	From To	☐ Water Supply ☐ Replacement Well	Recommended pump	depth (m/ft) 25	S. S. 25/1/16
11-	CONCIDIO, FIAS	r suc, steel)	(cm/in)	1	Test Hole  Recharge Weli	Recommended pump	rate 30 /	F O 1 30 100 10
04	State_		/}\$	19 2	Dewatering Well	(Vmln / GPM)	470	70/275
				7 <i>0 29c</i>	Observation and/or Monitoring Hole	Well production (I/min	7 GPM) 40 50	-55 40/05/13
				'	Alteration (Construction)	Disipfected?	50	7.60 50 100.90
			~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~		Abandoned,	Yes 🗆 No	60 7	125 60 1715
andrew S	A. Cons	<u> BERLIN</u>			Insufficient Supply  Abandoned, Poor		War www.	
Outside Diameter	Materia (Plastic, Galvani	al ized Steen	Slot No.	Depth ( <i>m/ft)</i>	Water Quality ☐ Abandoned, other,	Please provide a map l	below following instructions	on the back.
(cm/ln)	( storie) Server	200, 010017		From To	specify specify	,	/ 1\ /	
					Other, specify		1 I'N	
					kao (SSI) (filiateria) (SSI)	Med	-	
	d at Depth Kin		~ 1	Untested [ Fror	Depth ( <i>m/ft)</i> Diameter To (cm/in)	110		
	n/ft) ☐ Gas ☐ d at Depth Kin				20 8	]		•
(m	v/ft) ☐ Gas ☐	Other, spec	cify	20	3 1/-	111.		
	d at Depth Kin			Untested 670	270 00	11 1		
(m	//ft) ☐ Gas ☐			o littori		11 -		
	eme of Well Co	priactor		1 /17 -	Well Contractor's Licence No.	₹  <i> </i>	Du	sproon > )
Keiz	T (UM)	110 0	unar	UNIO .	6964	<u>  `</u>	——————————————————————————————————————	
Business Ac	dress (Street N	1 10 1			Municipality	Commenta:		
Province	Posta	Code	Business E	-mail Address	MUCH			
ON	do	والمستراني المرا	71		<u> </u>	Well owner's Date Pa	ackage Delivered	Only Jan
Bus.Telephoi	ne No. (inc. area	code) Nar	ne of Well Tec	hnician (Last Man	ne, First Name)	package deliyered	<b>₹-3</b>   M   M   B  D   1   1   1   1   1   1   1   1   1	
Well Technicia	an's Licence No.	Signature	of Technician	end/or Contractor	Date Submitted	Yes Date/W	ork Completed	
$\angle$	32	<i>FE</i>	UNIU	end/or Contractor		125 No and 1/2	3/1/20 E	Pvet
0506E/(2007/1	2) @ Queen's P	rinter for Onta	irio, 2007		Ministry's Copy		/ - 30	0.000

Ontario Ministry of the Environ	onment Well	Tag#: A1309		V n 903 Ontario W		Record
Measurements recorded in:	ic Imperial	130958		Page	9	of
Well Owner's Information First Name Last	Name / Organization	Twited.	E-mail Address			Constructed
Mailing Address (Street Number/Name)	yel roomes	Municipality /	Province Postal Code	Telephone	,	ell Owner area code)
T.O. Doy 213 Use.	hur St. &	Jungham	ON MOGRA	40		
Well Location  Address of Well Location (Street Number	r/Name)	ownship	Lot	Concessi	on	
786 Hubsam 0	rest Ha.	Visetokau	estre 3	Concessi	/	
County/District/Municipality		City/Town/Village	/	Province	Posta	Code
UTM Coordinates   Zone , Easting	Northing	Municipal Plan and Sublo	ot Number	Ontario Other		
NAD   8   3   7   7   8   8   6	14941843					
Overburden and Bedrock Materials/					Don	the (see (fit)
General Colour Most Common	Material Oth	ner Materials	General Description		From	oth ( <i>m/ft</i> ) To
Gellow sand	-		hard		0	9
neypood granile			lasa	, ,	9	15
Olar L of granell		4	While & Drown G.	sance,	8	100
Upple De Granell			D	1	20	140
the L granile			Sono grey		40	250
	Annular Space	anticompanie frances		ell Yield Testin		
	pe of Sealant Used aterial and Type)	Volume Placed (m³/ft³)	After test of well yield, water was:	Draw Down Time Water Lev		ecovery Water Level
0 20 Benton	ite prost	1	Other, specify	(min) (m/ft)	(min)	(m/ft)
	w yroc		If pumping discontinued, give reason:	Static 7 6	7	
			-	1 11.1:	3 1	161,45
			Pump intake set at (m/ft)	2 14, 3	0 2	159.57
			Pumping rate (I/min / GPM)	3 17.50	3	15E 03
Method of Construction	Well Us  ☐ Public ☐ Comme		5	4 20-6	5 4	155 FD
☐ Cable Tool ☐ Diamond ☐ Rotary (Conventional) ☐ Jetting	☐ Public ☐ Comme ☐ Domestic ☐ Municip	al Dewatering	Duration of pumping	5 23.73	5	152 05
☐ Rotary (Reverse) ☐ Driving ☐ Digging	☐ Livestock ☐ Test Ho☐ Irrigation ☐ Cooling	le	hrs + min Final water level end of pumping (m/ft)	4-70	2	20173
Air percussion	☐ Industrial	arm conditioning	B 163.50	10 0/170	10	45,90
Other, specify	Other, specify	Chatura of Mall	If flowing give rate (I/min / GPM)	15 90,4	5 15	364
Inside Open Hole OR Material	Wall Depth (m/ft)	Status of Well  Water Supply	Recommended pump depth (m/ft)	20 600/l	20	29.60
	nickness From To	Replacement Well Test Hole	245	25 73.7	25	129-95
64 None	18f -15 20	Recharge Well	Recommended pump rate (I/min / GPM)	30 8	30	16.45
tothe course hold	20 200	Dewatering Well Observation and/or	5	40 90,4	5 40	124.50
Os open me	00 000	Monitoring Hole	Well production (I/min / GPM)	50 186	9 50	92/0
		Alteration (Construction)	Disinfected?	120-	, /	CW as
		Abandoned, Insufficient Supply	Yes No	60/05-6	60	Trow
Outside Material	rd - Screen  Depth (m/ft)	Abandoned, Poor Water Quality	Please provide a map below following	ell Location instructions on the	back.	
	Slot No. From To	Abandoned, other,	NORTHEY.	4/6/		
		specify	Day 9	EI/N		
		Other, specify	Se 124	11		
Water Details		lole Diameter	office			
Water found at Depth Kind of Water:	Fresh Untested Dep	th (m/ft) Diameter	W C			
(m/ft) Gas Other, specify		To (cm/in)	3 X			
Water found at Depth Kind of Water:   (m/ft) Gas Other, specify		00	100			
Water found at Depth Kind of Water:		250 68	30			
(m/ft) Gas Other, specify					-	
	nd Well Technician Informa	tion ell Contractor's Licence No.			1	
Business Name of Well Contractor	ustes upla	Sil Contractor's Licence No.	4/1/ #5			
Business Address (Street Number/Name)	/ 2/ ML	unicipality	Comments:			
36/3 NOUNTE MOO	- Me. to	welch				
Province Postal Code E	Business E-mail Address		Well owner's Date Package Delivered	ed Min	istry Use	e Only
Bus.Telephone No. (inc. area code) Name of	of Well Technician (Last Name,	First Name)	information package	Audit No.		
Well Technician's Licence No. Signature of T	Table Hells	to Submitted	delivered  Date Work Completed	Z	12	0691
vveir regrinicians Licence No. Signature of 1	Danisian and/or Contractor Da	PAR MAN 646	No 201 3 Kg/	602 Received		
0506E (2007/12) © Queen's Printer for Ontario, 2	2007	Well Owner's Con	The state of the s	- teceived		

	Well Tag No. (Place Sticker a			Record
Measurements recorded in:  Metric Imperial	A 138967	Regu	lation 903 Ontario Water Re	,
Well Owner's Information			Page /	OT /
First Name   Last Name / Organization	1	E-mail Address		Constructed
Mailing Address (Street Number/Name)			by W	/ell Owner
Po. Box 213 APTHUR 57	Municipality  Willeriam	Province Postal	Code Telephone No. (inc	area code)
Well Location		10 - 1 PP	<u> </u>	
Address of Well Location (Street Number/Name)	Township	Lot	Concession	
County/District/Municipality	City/Town/Village	PT	Drawinson   Drawin	10-1-
PETERBOLOUSH	LAKEFIELD		Province Posta Ontario	al Code
UTM Coordinates Zone Easting Northing	Municipal Plan and Subl		Other	
NAD 8 3 / 7 7 3 5 3 / 5 4 9 4 2 6	16			
Overburden and Bedrock Materials/Abandonment Seali General Colour Most Common Material	ng Record (see instructions on the Other Materials	General Desci	intion De	pth (m/ft)
			From	To
	BRAUEL	SOFT	7	011
GREY GRANITE		HARO	1	86
BLACK GRANITE		1-tars	86	
GREY GRANITE		HARD	108	1/20
BLACK GRANITE		HARO	/20	290
PINK GRANITE		HARD	290	320
Annular Space Annular Space			of Well Yield Testing	
Depth Set at ( <i>mlft</i> )  From To  Type of Sealant Used ( <i>Material and Type</i> )	Volume Placed (m³/ft³)	After test of well yield, water was	Time Water Level Time	Recovery Water Level
0 20 BENTONITE SCUR		Other, specify	(min) (m/ft) (min)	(m/ft)
1)010/10/11/2 320/		If pumping discontinued, give re	ason: Static Level	
			1 /3.34 1	110.04
		Pump intake set at (m/ft)	2 14.34 2	100 24
			3 45.37 3	101.03
Method of Construction	Well Use	Pumping rate (//min / GPM)		106,75
	☐ Commercial ☐ Not used ☐ Municipal ☐ Dewatering	Duration of pumping	4 16 45 4	105.5 %
☐ Rotary (Reverse) ☐ Driving ☐ Livestock ☐	Test Hole	2 hrs + 0 min	5 /7.42 5	164.55
☐ Air percussion ☐ Industrial	Cooling & Air Conditioning	Final water level end of pumping	10 25.62 10	98.79
Other, specify Other, specify		If flowing give rate (Ilmin / GPM	15 31.46 15	94.09
Construction Record - Casing	Status of Well		20 37 69 20	89 81
Inside Open Hole OR Material Wall Depth (and Diameter (Galvanized, Fibreglass, Cemlin) Concrete, Plastic, Steel) (cmlin) From	m/ft) Water Supply  To Replacement Well	Recommended pump depth (r	25 43 67 25	81 63
(Chinny)	Test Hole	Recommended pump rate	30 48 54 30	97 51
618 STEEL 188 0	Recharge Well  Dewatering Well	(Ilmin I GPM) 3-5	= 3//	34.00
618 OPEN HOLE 20	3 26 Observation and/or Monitoring Hole	Well production (Ilmin / GPM)	40 5 7, 66 40	16.36
	☐ Alteration	Disinfected?	50 65.50 50	70.68
	(Construction)  Abandoned,	Yes No	60 74.1 60	66.35
Construction Record - Screen	Insufficient Supply Abandoned, Poor	Мар	of Well Location 4,95	
Outside Diameter (Plastic, Galvanized, Steel) Slot No. Prom	Abandanad ather	Please provide a map below fol	lowing instructions on the back.	1
(cmlin) (Plastic, Galvanized, Steel) From	To Abandoned, other, specify	MODINE	BAY RD _	N
	Other, specify	100121615	10141 20	
	———————		76	
Water Details	Hole Diameter	Quea		OWELL
Water found at Depth Kind of Water: Fresh Untested	Depth ( <i>m/ft</i> ) Diameter From To ( <i>cm/in</i> )	CHEBOUTA	UNIT TE	
// (m/ft) ☐ Gas ☐ Other, specify  Water found at Depth Kind of Water: ☐ Fresh ☐ Untested	0 20 83/4	RD	11 9	
(m/ft) Gas Other, specify	20 320 6/8		]]	
Water found at Depth Kind of Water: Fresh Untested	0 )00 08			
(m/ft) Gas Other, specify	Information			
Well Contractor and Well Technician Business Name of Well Contractor	Well Contractor's Licence No.		110-	\
WENSLEY WATER WELL LY		STON	- CAKE	
Business Address (Street Number/Name)	Municipality FETTREORAGEH	Comments: PUMPED	FOR 2HRS U	INTER.
Province Postal Code Business E-mail Addre		LEVEL @ 114.		7
ON KOKRYID		Well owner's Date Package D	Pelivered Ministry U:	se Only
Bus.Telephone No. (inc. area code) Name of Well Technician (La		information package	A M D D Audit No.	400
Well Technician's Licence No. Signature of Technician and/or Con	tractor Date Submitted	delivered Date Work Com		439
0632	20 44 W = 313	29144	M M B To Received	
0506E (2007/12) © Queen's Printer for Ontario, 2007	Well Owner's Cop	V	TOUGHOU	

Well Tag No. (Place Sticker and/or Print Below) Ministry of Well Record the Environment Regulation 903 Ontario Water Resources Act 167169 Metric Imperial / of / Page\_ **Well Owner's Information** First Name Last Name / Organization E-mail Address Well Constructed by Well Owner ROYAL HOMES LIN Mailing Address (Street Number/Name) Municipality Province Postal Code Telephone No. (inc. area code) PRTHUR ST PO BOX 213 WINGHAM ON M962 40 **Well Location** Address of Well Location (Street Number/Name) Township 4786 BUDLEICH
City/Town/Village
LAKEFIECD County/District/Municipality Province Postal Code PETERBORO Ontario UTM Coordinates | Zone | Easting Municipal Plan and Sublot Number Other NAD 8 3 1 7 7 3 5 5 00 4841762 Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form) General Colour Most Common Material Depth (m/ft) Other Materials General Description From SAND RED WET 0 16 BLACK GRANITE HARD 95 16 GLAN ITE FRACTURED 95 100 CRANITI HARD BLACK 128 100 GRANITE WHITE HARD 128 140 CRANITE BLACK HARD 140 **Annular Space** Results of Well Yield Testing Depth Set at (m/ft) After test of well yield, water was: Type of Sealant Used Volume Placed Draw Down (Material and Type)  $(m^3/ft^3)$ Clear and sand free Time Water Level Time Water Level Other, specify (min) 14 7 (m/ft) (min) (m/ft) 3EM TONITE Statio If pumping discontinued, give reason: 8.6 Level 67.8 11.2 1 Pump intake set at (m/ft) 13.6 2 155 15.6 3 Pumping rate (Ilmin | GPM) **Method of Construction** Well Use Cable Tool
Rotary (Conventional) ☐ Diamond Public □ Commercial 17.7 ☐ Not used ☐ Jetting Duration of pumping Domestic Municipal Dewatering min 5 ☐ Driving / hrs + 19.7 5 Rotary (Reverse) Livestock Test Hole ■ Monitoring Boring □ Digging ☐ Irrigation ☐ Cooling & Air Conditioning Final water level end of pumping (m/ft) 10 10 Air percussion ☐ Industrial 68 Other, specify Other, specify 35.1 If flowing give rate (Ilmin / GPM) Construction Record - Casing Status of Well 20 20 Open Hole OR Material Wall Thickness Inside Depth (m/ft) Water Supply Recommended pump depth (m/ft) Diameter (cm/in) (Galvanized, Fibreglass, Concrete, Plastic, Steel) Replacement Well 15. From To 25 25 (cm/in) Test Hole Recommended pump rate Recommend (Ilmin / GPM) 20 Recharge Well 30 188 50 79 Dewatering Well 40 40 20 160 Observation and/or Well production (Ilmin | GPM) Monitoring Hole 16.7 Alteration 50 50 (Construction) Abandoned, Insufficient Supply 13. Yes No 60 Construction Record - Screen Map of Well Location Abandoned, Poor Outside Please provide a map below following instructions on the back. Depth (m/ft) Water Quality Material Diamete Slot No. (Plastic, Galvanized, Steel) Abandoned, other, (cm/in) NORTHYS specify BAY RR Other, specify CHEBOUTEQUIN RD Water Details **Hole Diameter** Water found at Depth Kind of Water: Fresh Luntested Diameter From (cm/in) 120 (m/ft) Gas Other, specify 8 3/4 20 Water found at Depth Kind of Water: Fresh Untested (m/ft) Gas Other, specify Water found at Depth Kind of Water: Fresh Untested (m/ft) Gas Other, specify SWELL Well Contractor and Well Technician Information Well Contractor's Licence No. WENSLEY WATER WELL LTD 6 5 STONEY LAKE Business Address (Street Number/Name) Municipality Comments: LAKEFIELD Business E-mail Address Province Postal Code KOL2HO Well owner's information Date Package Delivered Ministry Use Only Bus.Telephone No. (inc. area code) Name of Well Technician (Last Name, First Name) Audit No. package delivered 70156521629 ERIC WENSLEY z 178443 Date Work Completed Yes Well Technician's Licence No. Signature of Technician and/or Contractor Date Submitted 20149616 No 29140630 0506E (2007/12) © Queen's Printer for Ontario, 2007 Well Owner's Copy

# **APPENDIX E**

Pumping Test Data



**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

ocation: Burleigh Pumping Test: TW-1 Prelim.		Pumping well: TW-1	
Test conducted by: DM/MC	Test date: 5/6/2013		
Analysis performed by: DM/BK	TW-1 - Time-Drawdown	Analysis date: 5/12/2014	
Aguifer Thickness:	Discharge: variable, average rate 0.5272 [l/s]		





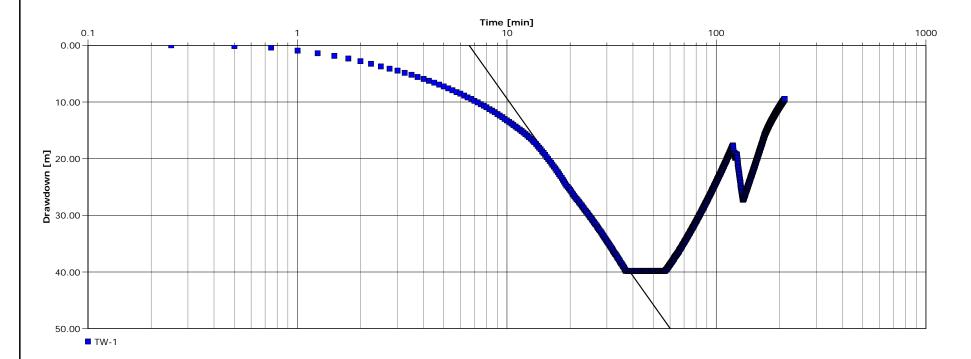
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-1 Prelim.	Pumping well: TW-1		
Test conducted by: DM/MC	Test date: 5/6/2013			
Analysis performed by: DM/BK Cooper-Jacob I - pumped well only		Analysis date: 4/24/2015		
Aquifer Thickness:	Discharge: variable, average rate 0.5272 [l/s]			



Observation well	Transmissivity	Storage coefficient	Radial distance to PW
	[m²/d]		[m]
™-1	1.60 × 10 <sup>-1</sup>		0.08



**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Location: Burleigh	Pumping Test: TW-2 Prelim.	Pumping well: TW-2	
Test conducted by: DM/MC	Test date: 6/12/2013		
Analysis performed by: DM/BK	TW-2, Time-Drawdown pumped well	Analysis date: 5/12/2014	
Aquifer Thickness:	Discharge: variable, average rate 0.126 [l/s]		





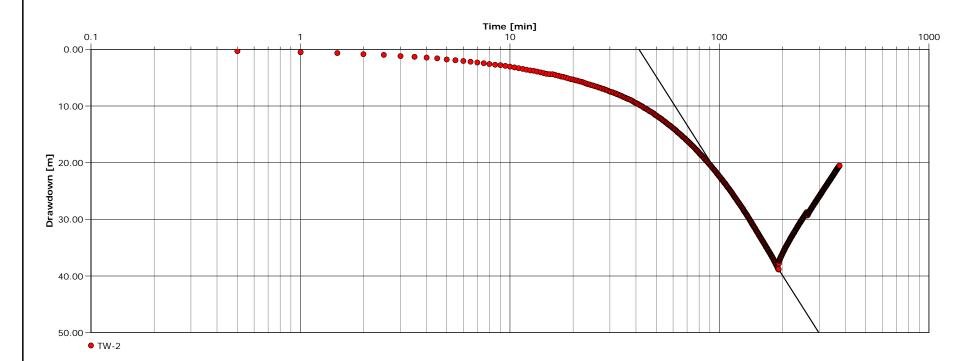
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-2 Prelim.	Pumping well: TW-2	
Test conducted by: DM/MC		Test date: 6/12/2013	
Analysis performed by: DM/BK	TW-2, Cooper-Jacob, pumped well	Analysis date: 4/24/2015	
Aquifer Thickness:	Discharge: variable, average rate 0.126 [l/s]		



Observation well	Transmissivity	Storage coefficient	Radial distance to PW
	[m²/d]		[m]
TW-2	3.42 × 10 <sup>-2</sup>	3.45 × 10 <sup>-1</sup>	0.08

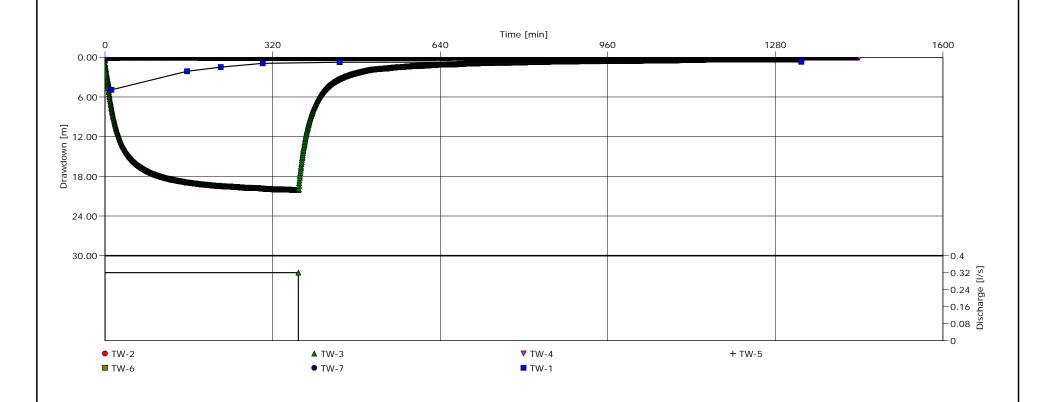


**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Location: Burleigh	Pumping Test: TW-3 (6hr)	Pumping well: TW-3		
Test conducted by: DM/BP		Test date: 7/17/2014		
Analysis performed by: DM/BK	TW-3 Time - Drawdown	Analysis date: 4/22/2015		
Aguifer Thickness: 91.99 m	Discharge: variable, average rate 0.32 [l/s]	Discharge: variable, average rate 0.32 [l/s]		





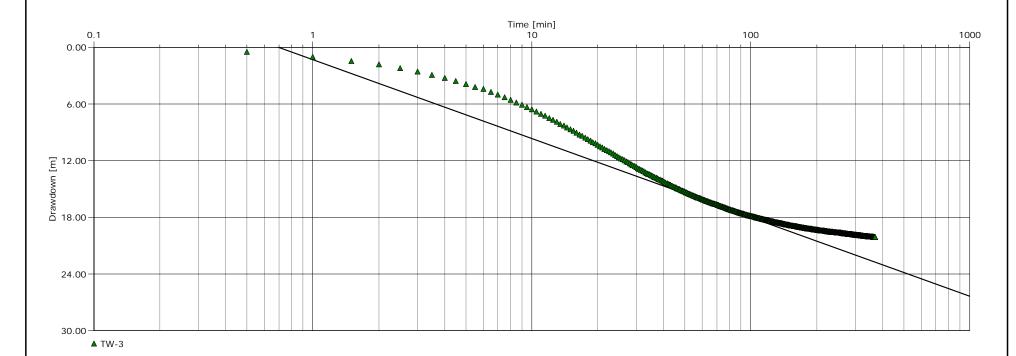
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-3 (6hr)	Pumping well: TW-3	
Test conducted by: DM/BP		Test date: 7/17/2014	
Analysis performed by: DM/BK	TW-3 Cooper-Jacob Drawdown pumped well	Analysis date: 4/22/2015	
Aquifer Thickness: 91.99 m	Discharge: variable, average rate 0.32 [l/s]		



Observation well	Transmissivity	Hydraulic Conductivity	Storage coefficient	Radial distance to PW	
	[m²/d]	[m/d]		[m]	
TW-3	6.06 × 10 <sup>-1</sup>	6.59 × 10 <sup>-3</sup>		0.08	



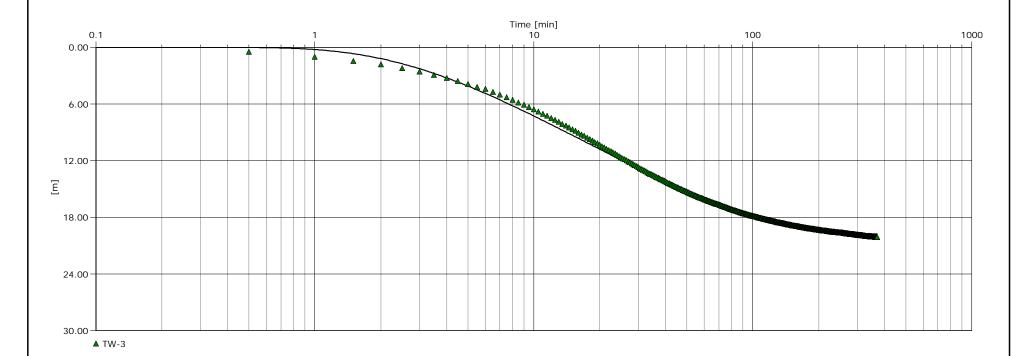
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-3 (6hr)	Pumping well: TW-3		
Test conducted by: DM/BP		Test date: 7/17/2014		
Analysis performed by: DM/BK	TW-3 Hantush - pumped well	Analysis date: 4/22/2015		
Aquifer Thickness: 91.99 m	Discharge: variable, average rate 0.32 [l/s]			



#### Calculation after Hantush

Observation well	Transmissivity	Hydraulic Conductivity	Storage coefficient	Hydr. resistance	Radial distance to PW	
	[m²/d]	[m/d]		[min]	[m]	
TW-3	3.30 × 10 <sup>-1</sup>	3.59 × 10 <sup>-3</sup>		4.00 × 10 <sup>2</sup>	0.08	



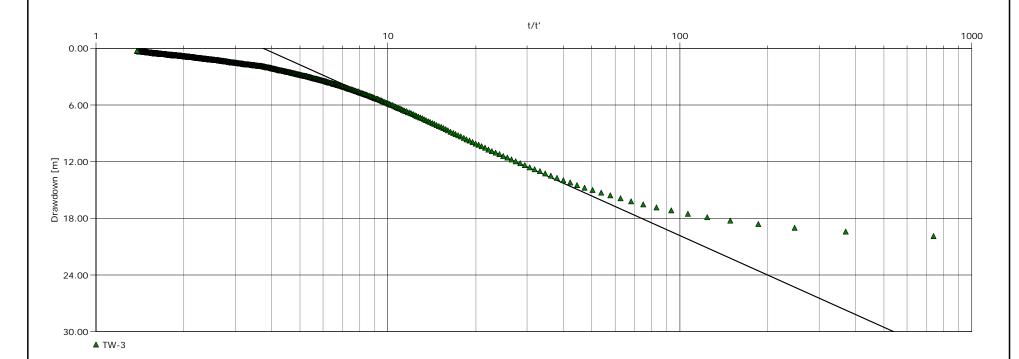
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-3 (6hr)	Pumping well: TW-3		
Test conducted by: DM/BP		Test date: 7/17/2014		
Analysis performed by: DM/BK	TW-3 Theis Recovery Analysis date: 4/22/2015			
Aquifer Thickness: 91.99 m	Discharge: variable, average rate 0.32 [l/s]			



#### Calculation after Theis & Jacob

Observation well	Transmissivity	Hydraulic Conductivity	Radial distance to PW
	[m²/d]	[m/d]	[m]
TW-3	3.64 × 10 <sup>-1</sup>	3.96 × 10 <sup>-3</sup>	0.08

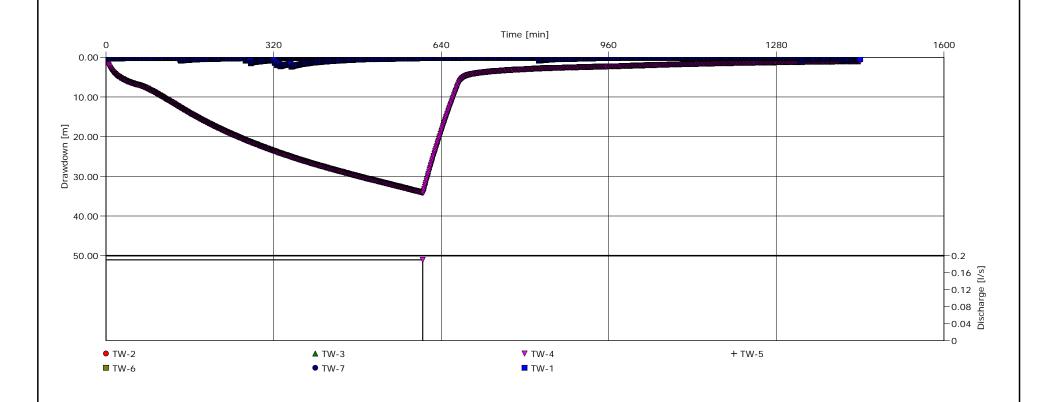


**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Location: Burleigh	Pumping Test: TW-4(10hr)	Pumping well: TW-4		
Test conducted by: DM/BP		Test date: 12/2/2014		
Analysis performed by: DM/BK	TW-4 Time - Drawdown all wells	TW-4 Time - Drawdown all wells Analysis date: 4/22/2015		
Aguifer Thickness: 89 47 m Discharge: variable, average rate 0.19 [l/s]				





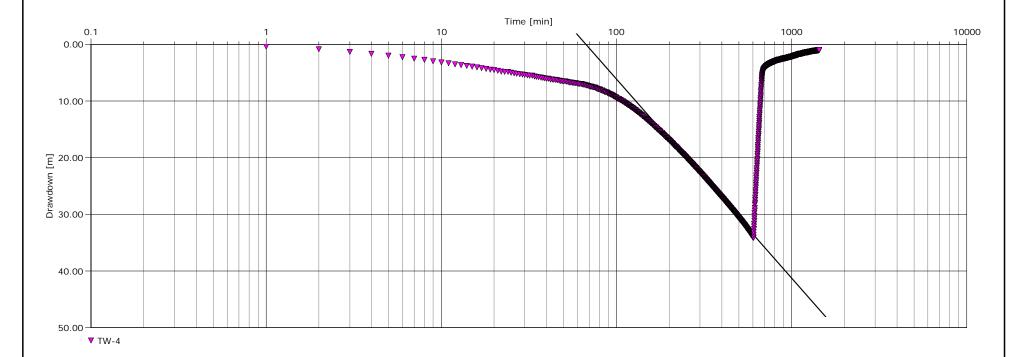
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-4(10hr)	Pumping well: TW-4		
Test conducted by: DM/BP		Test date: 12/2/2014		
Analysis performed by: DM/BK	TW-4 Cooper- Jacob I Analysis date: 4/24/2015			
Aquifer Thickness: 89.47 m	Discharge: variable, average rate 0.19 [l/s]			



ſ	Observation well	Transmissivity	Hydraulic Conductivity	Storage coefficient	Radial distance to PW	
		[m²/d]	[m/d]		[m]	
	TW-4	8.56 × 10 <sup>-2</sup>	9.56 × 10 <sup>-4</sup>		0.08	



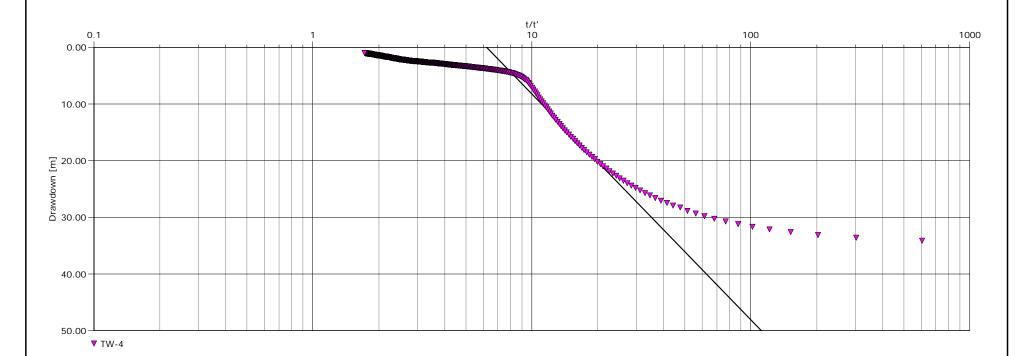
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-4(10hr)	Pumping well: TW-4			
Test conducted by: DM/BP	Test date: 12/2/2014				
Analysis performed by: DM/BK TW-4 Theis Recovery pumped well		Analysis date: 4/24/2015			
Aquifer Thickness: 89.47 m	Discharge: variable, average rate 0.19 [l/s]				



### Calculation after Theis & Jacob

Observation well	Transmissivity	Hydraulic Conductivity	Radial distance to PW
	[m²/d]	[m/d]	[m]
TW-4	7.54 × 10 <sup>-2</sup>	8.43 × 10 <sup>-4</sup>	0.08

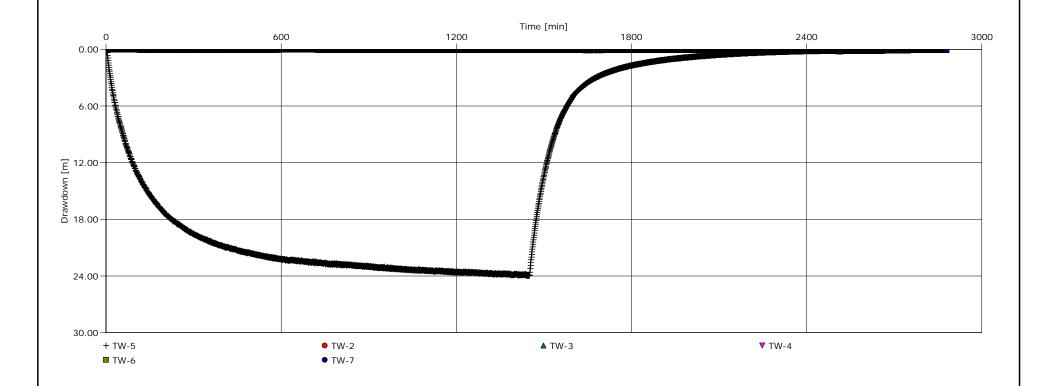


**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Location: Burleigh Pumping Test: TW-5 (24hr)		Pumping well: TW-5	
Test conducted by: DM/BP	Test date: 2/19/2015		
Analysis performed by: DM/BK TW-5 Time - Drawdown all wells		Analysis date: 4/22/2015	
Aquifer Thickness: 79.55 m	Discharge: variable, average rate 0.09 [l/s]		





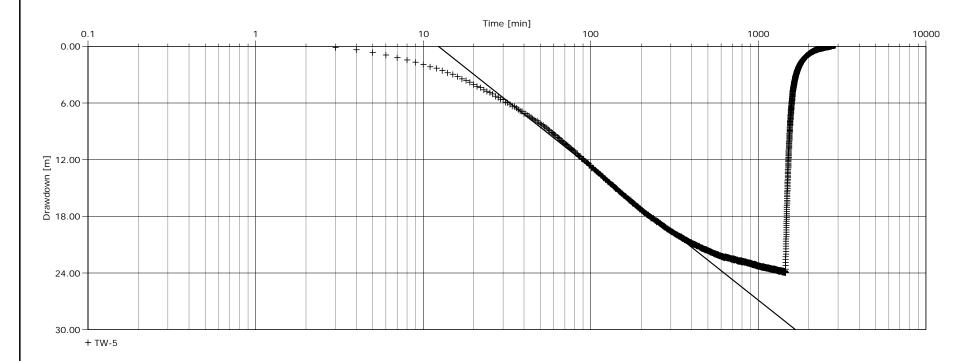
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-5 (24hr)	Pumping well: TW-5				
Test conducted by: DM/BP	Test date: 2/19/2015					
Analysis performed by: DM/BK TW-5 Cooper - Jacob 1 pumped well		Analysis date: 4/24/2015				
Aquifer Thickness: 79.55 m	Discharge: variable, average rate 0.09 [l/s]					



Observation well	Transmissivity	Hydraulic Conductivity	Storage coefficient	Radial distance to PW	
	[m²/d]	[m/d]		[m]	
TW-5	1.01 × 10 <sup>-1</sup>	1.27 × 10 <sup>-3</sup>	3.04 × 10 <sup>-1</sup>	0.08	



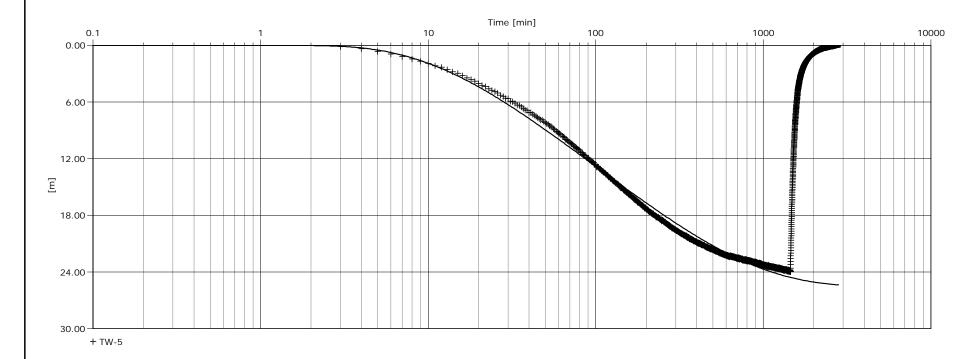
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-5 (24hr)	Pumping well: TW-5	
Test conducted by: DM/BP	Test date: 2/19/2015		
Analysis performed by: DM/BK TW-5 Hantush pumped well		Analysis date: 4/24/2015	
Aquifer Thickness: 79.55 m	Discharge: variable, average rate 0.09 [l/s]		



### Calculation after Hantush

Observation well	Transmissivity	Hydraulic Conductivity	Storage coefficient	Hydr. resistance	Radial distance to PW	
	[m²/d]	[m/d]		[min]	[m]	
TW-5	9.20 × 10 <sup>-2</sup>	1.16 × 10 <sup>-3</sup>		3.40 × 10 <sup>3</sup>	0.08	



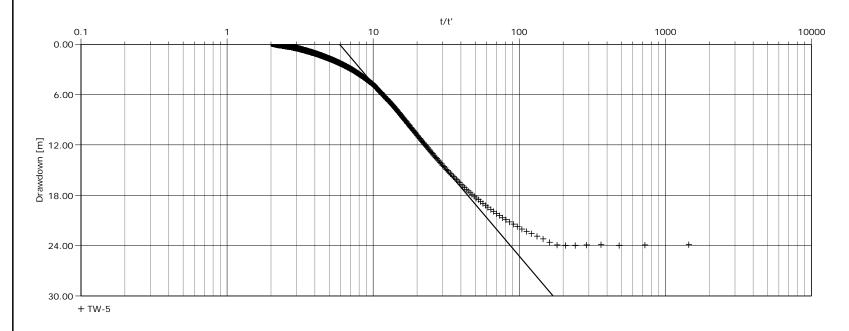
Pumping Test Analysis	Report
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Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-5 (24hr)	Pumping well: TW-5				
Test conducted by: DM/BP	Test date: 2/19/2015					
Analysis performed by: DM/BK TW-5 Theis Recovery pumped well		Analysis date: 4/24/2015				
Aquifer Thickness: 79.55 m	Discharge: variable, average rate 0.09 [l/s]					



### Calculation after Theis & Jacob

Observation well	Transmissivity	Hydraulic Conductivity	Radial distance to PW
	[m²/d]	[m/d]	[m]
TW-5	6.93 × 10 <sup>-2</sup>	8.71 × 10 <sup>-4</sup>	0.08

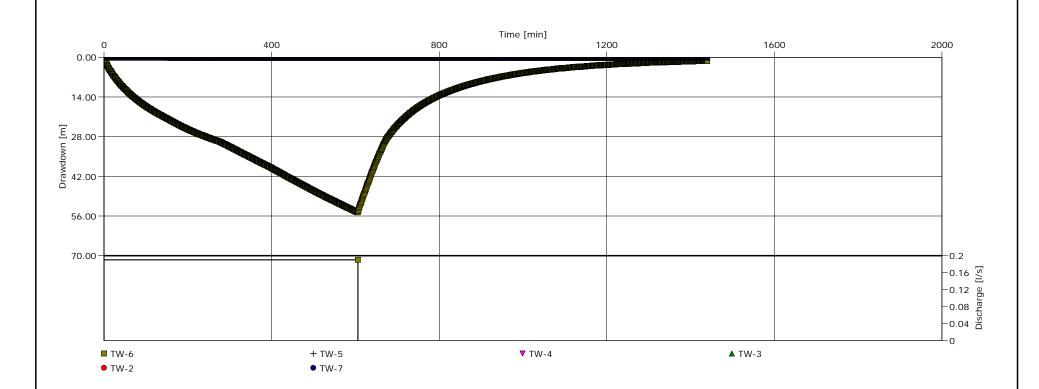


**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Location: Burleigh Pumping Test: TW-6 (10hr)		Pumping well: TW-6		
Test conducted by: DM/BP	Test date: 9/3/2014			
Analysis performed by: DM/BK TW-6 Time - Drawdown all wells		Analysis date: 9/10/2014		
Aquifer Thickness: 97.96 m	Discharge: variable, average rate 0.19 [l/s]	Discharge: variable, average rate 0.19 [l/s]		





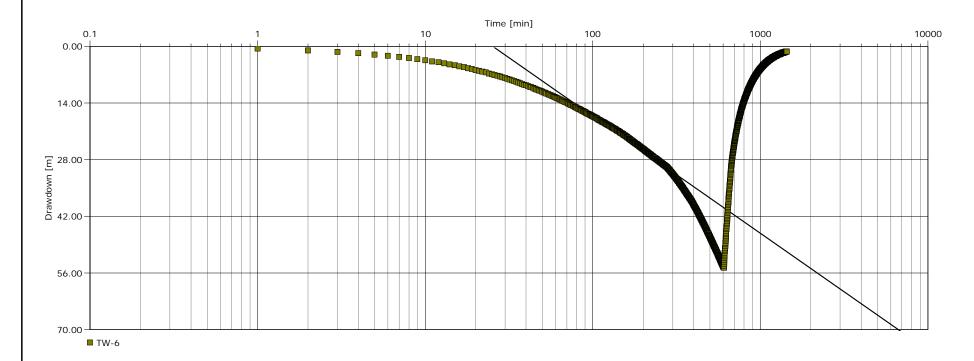
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-6 (10hr)	Pumping well: TW-6	
Test conducted by: DM/BP	Test date: 9/3/2014		
Analysis performed by: DM/BK TW-6 Cooper & Jacob 1 pumped well		Analysis date: 4/24/2015	
Aquifer Thickness: 97.96 m	Discharge: variable, average rate 0.19 [l/s]		



Observation well	Transmissivity	Hydraulic Conductivity	Storage coefficient	Radial distance to PW
	[m²/d]	[m/d]		[m]
TW-6	1.04 × 10 <sup>-1</sup>	1.06 × 10 <sup>-3</sup>		0.08



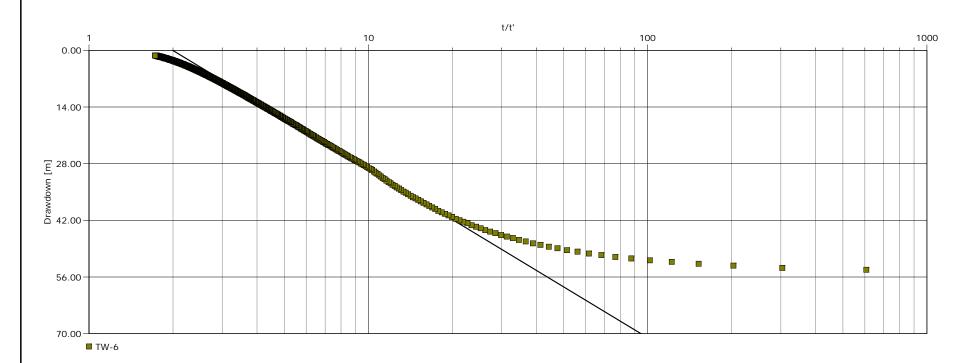
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-6 (10hr)	Pumping well: TW-6
Test conducted by: DM/BP		Test date: 9/3/2014
Analysis performed by: DM/BK	TW-6 Theis Recovery pumped well	Analysis date: 4/24/2015
Aquifer Thickness: 97.96 m	Discharge: variable, average rate 0.19 [l/s]	



### Calculation after Theis & Jacob

Observation well	Transmissivity	Hydraulic Conductivity	Radial distance to PW	
	[m²/d]	[m/d]	[m]	
TW-6	7.19 × 10 <sup>-2</sup>	7.34 × 10 <sup>-4</sup>	0.08	

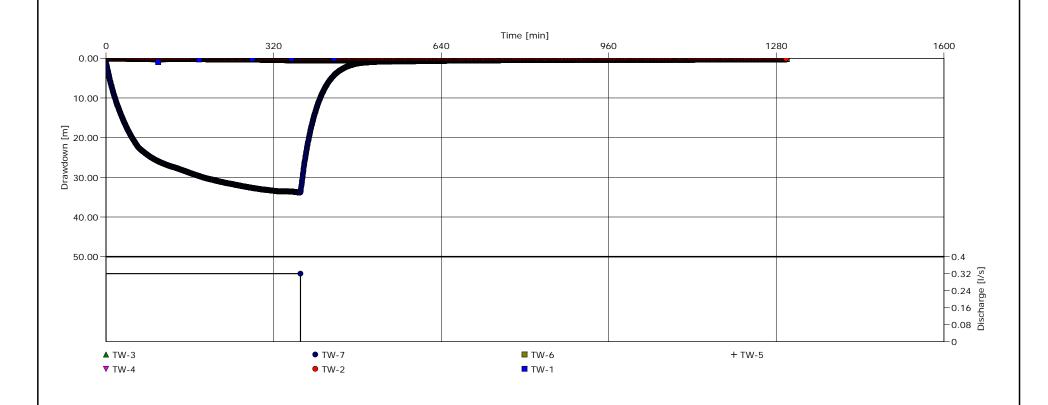


**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Location: Burleigh	Pumping Test: TW-7 (6hr)	Pumping well: TW-7
Test conducted by: DM/BP		Test date: 7/16/2014
Analysis performed by: DM/BK	TW-7 Time - Drawdown all wells	Analysis date: 4/21/2015
Aguifer Thickness: 52.76 m	Discharge rate: 0.32 [l/s]	





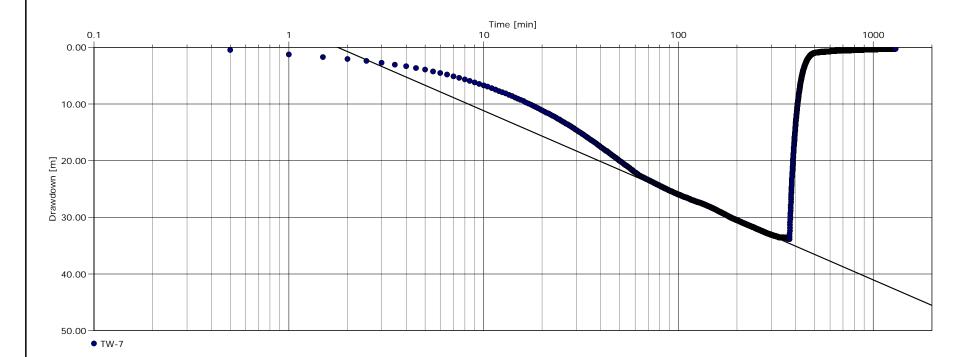
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-7 (6hr)	Pumping well: TW-7
Test conducted by: DM/BP		Test date: 7/16/2014
Analysis performed by: DM/BK	TW-7 Cooper- Jacob 1 pumped well	Analysis date: 4/24/2015
Aquifer Thickness: 52.76 m	Discharge rate: 0.32 [l/s]	



Observation well	Transmissivity	Hydraulic Conductivity	Storage coefficient	Radial distance to PW	
	[m²/d]	[m/d]		[m]	
TW-7	3.39 × 10 <sup>-1</sup>	6.43 × 10 <sup>-3</sup>		0.08	



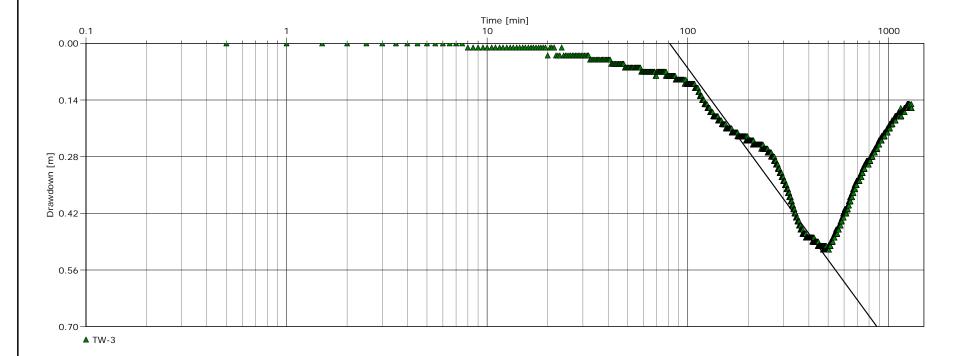
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-7 (6hr)	Pumping well: TW-7
Test conducted by: DM/BP		Test date: 7/16/2014
Analysis performed by: DM/BK	TW-7 (TW-3 obs) Cooper - Jacob	Analysis date: 4/24/2015
Aquifer Thickness: 52.76 m	Discharge rate: 0.32 [l/s]	



Observation well	Transmissivity	Hydraulic Conductivity	Storage coefficient	Radial distance to PW
	[m²/d]	[m/d]		[m]
TW-3	$7.47 \times 10^{0}$	1.42 × 10 <sup>-1</sup>	4.61 × 10 <sup>-5</sup>	143.13



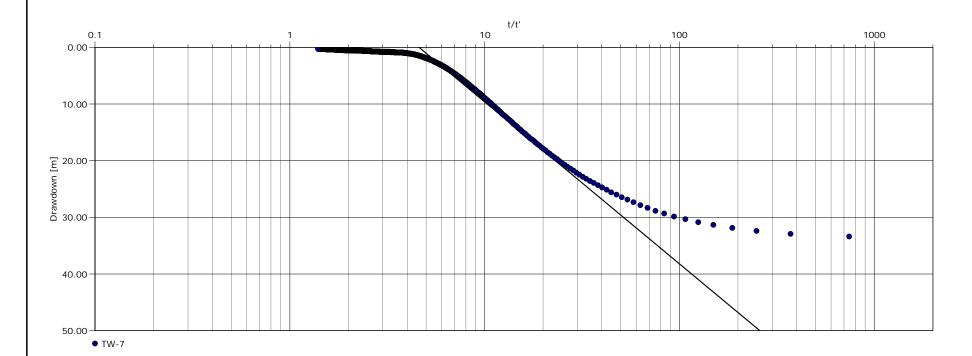
**Pumping Test Analysis Report** 

Project: Pilgrim's Rest

Number: 12-1629

Client: Pieter Venema

Location: Burleigh	Pumping Test: TW-7 (6hr)	Pumping well: TW-7
Test conducted by: DM/BP		Test date: 7/16/2014
Analysis performed by: DM/BK	TW-7 Theis Recovery pumped well	Analysis date: 4/24/2015
Aquifer Thickness: 52.76 m	Discharge rate: 0.32 [l/s]	



### Calculation after Theis & Jacob

Observation well	Transmissivity	Hydraulic Conductivity	Radial distance to PW
	[m²/d]	[m/d]	[m]
TW-7	1.77 × 10 <sup>-1</sup>	3.36 × 10 <sup>-3</sup>	0.08

# APPENDIX F

Water Quality Data

		TW-3 (4hrs)	TW-3 (6HRS)	TW-4 (5HRS)	TW-4 (10 hrs.)	TW-4	TW-5 (6 hrs.)	TW-5 (12 hrs.)	TW-5 (18 hrs.)	TW-5 (24 hrs.)	TW-6 (6hrs)	TW-6 (12 hrs.)	TW-6 (18 hrs.)
Parameter	Units (a)	17-Jul-14	17-Jul-14	2-Dec-14	2-Dec-14	7-May-15	19-Feb-15	19-Feb-15	19-Feb-15	19-Feb-15	26-Nov-14	26-Nov-14	26-Nov-14
Hardness (CaCO3)		194	194	181	159		75	59	57	58	30	20	18
Alkalinity (CaCO3) to pH 4.5		161	158	145	143		168	162	158	157	154	151	145
Bicarbonate (as CaCO3)		161	158	145	143		168	162	158	157	121	119	117
Carbonate (as CaCO3)		< 5	< 5	< 5	< 3		<5	<5	<5	<5	33	32	29
Conductivity @ 25°C	μmho/cm	380	373	338	336		339	339	339	338	329	325	329
pH @ 25°C	pH Units	7.92	7.94	7.74	7.8		7.76	8.01	8.05	8.07	9.03	9.01	9
Colour	TCU	3	4	19	< 2		13	27	19	17	13	9	10
Turbidity	NTU	11.3	25.2	134	75.2		13.8	2.5	2.1	1.3	19.5	6.2	5.6
Turbidity (field)	NTU	5.96	2.41*	4.11	3.85	2.04*	-	2.49	1.39	1.19*	20.8	9.31	4.58
Chloride		4.2	3.2	4.4	5.4		3.2	3.6	3.7	3.6	3.9	3.2	3.3
Fluoride		0.2	0.2	0.3	0.3		0.7	0.9	1	0.9	1.8	2.1	2.1
Nitrite (N)		< 0.1	< 0.1	< 0.1	< 0.1		<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Nitrate (N)		< 0.1	< 0.1	< 0.1	< 0.1		<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1
Sulphate		25	25	29	32		19	20	20	20	20	21	24
Calcium		63.4	63.3	63.3	55		22.6	17.3	16.7	16.8	4.92	3.5	3.27
Magnesium		8.73	8.76	5.64	5.16		4.54	3.7	3.7	3.8	4.32	2.83	2.37
Sodium		7	6	8.5	11.5		50.8	57.8	58.7	58.3	78.5	80	78.9
Potassium		1.7	1.7	2.6	2.3		1.9	1.7	1.7	1.7	1	0.8	0.8
Iron		3.48	3.77	9.98	7.79		2.19	0.355	0.177	0.129	2.03	1.19	0.949
Copper		< 0.002	< 0.002	< 0.002	< 0.002		<0.002	<0.002	<0.002	<0.002	<0.002	<0.002	<0.002
Manganese		0.529	0.522	0.693	0.596		0.232	0.119	0.096	0.087	0.055	0.025	0.02
Zinc		< 0.005	< 0.005	0.018	0.012		<0.005	<0.005	<0.005	<0.005	<0.005	<0.005	<0.005
Ammonia (N)-Total		0.04	0.05	< 0.01	0.04		0.05	0.05	0.05	0.05	< 0.01	<0.01	<0.01
o-Phosphate (P)		< 0.01	< 0.01	< 0.01	0.02		<0.01	<0.01	<0.01	<0.01	< 0.01	<0.01	<0.01
Dissolved Organic Carbon		2.4	2.6	4.6	22.9		4	3.6	3.6	3.5	2.5	2	1.8
Total Coliform	cfu/100mL	0	0	0	12			0		0		(d)	
E coli	cfu/100mL	0	0	0	0			0		0		(d)	
Heterotrophic Plate Count	cfu/mL	118	< 2	46	90			112		34		>500	
TDS (ion sum calc.)		210	207	211	226		205	203	201	200	209	205	203

- (a) All units are in mg/L unless specified otherwise
- (b) When both nitrate and nitrite are present, the sum of both should not exceed 10 mg/L
- (c) Yellow highlighted values indicate that sodium levels exceed the 20 mg/L warning level for sodium restricted diets
- (d) No result due to sample matrix interference
- (e) Turbidity limit for wells requiring disinfection due to bacteria issues is 1 ntu, otherwise the aesthetic limit is 5 ntu
- \*Stabilized in 120 mL bottle containing 0.5 mL of 50% HNO <sub>3</sub> solution to prevent the formation of iron precipitates

		TW-6 (24 hrs.)	TW-6 (5 hrs.)	TW-6 (10 hrs.)	TW-7 (3 hrs.)	TW-7 (6 hrs.)	ODWQS
Parameter	Units (a)	26-Nov-14	5-Feb-15	5-Feb-15	16-Jul-14	16-Jul-14	ODWQS
Hardness (CaCO3)		16	12	13	170	168	80-100
Alkalinity (CaCO3) to pH 4.5		145	147	145	144	145	30-500
Bicarbonate (as CaCO3)		119	147	145	144	145	-
Carbonate (as CaCO3)		26	26	18	<5	<5	-
Conductivity @ 25°C	μmho/cm	325	315	329	332	330	-
pH @ 25°C	pH Units	8.99	8.85	8.78	7.78	7.74	6.5-8.5
Colour	TCU	9	6	7	7	6	5
Turbidity	NTU	4.5	3.2	2.8	77.6	60.9	5 (1) <sup>(e)</sup>
Turbidity (field)	NTU	2.18*	2.46	1.84	2.65	0.89	5 (1) <sup>(e)</sup>
Chloride		3.2	1.7	2.1	4	4.1	250
Fluoride		2.2	1.9	2	0.3	0.3	1.5
Nitrite (N)		<0.1	<0.1	<0.1	<0.1	<0.1	1(b)
Nitrate (N)		<0.1	<0.1	<0.1	<0.1	<0.1	10 (b)
Sulphate		24	18	24	22	21	500
Calcium		3.07	3.4	3.3	58.2	57.4	-
Magnesium		1.94	0.97	1.06	5.93	5.92	-
Sodium		78	69.3	71.9	5.3	5.9	200
Potassium		0.8	0.9	0.8	1.8	1.8	-
Iron		0.703	0.248	0.266	5.82	5.37	0.3
Copper		<0.002	<0.002	<0.002	<0.002	<0.002	1
Manganese		0.015	0.009	0.009	0.494	0.469	0.05
Zinc		<0.005	<0.005	<0.005	<0.005	<0.005	5
Ammonia (N)-Total		< 0.01	0.04	0.03	0.03	0.03	-
o-Phosphate (P)		<0.01	<0.01	<0.01	<0.01	<0.01	-
Dissolved Organic Carbon		2	1.3	1.3	3.2	2.9	5
Total Coliform	cfu/100mL	(d)	0	0		0	0
E coli	cfu/100mL	(d)	0	0		0	0
Heterotrophic Plate Count	cfu/mL	>500	358	142		2	<u>-</u>
TDS (ion sum calc.)		201	184	192	190	189	500



Final Report

C.O.C.: G34505 REPORT No. B14-17749

Report To:

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

**Attention:** Dan MacIntyre

DATE RECEIVED: 17-Jul-14
DATE REPORTED: 22-Jul-14

SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

2378 Holly Lane

Ottawa Ontario K1V 7P1 Tel: 613-526-0123

Fax: 613-526-1244

JOB/PROJECT NO.: Pilgrims Rest

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-7 (3hrs)	TW-7 (6hrs)	
			Sample I.D.		B14-17749-1	B14-17749-2	
			Date Collecte	ed	16-Jul-14	16-Jul-14	
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Hardness (as CaCO3)	mg/L	1	SM 3120	18-Jul-14/O	170	168	
Alkalinity(CaCO3) to pH4.5	mg/L	5	SM 2320B	18-Jul-14/O	144	145	
Carbonate (as CaCO3)	mg/L	5	SM 2320B	18-Jul-14/O	< 5	< 5	
Bicarbonate(as CaCO3)	mg/L	5	SM 2320B	18-Jul-14/O	144	145	
Conductivity @25°C	µmho/cm	1	SM 2510B	18-Jul-14/O	332	330	
pH @25°C	pH Units		SM 4500H	17-Jul-14/O	7.78	7.74	
Colour	TCU	2	SM 2120C	22-Jul-14/O	7	6	
Turbidity	NTU	0.1	SM 2130	22-Jul-14/O	77.6	60.9	
Fluoride	mg/L	0.1	SM4110C	18-Jul-14/O	0.3	0.3	
Chloride	mg/L	0.5	SM4110C	18-Jul-14/O	4.0	4.1	
Nitrite (N)	mg/L	0.1	SM4110C	18-Jul-14/O	< 0.1	< 0.1	
Nitrate (N)	mg/L	0.1	SM4110C	18-Jul-14/O	< 0.1	< 0.1	
Sulphate	mg/L	1	SM4110C	18-Jul-14/O	22	21	
Calcium	mg/L	0.02	SM 3120	18-Jul-14/O	58.2	57.4	
Magnesium	mg/L	0.01	SM 3120	18-Jul-14/O	5.93	5.92	
Sodium	mg/L	0.2	SM 3120	18-Jul-14/O	5.3	5.9	
Potassium	mg/L	0.1	SM 3120	18-Jul-14/O	1.8	1.8	
Copper	mg/L	0.002	SM 3120	18-Jul-14/O	< 0.002	< 0.002	
Iron	mg/L	0.005	SM 3120	18-Jul-14/O	5.82	5.37	
Manganese	mg/L	0.001	SM 3120	18-Jul-14/O	0.494	0.469	
Zinc	mg/L	0.005	SM 3120	18-Jul-14/O	< 0.005	< 0.005	
Ammonia (N)-Total	mg/L	0.01	MOEE 3364	17-Jul-14/O	0.03	0.03	
o-Phosphate (P)	mg/L	0.01	MOEE 3366	17-Jul-14/O	< 0.01	< 0.01	
Dissolved Organic Carbon	mg/L	0.2	EPA 415.1	21-Jul-14/O	3.2	2.9	
Total Coliform	cfu/100mL	1	MOE E3407	17-Jul-14/O		0	
E coli	cfu/100mL	1	MOE E3407	17-Jul-14/O		0	
Heterotrophic Plate Count	cfu/mL	2	SM 9215C	17-Jul-14/O		2	
Anion Sum	meq/L		Calc.	21-Jul-14/O	3.46	3.46	

M.D.L. = Method Detection Limit

Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill

Krystyna Pipin , M. Sc.



Final Report

C.O.C.: G34505 REPORT No. B14-17749

Report To:

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

Attention: Dan MacIntyre

DATE RECEIVED: 17-Jul-14
DATE REPORTED: 22-Jul-14

SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

2378 Holly Lane

Ottawa Ontario K1V 7P1 Tel: 613-526-0123

Fax: 613-526-1244

JOB/PROJECT NO.: Pilgrims Rest

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-7 (3hrs)	TW-7 (6hrs)	
			Sample I.D.		B14-17749-1	B14-17749-2	
				ed	16-Jul-14	16-Jul-14	
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Cation Sum	meq/L		Calc.	21-Jul-14/O	4.01	3.97	
% Difference	%		Calc.	21-Jul-14/O	7.41	6.78	
Ion Ratio	AS/CS		Calc.	21-Jul-14/O	0.862	0.873	
Sodium Adsorption Ratio	-		Calc.	21-Jul-14/O	0.177	0.199	
TDS(ion sum calc.)	mg/L		Calc.	21-Jul-14/O	190	189	
Conductivity (calc.)	µmho/cm		Calc.	21-Jul-14/O	343	343	
TDS(calc.)/EC(actual)	-		Calc.	21-Jul-14/O	0.572	0.574	
EC(calc.)/EC(actual)	-		Calc.	21-Jul-14/O	1.03	1.04	
Langelier Index(25°C)	S.I.		Calc.	21-Jul-14/O	0.281	0.237	

Krystyna Pipin , M. Sc.



Final Report

C.O.C.: G31356 REPORT No. B14-17857

Rev. 1

Report To:

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

Attention: Dan MacIntyre

DATE RECEIVED: 18-Jul-14
DATE REPORTED: 24-Jul-14

SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

2378 Holly Lane

Ottawa Ontario K1V 7P1 Tel: 613-526-0123

Fax: 613-526-1244

JOB/PROJECT NO.: Pilgrims Rest

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-3 - (4hrs)	TW-3 - (6hrs)	
			Sample I.D.		B14-17857-1	B14-17857-2	
			Date Collecte	ed	17-Jul-14	17-Jul-14	
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Hardness (as CaCO3)	mg/L	1	SM 3120	21-Jul-14/O	194	194	
Alkalinity(CaCO3) to pH4.5	mg/L	5	SM 2320B	18-Jul-14/O	161	158	
Carbonate (as CaCO3)	mg/L	5	SM 2320B	18-Jul-14/O	< 5	< 5	
Bicarbonate(as CaCO3)	mg/L	5	SM 2320B	18-Jul-14/O	161	158	
Conductivity @25°C	µmho/cm	1	SM 2510B	18-Jul-14/O	380	373	
pH @25°C	pH Units		SM 4500H	18-Jul-14/O	7.92	7.94	
Colour	TCU	2	SM 2120C	22-Jul-14/O	3	4	
Turbidity	NTU	0.1	SM 2130	22-Jul-14/O	11.3	25.2	
Fluoride	mg/L	0.1	SM4110C	21-Jul-14/O	0.2	0.2	
Chloride	mg/L	0.5	SM4110C	21-Jul-14/O	4.2	3.2	
Nitrite (N)	mg/L	0.1	SM4110C	21-Jul-14/O	< 0.1	< 0.1	
Nitrate (N)	mg/L	0.1	SM4110C	21-Jul-14/O	< 0.1	< 0.1	
Sulphate	mg/L	1	SM4110C	21-Jul-14/O	25	25	
Calcium	mg/L	0.02	SM 3120	21-Jul-14/O	63.4	63.3	
Magnesium	mg/L	0.01	SM 3120	21-Jul-14/O	8.73	8.76	
Sodium	mg/L	0.2	SM 3120	21-Jul-14/O	7.0	6.0	
Potassium	mg/L	0.1	SM 3120	21-Jul-14/O	1.7	1.7	
Copper	mg/L	0.002	SM 3120	21-Jul-14/O	< 0.002	< 0.002	
Iron	mg/L	0.005	SM 3120	21-Jul-14/O	3.48	3.77	
Manganese	mg/L	0.001	SM 3120	21-Jul-14/O	0.529	0.522	
Zinc	mg/L	0.005	SM 3120	21-Jul-14/O	< 0.005	< 0.005	
Ammonia (N)-Total	mg/L	0.01	MOEE 3364	21-Jul-14/O	0.04	0.05	
o-Phosphate (P)	mg/L	0.01	MOEE 3366	21-Jul-14/O	< 0.01	< 0.01	
Dissolved Organic Carbon	mg/L	0.2	EPA 415.1	23-Jul-14/O	2.4	2.6	
Total Coliform	cfu/100mL	1	MOE E3407	18-Jul-14/O	0	0	
E coli	cfu/100mL	1	MOE E3407	18-Jul-14/O	0	0	
Heterotrophic Plate Count	cfu/mL	2	SM 9215C	18-Jul-14/O	118	< 2	
Anion Sum	meq/L		Calc.	22-Jul-14/O	3.85	3.77	

NOTE: Revision created to add DOC upon client request

Krystyna Pipin , M. Sc.

Lab Supervisor

M.D.L. = Method Detection Limit

Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill



**Caduceon Environmental Laboratories** 

Final Report

C.O.C.: G31356 REPORT No. B14-17857

Rev. 1

Report To:

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

Attention: Dan MacIntyre

DATE RECEIVED: 18-Jul-14

DATE REPORTED: 24-Jul-14
SAMPLE MATRIX: Groundwater

2378 Holly Lane

Ottawa Ontario K1V 7P1 Tel: 613-526-0123

Fax: 613-526-1244

JOB/PROJECT NO.: Pilgrims Rest

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-3 - (4hrs)	TW-3 - (6hrs)	
			Sample I.D.		B14-17857-1	B14-17857-2	
			Date Collect	ed	17-Jul-14	17-Jul-14	
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Cation Sum	meq/L		Calc.	22-Jul-14/O	4.44	4.42	
% Difference	%		Calc.	22-Jul-14/O	7.10	7.84	
Ion Ratio	AS/CS		Calc.	22-Jul-14/O	0.867	0.855	
Sodium Adsorption Ratio	-		Calc.	22-Jul-14/O	0.218	0.189	
TDS(ion sum calc.)	mg/L		Calc.	22-Jul-14/O	210	207	
Conductivity (calc.)	µmho/cm		Calc.	22-Jul-14/O	387	381	
TDS(calc.)/EC(actual)	-		Calc.	22-Jul-14/O	0.554	0.555	
EC(calc.)/EC(actual)	-		Calc.	22-Jul-14/O	1.02	1.02	
Langelier Index(25°C)	S.I.		Calc.	22-Jul-14/O	0.495	0.508	

NOTE: Revision created to add DOC upon client request

M.D.L. = Method Detection Limit

Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill

Krystyna Pipin , M. Sc.



Final Report

C.O.C.: G32220 REPORT No. B14-22908

Client I.D.

Report To:

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

**Attention:** Dan MacIntyre

DATE RECEIVED: 05-Sep-14
DATE REPORTED: 12-Sep-14
SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

285 Dalton Ave

Kingston Ontario K7K 6Z1 Tel: 613-544-2001

Fax: 613-544-2770

JOB/PROJECT NO.: Piligrims

P.O. NUMBER: 12-1629

WATERWORKS NO.

TW-6 -

					(10hrs)		
			Sample I.D.		B14-22908-1		
			Date Collecte	ed	03-Sep-14		
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Alkalinity(CaCO3) to pH4.5	mg/L	5	SM 2320B	05-Sep-14/O	154		
Bicarbonate(as CaCO3)	mg/L	5	SM 2320B	05-Sep-14/O	128		
Carbonate (as CaCO3)	mg/L	5	SM 2320B	05-Sep-14/O	26		
pH @25°C	pH Units		SM 4500H	05-Sep-14/O	8.95		
Conductivity @25°C	µmho/cm	1	SM 2510B	09-Sep-14/O	339		
Colour	TCU	2	SM 2120C	08-Sep-14/O	23		
Turbidity	NTU	0.1	SM 2130	09-Sep-14/O	18.6		
Fluoride	mg/L	0.1	SM4110C	08-Sep-14/O	2.5		
Chloride	mg/L	0.5	SM4110C	08-Sep-14/O	3.9		
Nitrite (N)	mg/L	0.1	SM4110C	08-Sep-14/O	< 0.1		
Nitrate (N)	mg/L	0.1	SM4110C	08-Sep-14/O	< 0.1		
Sulphate	mg/L	1	SM4110C	08-Sep-14/O	21		
o-Phosphate (P)	mg/L	0.01	MOEE 3366	08-Sep-14/O	< 0.01		
Ammonia (N)-Total	mg/L	0.01	MOEE 3364	08-Sep-14/O	0.03		
Dissolved Organic Carbon	mg/L	0.2	EPA 415.1	09-Sep-14/O	1.5		
TDS (Calc. from Cond.)	mg/L		Calc.	09-Sep-14	228		
Hardness (as CaCO3)	mg/L	1	SM 3120	08-Sep-14/O	41		
Calcium	mg/L	0.02	SM 3120	08-Sep-14/O	4.25		
Copper	mg/L	0.002	SM 3120	08-Sep-14/O	< 0.002		
Iron	mg/L	0.005	SM 3120	08-Sep-14/O	3.62		
Magnesium	mg/L	0.01	SM 3120	08-Sep-14/O	7.43		
Manganese	mg/L	0.001	SM 3120	08-Sep-14/O	0.065		
Potassium	mg/L	0.1	SM 3120	08-Sep-14/O	0.9		
Sodium	mg/L	0.2	SM 3120	08-Sep-14/O	78.7		
Zinc	mg/L	0.005	SM 3120	08-Sep-14/O	< 0.005		
Anion Sum	meq/L		Calc.	09-Sep-14/O	3.76		
Cation Sum	meq/L		Calc.	09-Sep-14/O	4.47		

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M.D.L. = Method Detection Limit Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill



Final Report

C.O.C.: G32220 REPORT No. B14-22908

Report To:

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

**Attention:** Dan MacIntyre

DATE RECEIVED: 05-Sep-14
DATE REPORTED: 12-Sep-14

SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

285 Dalton Ave

Kingston Ontario K7K 6Z1

Tel: 613-544-2001 Fax: 613-544-2770

JOB/PROJECT NO.: Piligrims

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-6 - (10hrs)		
			Sample I.D.		B14-22908-1		
			Date Collect	ed	03-Sep-14		
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
% Difference	%		Calc.	09-Sep-14/O	8.55		
Ion Ratio	AS/CS		Calc.	09-Sep-14/O	0.842		
Sodium Adsorption Ratio	-		Calc.	09-Sep-14/O	5.33		
TDS(ion sum calc.)	mg/L		Calc.	09-Sep-14/O	215		
Conductivity (calc.)	µmho/cm		Calc.	09-Sep-14/O	366		
TDS(calc.)/EC(actual)	-		Calc.	09-Sep-14/O	0.635		
EC(calc.)/EC(actual)	-		Calc.	09-Sep-14/O	1.08		
Langelier Index(25°C)	S.I.		Calc.	09-Sep-14/O	0.333		

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M.D.L. = Method Detection Limit
Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill



Final Report

C.O.C.: G31363 REPORT No. B14-31309

Report To:

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

Attention: Dan MacIntyre

DATE RECEIVED: 03-Dec-14
DATE REPORTED: 08-Dec-14

SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

2378 Holly Lane

Ottawa Ontario K1V 7P1 Tel: 613-526-0123

Fax: 613-526-1244

JOB/PROJECT NO.: Pilgrims Rest

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-4 - (5hrs)		
			Sample I.D.		B14-31309-1		
			Date Collecte	ed	02-Dec-14		
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Hardness (as CaCO3)	mg/L	1	SM 3120	04-Dec-14/O	181		
Alkalinity(CaCO3) to pH4.5	mg/L	5	SM 2320B	04-Dec-14/O	145		
Bicarbonate(as CaCO3)	mg/L	5	SM 2320B	04-Dec-14/O	145		
Carbonate (as CaCO3)	mg/L	5	SM 2320B	04-Dec-14/O	< 5		
Conductivity @25°C	µmho/cm	1	SM 2510B	04-Dec-14/O	338		
pH @25°C	pH Units		SM 4500H	04-Dec-14/O	7.74		
Colour	TCU	2	SM 2120C	05-Dec-14/O	19		
Turbidity	NTU	0.1	SM 2130	05-Dec-14/O	134		
Fluoride	mg/L	0.1	SM4110C	05-Dec-14/O	0.3		
Chloride	mg/L	0.5	SM4110C	05-Dec-14/O	4.4		
Nitrite (N)	mg/L	0.1	SM4110C	05-Dec-14/O	< 0.1		
Nitrate (N)	mg/L	0.1	SM4110C	05-Dec-14/O	< 0.1		
Sulphate	mg/L	1	SM4110C	05-Dec-14/O	29		
Calcium	mg/L	0.02	SM 3120	04-Dec-14/O	63.3		
Magnesium	mg/L	0.01	SM 3120	04-Dec-14/O	5.64		
Sodium	mg/L	0.2	SM 3120	04-Dec-14/O	8.5		
Potassium	mg/L	0.1	SM 3120	04-Dec-14/O	2.6		
Copper	mg/L	0.002	SM 3120	04-Dec-14/O	< 0.002		
Iron	mg/L	0.005	SM 3120	04-Dec-14/O	9.98		
Manganese	mg/L	0.001	SM 3120	04-Dec-14/O	0.693		
Zinc	mg/L	0.005	SM 3120	04-Dec-14/O	0.018		
Ammonia (N)-Total	mg/L	0.01	MOEE 3364	04-Dec-14/O	< 0.01		
o-Phosphate (P)	mg/L	0.01	MOEE 3366	04-Dec-14/O	< 0.01		
Dissolved Organic Carbon	mg/L	0.2	EPA 415.1	04-Dec-14/O	4.6		
Total Coliform	cfu/100mL	1	MOE E3407	03-Dec-14/O	0		
E coli	cfu/100mL	1	MOE E3407	03-Dec-14/O	0		
Heterotrophic Plate Count	cfu/mL	2	SM 9215C	03-Dec-14/O	46		
Anion Sum	meq/L		Calc.	08-Dec-14/O	3.62		

NOTE: Groundwater not field-filtered for metals.

M.D.L. = Method Detection Limit Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill Greg Clarkin , BSc., C. Chem Lab Manager - Ottawa District



Final Report

C.O.C.: G31363 REPORT No. B14-31309

Report To:

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

Attention: Dan MacIntyre

DATE RECEIVED: 03-Dec-14

DATE REPORTED: 08-Dec-14
SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

2378 Holly Lane

Ottawa Ontario K1V 7P1

Tel: 613-526-0123 Fax: 613-526-1244

JOB/PROJECT NO.: Pilgrims Rest

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-4 - (5hrs)		
			Sample I.D.		B14-31309-1		
			Date Collect	ed	02-Dec-14		
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Cation Sum	meq/L		Calc.	08-Dec-14/O	4.62		
% Difference	%		Calc.	08-Dec-14/O	12.1 <sup>1</sup>		
Ion Ratio	AS/CS		Calc.	08-Dec-14/O	0.783		
Sodium Adsorption Ratio	-		Calc.	08-Dec-14/O	0.274		
TDS(ion sum calc.)	mg/L		Calc.	08-Dec-14/O	211		
Conductivity (calc.)	µmho/cm		Calc.	08-Dec-14/O	370		
TDS(calc.)/EC(actual)	-		Calc.	08-Dec-14/O	0.624		
EC(calc.)/EC(actual)	-		Calc.	08-Dec-14/O	1.10		
Langelier Index(25°C)	S.I.		Calc.	08-Dec-14/O	0.269		

<sup>1</sup> Outside of 10% Acceptance Criteria

NOTE: Groundwater not field-filtered for metals.

M.D.L. = Method Detection Limit Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill Greg Clarkin , BSc., C. Chem Lab Manager - Ottawa District



Final Report

C.O.C.: DW38874 REPORT No. B14-31467

Client I D

**Report To:** 

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

Attention: Christa Lemelin

DATE RECEIVED: 04-Dec-14

DATE REPORTED: 11-Dec-14

SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

285 Dalton Ave

Kingston Ontario K7K 6Z1

Tel: 613-544-2001 Fax: 613-544-2770

JOB/PROJECT NO.: Pilgrims Rest

P.O. NUMBER: 12-1629

WATERWORKS NO.

T\//\_4

			Client I.D.		TW-4		
			Sample I.D.		B14-31467-1		
			Date Collecte	ed	02-Dec-14		
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Total Coliform	cfu/100mL	1	MOE E3407	04-Dec-14/K	12		
E coli	cfu/100mL	1	MOE E3407	04-Dec-14/K	0		
Heterotrophic Plate Count	cfu/mL	10	SM9215D	04-Dec-14/K	90		
Alkalinity(CaCO3) to pH4.5	mg/L	3	SM 2320	05-Dec-14/K	143		
Carbonate (as CaCO3)	mg/L	3	SM 2320	05-Dec-14/K	< 3		
Bicarbonate(as CaCO3)	mg/L	3	SM 2320	05-Dec-14/K	143		
pH @25°C	pH Units		SM4500H+	05-Dec-14/K	7.80		
Conductivity @25°C	µmho/cm	1	SM 2510B	08-Dec-14/O	336		
Colour	TCU	2	SM2120C	05-Dec-14/K	< 2		
Turbidity	NTU	0.2	SM2130B	05-Dec-14/K	75.2		
Fluoride	mg/L	0.1	SM4110C	08-Dec-14/O	0.3		
Chloride	mg/L	0.5	SM4110C	08-Dec-14/O	5.4		
Nitrite (N)	mg/L	0.1	SM4110C	08-Dec-14/O	< 0.1		
Nitrate (N)	mg/L	0.1	SM4110C	08-Dec-14/O	< 0.1		
Sulphate	mg/L	1	SM4110C	08-Dec-14/O	32		
o-Phosphate (P)	mg/L	0.01	PE4500-S	10-Dec-14/K	0.02		
Ammonia (N)-Total	mg/L	0.01	SM4500- NH3-H	05-Dec-14/K	0.04		
Dissolved Organic Carbon	mg/L	0.2	EPA 415.1	09-Dec-14/O	22.9		
TDS (Calc. from Cond.)	mg/L		Calc.	09-Dec-14	226		
Hardness (as CaCO3)	mg/L	1	SM 3120	08-Dec-14/O	159		
Calcium	mg/L	0.02	SM 3120	08-Dec-14/O	55.0		
Copper	mg/L	0.002	SM 3120	08-Dec-14/O	< 0.002		
Iron	mg/L	0.005	SM 3120	08-Dec-14/O	7.79		
Magnesium	mg/L	0.01	SM 3120	08-Dec-14/O	5.16		
Manganese	mg/L	0.001	SM 3120	08-Dec-14/O	0.596		
Potassium	mg/L	0.1	SM 3120	08-Dec-14/O	2.3		
Sodium	mg/L	0.2	SM 3120	08-Dec-14/O	11.5		

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M.D.L. = Method Detection Limit Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill



**Final Report** 

C.O.C.: DW38874 REPORT No. B14-31467

**Report To:** 

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

Attention: Christa Lemelin

DATE RECEIVED: 04-Dec-14

DATE REPORTED: 11-Dec-14

SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

285 Dalton Ave

Kingston Ontario K7K 6Z1

Tel: 613-544-2001

Fax: 613-544-2770

JOB/PROJECT NO.: Pilgrims Rest

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-4		
			Sample I.D.		B14-31467-1		
			Date Collect	ed	02-Dec-14		
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Zinc	mg/L	0.005	SM 3120	08-Dec-14/O	0.012		
Anion Sum	meq/L		Calc.	09-Dec-14/O	3.70		
Cation Sum	meq/L		Calc.	09-Dec-14/O	4.17		
% Difference	%		Calc.	09-Dec-14/O	6.04		
Ion Ratio	AS/CS		Calc.	09-Dec-14/O	0.886		
Sodium Adsorption Ratio	-		Calc.	09-Dec-14/O	0.397		
Langelier Index(25°C)	S.I.		Calc.	09-Dec-14/O	0.264		

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M.D.L. = Method Detection Limit
Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill



Final Report

C.O.C.: G31370 **REPORT No. B15-02668** 

**Report To:** 

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

**Attention:** Brad Pettersone

DATE RECEIVED: 09-Feb-15

DATE REPORTED: 11-Feb-15

SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

2378 Holly Lane

Ottawa Ontario K1V 7P1

Tel: 613-526-0123

Fax: 613-526-1244

JOB/PROJECT NO.: Pilgrims Rest

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-6 - (5hrs)	TW-6 - (10hrs)	
			Sample I.D.  Date Collected		B15-02668-1	B15-02668-2	
					05-Feb-15	05-Feb-15	
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Hardness (as CaCO3)	mg/L	1	SM 3120	09-Feb-15/O	12	13	
Alkalinity(CaCO3) to pH4.5	mg/L	5	SM 2320B	09-Feb-15/O	147	145	
Bicarbonate(as CaCO3)	mg/L	5	SM 2320B	09-Feb-15/O	147	145	
Carbonate (as CaCO3)	mg/L	5	SM 2320B	09-Feb-15/O	26	18	
Conductivity @25°C	µmho/cm	1	SM 2510B	09-Feb-15/O	315	329	
pH @25°C	pH Units		SM 4500H	09-Feb-15/O	8.85	8.78	
Colour	TCU	2	SM 2120C	10-Feb-15/O	6	7	
Turbidity	NTU	0.1	SM 2130	09-Feb-15/O	3.2	2.8	
Chloride	mg/L	0.5	SM4110C	09-Feb-15/O	1.7	2.1	
Fluoride	mg/L	0.1	SM4110C	09-Feb-15/O	1.9	2.0	
Nitrite (N)	mg/L	0.1	SM4110C	09-Feb-15/O	< 0.1	< 0.1	
Nitrate (N)	mg/L	0.1	SM4110C	09-Feb-15/O	< 0.1	< 0.1	
Sulphate	mg/L	1	SM4110C	09-Feb-15/O	18	24	
Ammonia (N)-Total	mg/L	0.01	MOEE 3364	11-Feb-15/O	0.04	0.03	
o-Phosphate (P)	mg/L	0.01	MOEE 3366	11-Feb-15/O	< 0.01	< 0.01	
Dissolved Organic Carbon	mg/L	0.2	EPA 415.1	09-Feb-15/O	1.3	1.3	
Calcium	mg/L	0.02	SM 3120	09-Feb-15/O	3.40	3.30	
Magnesium	mg/L	0.01	SM 3120	09-Feb-15/O	0.97	1.06	
Sodium	mg/L	0.2	SM 3120	09-Feb-15/O	69.3	71.9	
Potassium	mg/L	0.1	SM 3120	09-Feb-15/O	0.9	0.8	
Copper	mg/L	0.002	SM 3120	09-Feb-15/O	< 0.002	< 0.002	
Iron	mg/L	0.005	SM 3120	09-Feb-15/O	0.248	0.266	
Manganese	mg/L	0.001	SM 3120	09-Feb-15/O	0.009	0.009	
Uranium	mg/L	0.00005	EPA 200.8	09-Feb-15/O	0.00950	0.00939	
Zinc	mg/L	0.005	SM 3120	09-Feb-15/O	< 0.005	< 0.005	
Total Coliform	cfu/100mL	1	MOE E3407	07-Feb-15/O	0	0	
E coli	cfu/100mL	1	MOE E3407	07-Feb-15/O	0	0	

M.D.L. = Method Detection Limit Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill Krystyna Pipin, M. Sc.



Final Report

C.O.C.: G31370 **REPORT No. B15-02668** 

**Report To:** 

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

**Attention:** Brad Pettersone

DATE RECEIVED: 09-Feb-15

DATE REPORTED: 11-Feb-15 SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

2378 Holly Lane

Ottawa Ontario K1V 7P1

Tel: 613-526-0123

Fax: 613-526-1244

JOB/PROJECT NO.: Pilgrims Rest

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-6 - (5hrs)	TW-6 - (10hrs)	
			Sample I.D.		B15-02668-1	B15-02668-2	
			Date Collected		05-Feb-15	05-Feb-15	
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed			
Heterotrophic Plate Count	cfu/mL	2	SM 9215C	07-Feb-15/O	358	142	
Anion Sum	meq/L		Calc.	10-Feb-15/O	3.45	3.55	
Cation Sum	meq/L		Calc.	10-Feb-15/O	3.30	3.42	
% Difference	%		Calc.	10-Feb-15/O	2.19	1.89	
Ion Ratio	AS/CS		Calc.	10-Feb-15/O	1.04	1.04	
Sodium Adsorption Ratio	-		Calc.	10-Feb-15/O	8.53	8.82	
TDS(ion sum calc.)	mg/L		Calc.	10-Feb-15/O	184	192	
Conductivity (calc.)	µmho/cm		Calc.	10-Feb-15/O	309	322	
TDS(calc.)/EC(actual)	-		Calc.	10-Feb-15/O	0.585	0.583	
EC(calc.)/EC(actual)	-		Calc.	10-Feb-15/O	0.982	0.978	
Langelier Index(25°C)	S.I.		Calc.	10-Feb-15/O	0.135	0.0366	

1 HPC duplicate = 270 cfu

Krystyna Pipin, M. Sc. Lab Supervisor



Final Report

C.O.C.: G31373 REPORT No. B15-03624

Rev. 1

Report To:

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

**Attention:** Dan MacIntyre

DATE RECEIVED: 21-Feb-15

DATE REPORTED: 17-Apr-15

SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

2378 Holly Lane

Ottawa Ontario K1V 7P1 Tel: 613-526-0123 Fax: 613-526-1244

JOB/PROJECT NO.: Piligrim's

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-5 - (6hrs)	TW-5 - (12hrs)	TW-5 - (18hrs)	TW-5 - (24hrs)
			Sample I.D.		B15-03624-1	B15-03624-2	B15-03624-3	B15-03624-4
			Date Collect	ed	19-Feb-15	19-Feb-15	20-Feb-15	20-Feb-15
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed				•
Hardness (as CaCO3)	mg/L	1	SM 3120	23-Feb-15/O	75	59	57	58
Alkalinity(CaCO3) to pH4.5	mg/L	5	SM 2320B	23-Feb-15/O	168	162	158	157
Bicarbonate(as CaCO3)	mg/L	5	SM 2320B	23-Feb-15/O	168	162	158	157
Carbonate (as CaCO3)	mg/L	5	SM 2320B	23-Feb-15/O	< 5	< 5	< 5	< 5
Conductivity @25°C	µmho/cm	1	SM 2510B	23-Feb-15/O	339	339	339	338
pH @25°C	pH Units		SM 4500H	23-Feb-15/O	7.76	8.01	8.05	8.07
Colour	TCU	2	SM 2120C	25-Feb-15/O	13	27	19	17
Turbidity	NTU	0.1	SM 2130	25-Feb-15/O	13.8	2.5	2.1	1.3
Fluoride	mg/L	0.1	SM4110C	23-Feb-15/O	0.7	0.9	1.0	0.9
Chloride	mg/L	0.5	SM4110C	23-Feb-15/O	3.2	3.6	3.7	3.6
Nitrite (N)	mg/L	0.1	SM4110C	23-Feb-15/O	< 0.1	< 0.1	< 0.1	< 0.1
Nitrate (N)	mg/L	0.1	SM4110C	23-Feb-15/O	< 0.1	< 0.1	< 0.1	< 0.1
Sulphate	mg/L	1	SM4110C	23-Feb-15/O	19	20	20	20
Calcium	mg/L	0.02	SM 3120	23-Feb-15/O	22.6	17.3	16.7	16.8
Magnesium	mg/L	0.01	SM 3120	23-Feb-15/O	4.54	3.70	3.70	3.80
Sodium	mg/L	0.2	SM 3120	23-Feb-15/O	50.8	57.8	58.7	58.3
Potassium	mg/L	0.1	SM 3120	23-Feb-15/O	1.9	1.7	1.7	1.7
Copper	mg/L	0.002	SM 3120	23-Feb-15/O	< 0.002	< 0.002	< 0.002	< 0.002
Iron	mg/L	0.005	SM 3120	23-Feb-15/O	2.15	0.315	0.137	0.089
Manganese	mg/L	0.001	SM 3120	23-Feb-15/O	0.232	0.119	0.096	0.087
Zinc	mg/L	0.005	SM 3120	23-Feb-15/O	< 0.005	< 0.005	< 0.005	< 0.005
Ammonia (N)-Total	mg/L	0.01	MOEE 3364	23-Feb-15/O	0.05	0.05	0.05	0.05
o-Phosphate (P)	mg/L	0.01	MOEE 3366	23-Feb-15/O	< 0.01	< 0.01	< 0.01	< 0.01
Dissolved Organic Carbon	mg/L	0.2	EPA 415.1	23-Feb-15/O	4.0	3.6	3.6	3.5
Total Coliform	cfu/100mL	1	MOE E3407	21-Feb-15/O		0		0
E coli	cfu/100mL	1	MOE E3407	21-Feb-15/O		0		0
Heterotrophic Plate Count	cfu/mL	2	SM 9215C	21-Feb-15/O		112	1	34

Note: Revision created to correct Typo in sample I.D.

M.D.L. = Method Detection Limit

Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill

Gord Murphy



Final Report

C.O.C.: G31373 REPORT No. B15-03624

Rev. 1

Report To:

Oakridge Environmental

PO Box 431,

Peterborough ON K9J 6Z3 Canada

**Attention:** Dan MacIntyre

DATE RECEIVED: 21-Feb-15
DATE REPORTED: 17-Apr-15

SAMPLE MATRIX: Groundwater

**Caduceon Environmental Laboratories** 

2378 Holly Lane

Ottawa Ontario K1V 7P1 Tel: 613-526-0123 Fax: 613-526-1244

JOB/PROJECT NO.: Piligrim's

P.O. NUMBER: 12-1629

WATERWORKS NO.

			Client I.D.		TW-5 - (6hrs)	TW-5 - (12hrs)	TW-5 - (18hrs)	TW-5 - (24hrs)
			Sample I.D.		B15-03624-1	B15-03624-2	B15-03624-3	B15-03624-4
			Date Collect	ed	19-Feb-15	19-Feb-15	20-Feb-15	20-Feb-15
Parameter	Units	M.D.L.	Reference Method	Date/Site Analyzed				
Anion Sum	meq/L		Calc.	24-Feb-15/O	3.86	3.81	3.73	3.71
Cation Sum	meq/L		Calc.	24-Feb-15/O	3.89	3.76	3.75	3.74
% Difference	%		Calc.	24-Feb-15/O	0.409	0.694	0.333	0.418
Ion Ratio	AS/CS		Calc.	24-Feb-15/O	0.992	1.01	0.993	0.992
Sodium Adsorption Ratio	-		Calc.	24-Feb-15/O	2.55	3.29	3.38	3.34
TDS(ion sum calc.)	mg/L		Calc.	24-Feb-15/O	205	203	201	200
Conductivity (calc.)	µmho/cm		Calc.	24-Feb-15/O	353	350	347	346
TDS(calc.)/EC(actual)	-		Calc.	24-Feb-15/O	0.606	0.599	0.592	0.591
EC(calc.)/EC(actual)	-		Calc.	24-Feb-15/O	1.04	1.03	1.02	1.02
Langelier Index(25°C)	S.I.		Calc.	24-Feb-15/O	-0.0937	0.0260	-0.0806	0.0687

<sup>1</sup> Duplicate HPC = 60 cfu/mL

Note: Revision created to correct Typo in sample I.D.

M.D.L. = Method Detection Limit Site Analyzed=K-Kingston,W-Windsor,O-Ottawa,R-Richmond Hill Gord Murphy Lab Supervisor

# **APPENDIX G**

Well Certification Program

#### **Well Certification Program**

#### 1.0 Introduction

All development lots are subject to this *Well Certification Program*. It is the responsibility of the <u>lot owner</u> to ensure that this program is undertaken.

The Program requires that <u>prior to issuance of a Building Permit</u> for the lot, a Qualified Person is to be retained to provide assistance with respect to the placement and testing of private wells. A Qualified Person (QP) is a Hydrogeologist who is a licensed Professional Geoscientist in the Province of Ontario (APGO) or a licensed Professional Engineer with appropriate hydrogeological training and experience.

This Program draws upon the results of the hydrogeological study submitted in support of the development, County of Peterborough peer review, the Ontario Building Code, MOECC Procedure D-5-5 and/or the Ontario Drinking Water Quality Standards. Nothing in this Program should limit the Qualified Person from modifying the requirements as needed to suit the site conditions.

#### 2.0 Water Wells

The Program requires that a water well is to be constructed under the supervision of, and tested by, a Qualified Person who will certify as part of a written report that a drilled well has been constructed, meeting the minimum construction, water demand and water quality requirements as set forth herein. The report shall be submitted to the municipality as part of the Building Permit application. The Qualified Person is to ensure that the following tasks are completed:

- As a general guide, unless the Qualified Person recommends otherwise, new drilled wells are to be constructed at the locations illustrated on the accompanying Recommended Lot Servicing Plan Figure 12, from the Hydrogeological Study.
- Wherever possible, the distance separating wells and sewage systems is to be maximized, while complying with all required setbacks of O. Reg. 903, as amended, and the Ontario Building Code.
- Once the preceding constraints have been accommodated, a location for the new wells shall be staked-out in the field.
- All wells are to be drilled, constructed and sealed in accordance with O. Reg. 903, as amended at the location staked-out in the field.
- The target aquifer occurs within the Precambrian bedrock. Previous test well construction (during the hydrogeological study) has indicated that the aquifer(s) may require aggressive hydrofracturing in order to achieve an acceptable yield. The drilling contractor should be advised of this potential requirement in advance. Additional well development costs may, therefore, be incurred by the lot owner to

achieve a satisfactory outcome.

- The lot owner should be aware and understand that there is no guarantee that any new well constructed on the lot will be successful. More than one attempt to construct a well may be needed.
- The Qualified Person shall conduct a pumping test of the new well. The pumping test shall have a 6-hour minimum duration at a predetermined pumping rate as per the anticipated peak demand requirement referenced in MOECC Procedure D-5-5 (i.e., typically 18.75 L/m for a 4 bedroom residence). Following the pumping test there must be at least 95% water level recovery within 24 hours. The pumping test is to be conducted to determine if the well has an adequate and sustainable yield and whether supplemental water storage is required. A longer pumping test may be required in the case of a low yield well.
- The pumping test is to include water sampling and analysis of the parameters listed in MOECC Procedure D-5-5. Further well development may be necessary to demonstrate that turbidity is acceptable (i.e., not to exceed 5 NTU, in the absence of bacteria issue). Note: wellhead turbidity measurements can be more representative than laboratory reported data in some instances.
- Upon completion of the pumping test, the Qualified Person should advise the lot owner as to whether or not the well is acceptable for future use.
- In the event that any well is found to produce insufficient supply for domestic use, the Qualified Person shall instruct the lot owner as to the requirements of Ont. Reg. 903, as amended, with respect to the requirement for proper well abandonment. Alternatively, if deemed safe to do so by the Qualified Person, the well may be utilized as a supplementary source for lawn watering and other outdoor uses provided it does not constitute a route of access for surface or near surface sources of contamination. The Qualified Person supervising the well construction shall also ensure that the driller's contract includes appropriate stipulations concerning well abandonment. The lot owner should be aware that additional costs for well abandonment may be incurred, in the event that a well is not successful. The abandonment water well record shall be retained by the well owner and a copy included in the Qualified Person's report.
- In the event that a well is found to be unacceptable, hydraulic fracturing and/or a second attempt to construct a new well can be undertaken if desired. The Qualified Person shall ensure that the testing procedures outlined above are conducted on all new and hydrofracked wells. While there is no limit to the number of attempts that may be undertaken, the Qualified Person may provide recommendations for an alternative water supply, should well construction on the lot be deemed "unlikely to succeed" (see below).

#### 3.0 Alternative Water Supply

In the event that a minimum of three attempts to obtain a suitable water source for the lot are not successful, the Qualified Person may recommend any of the following alternatives, subject to obtaining permission from the municipality.

#### Shared Wells

- In the event that a suitable well, with sufficient excess yield is available on an immediately adjacent lot, the subject lot owner may enter into a private agreement with that lot owner for the sharing of the adjacent lot well. The owner of the subject lot will be responsible for arranging any legal agreements, contracts and/or easements necessary to facilitate the well sharing, sharing of water treatment (if applicable) sharing of well maintenance tasks/costs and sharing of wellhead protection tasks/costs.
- Prior to any such connection to a neighbouring well, the Qualified Person must either:
  - a) review an existing *Well Certification Program* report to verify that the neighbouring well has the needed yield and quality to support the combined water demands, or
  - b) conduct a new pumping test as outlined above, modified as needed to be applicable to the combined water demands, and
  - c) shall prepare the *Well Certification Program* report for the subject lot (see additional requirements, below) indicating how the subject lot will be serviced for water supply by the neighbouring well.
- Although there is no specific prohibition with regard to the sharing of private wells, the lot owner should be discouraged from utilizing this alternative unless absolutely necessary.
- Under no circumstances should a single well be connected to more than five (5) residences.

#### Surface Water Supply

• In the unlikely event that an approved well water supply is not possible, then the lot owner may employ as a last resort and only with the <u>written permission of the municipality</u>, a surface water source to ensure a supply of potable water to the subject lot. A surface water source would be subject to the recommendations of a Qualified Person with respect to water quality treatment requirements to meet the Ontario Drinking Water Quality Standards and security of the water source. Prior to applying for such permission, the lot owner must demonstrate that reasonable attempts have been made to secure an acceptable groundwater source. Use of a surface water source may not be practical for all lots.

- To implement a surface water supply, if approved (in concept) by the Municipality, the Qualified Person shall conduct whatever investigation is necessary to select the best location for a lake intake based on factors such as water quality, avoidance of navigation hazard, avoidance of impact to sensitive environmental features (inwater and/or within the shoreline buffer), etc. Consultation with, and/or approval by review agencies may be required in this instance.
- The Qualified Person shall provide a recommendation with regard to the appropriate treatment requirements to ensure a safe water supply. An opinion from a water treatment specialist may be required.

### 4.0 Report

A *Well Certification Program* report is to be prepared by a Qualified Person and submitted to the municipality in support of the application for a Building Permit for each lot. The report shall include the following.

- A description of the subject lot with regard to size, topography, drainage, soil
  conditions and any sensitive environmental features. Information may be obtained
  from a combination of in-field observations and descriptions provided in the
  hydrogeological study.
- A recent survey of the lot boundaries.
- The location of the on-site water well (or alternative supply, in exceptional circumstances). The location (footprint) of the on-site sewage system (existing or proposed).
- A copy of the well record(s).
- A description of the required pumping test and water quality data.
- A professional opinion indicating that the source and treatment system (as outlined in the report), will provide an adequate and sustainable supply of acceptable quality water for the subject lot. A summary of any unknowns or limitations on that opinion shall also be provided.
- Recommendations regarding, but not limited to: water treatment; supplemental water storage; wellhead protection; regular testing; maintenance; water conservation, and any other matters deemed appropriate by the Qualified Person.

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